# ATES SUBDIVISION UNIT

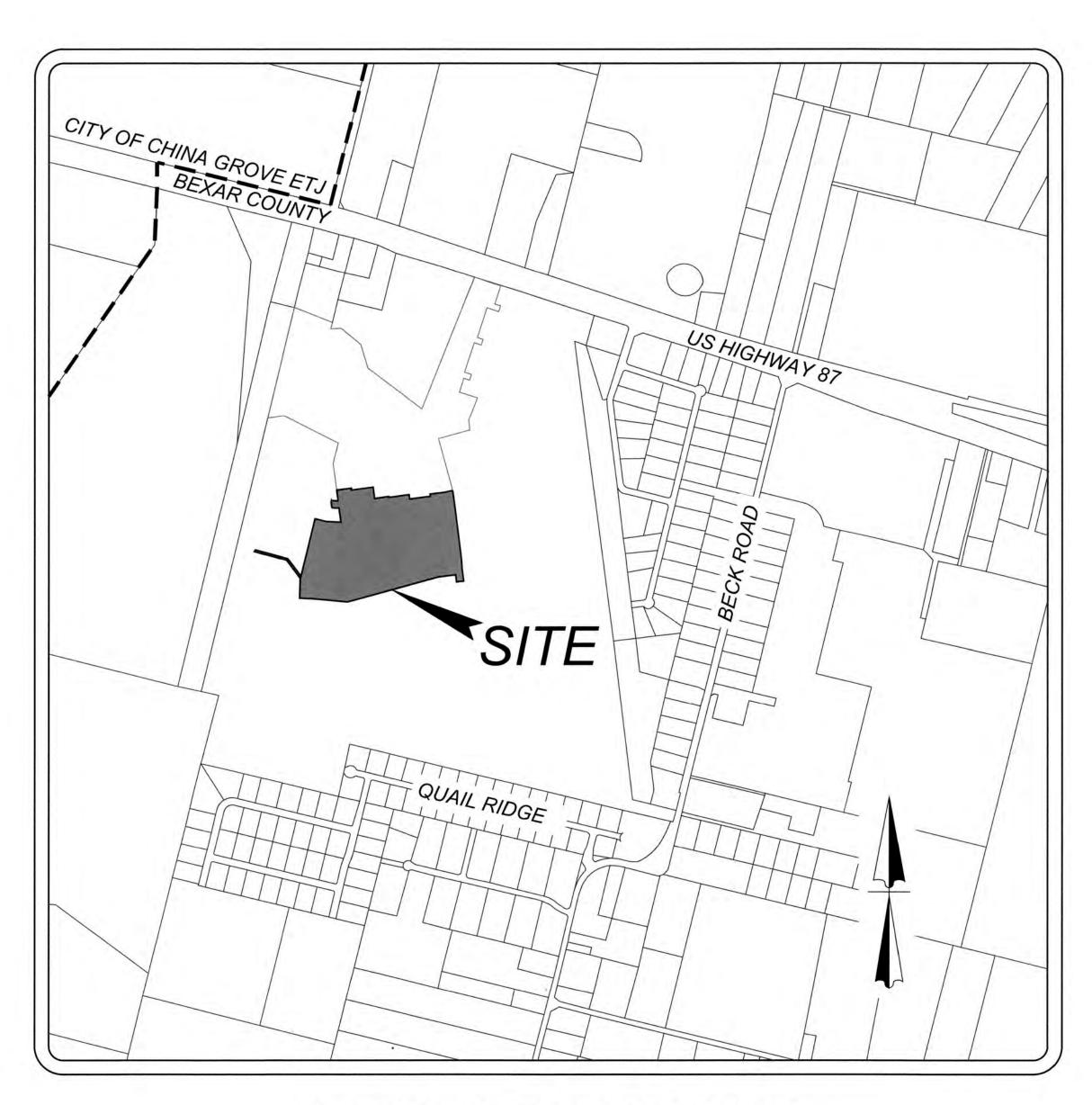
SHEET

NUMBER

### GATES SUBDIVISION UNIT 2

BEXAR COUNTY, TX

STREET, DRAINAGE, WATER, SANITARY SEWER, AND UTILITY IMPROVEMENTS



LOCATION MAP NOT-TO-SCALE

OWNER INFORMATION
MEH HOLDING COMPANY LTD
5210 THOUSAND OAKS, SUITE 1318
SAN ANTONIO, TX 78233

FROSTW	OOD HTS PLAN & PROFILE
CONCRE	TE DRIVEWAY DETAILS
WHEELC	HAIR RAMP DETAILS
TXDOT P	EDESTRIAN CURB RAMP DETAIL
TYPICAL	STREET DETAILS
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PRIVATE DRAIN "B" PLAN & PROFILE

FIELDSTONE WAY PLAN & PROFILE

HAWKWELL LN PLAN & PROFILE

PRIVATE DRAIN "A" PLAN & PROFILE (SHEET 1 OF 3)

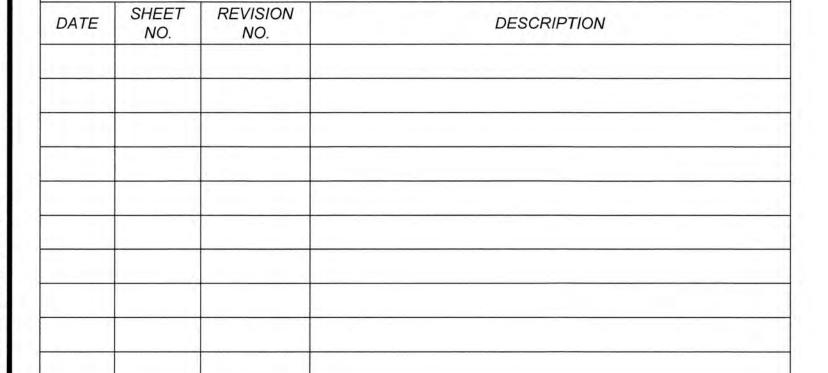
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MASTER DRAINAGE PLAN

OVERALL GRADING PLAN

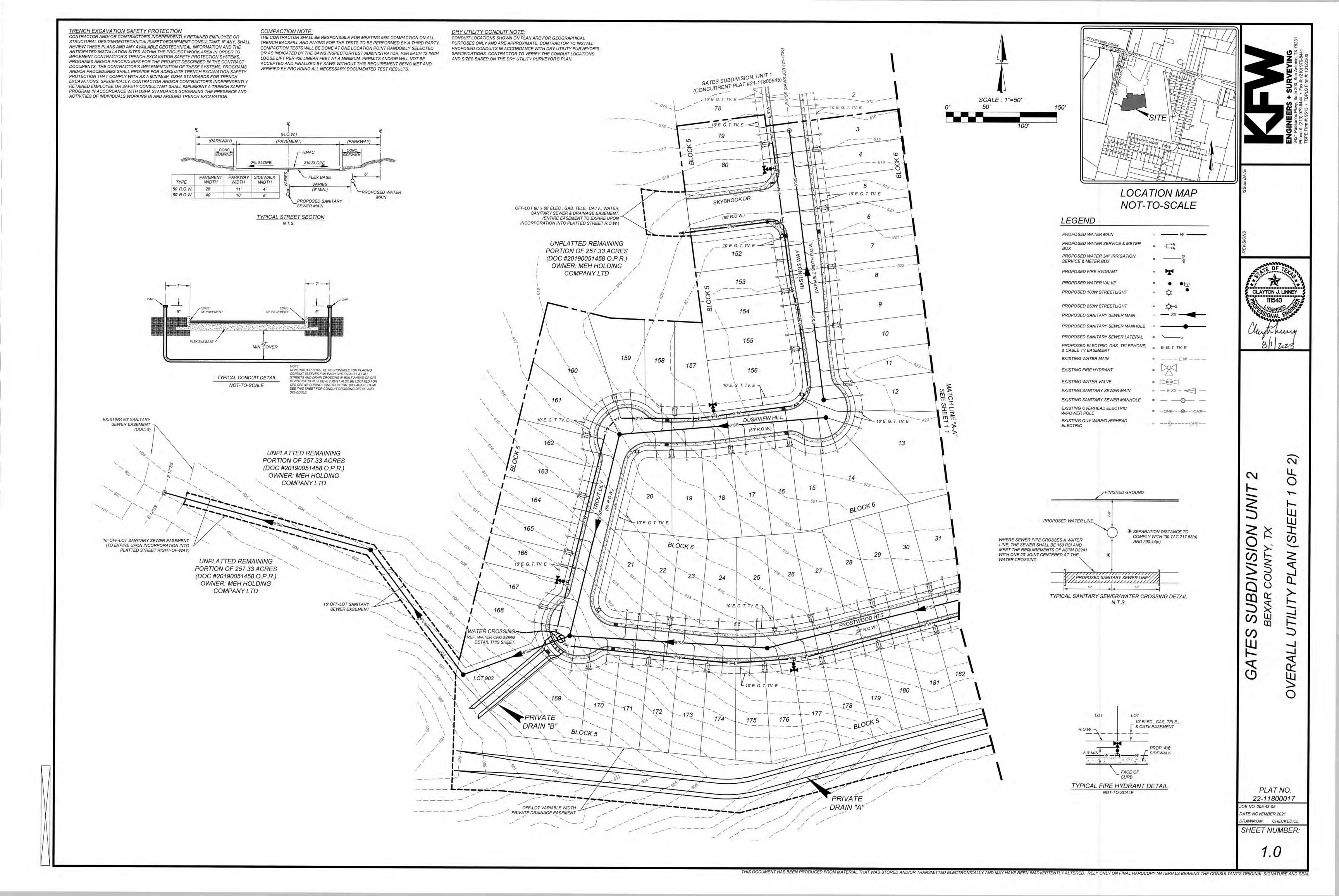


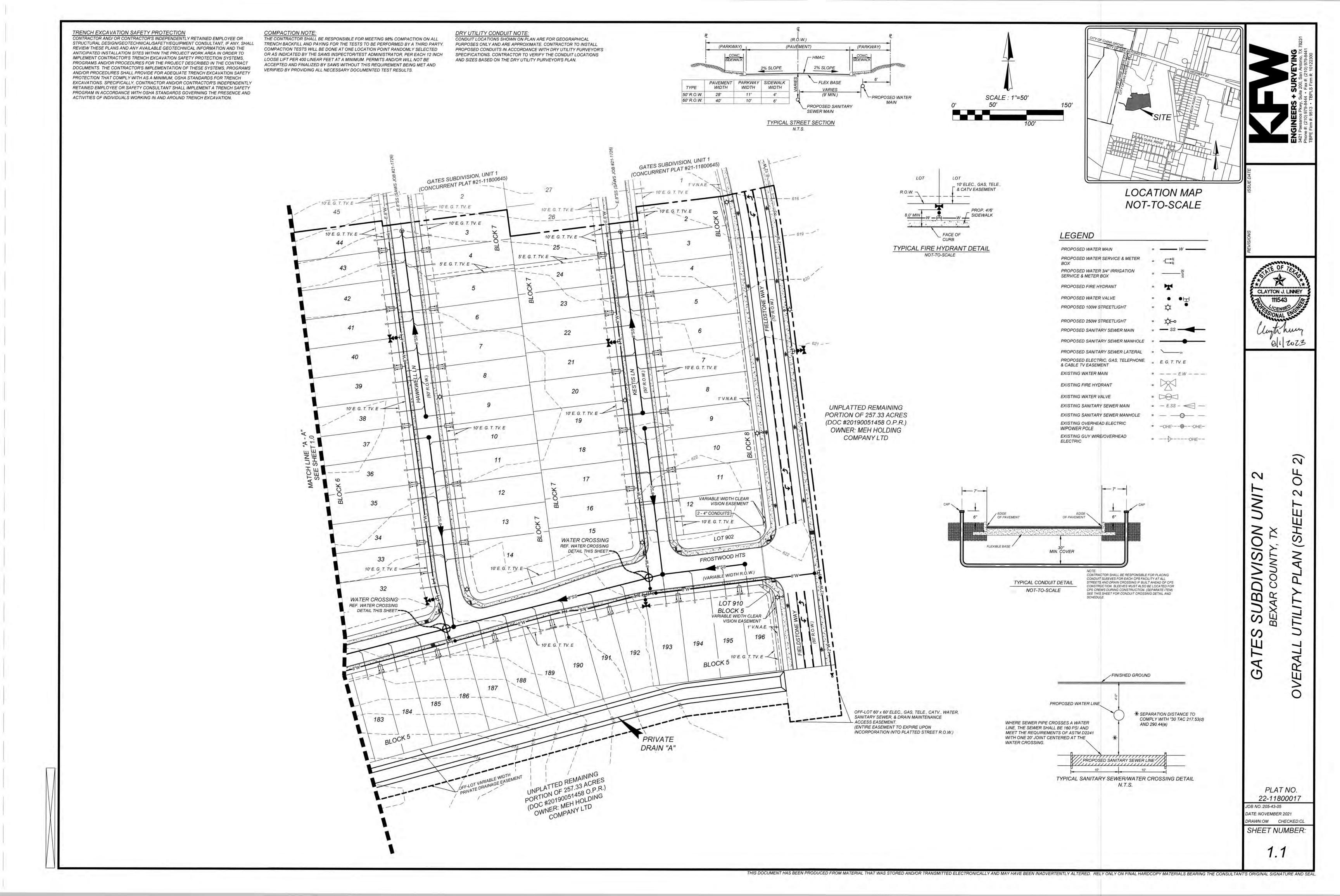
SHEET REVISION LOG





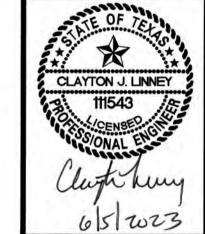
OB NO - 205-43-0











GATES SUBDIVISION UNIT 2 BEXAR COUNTY, TX

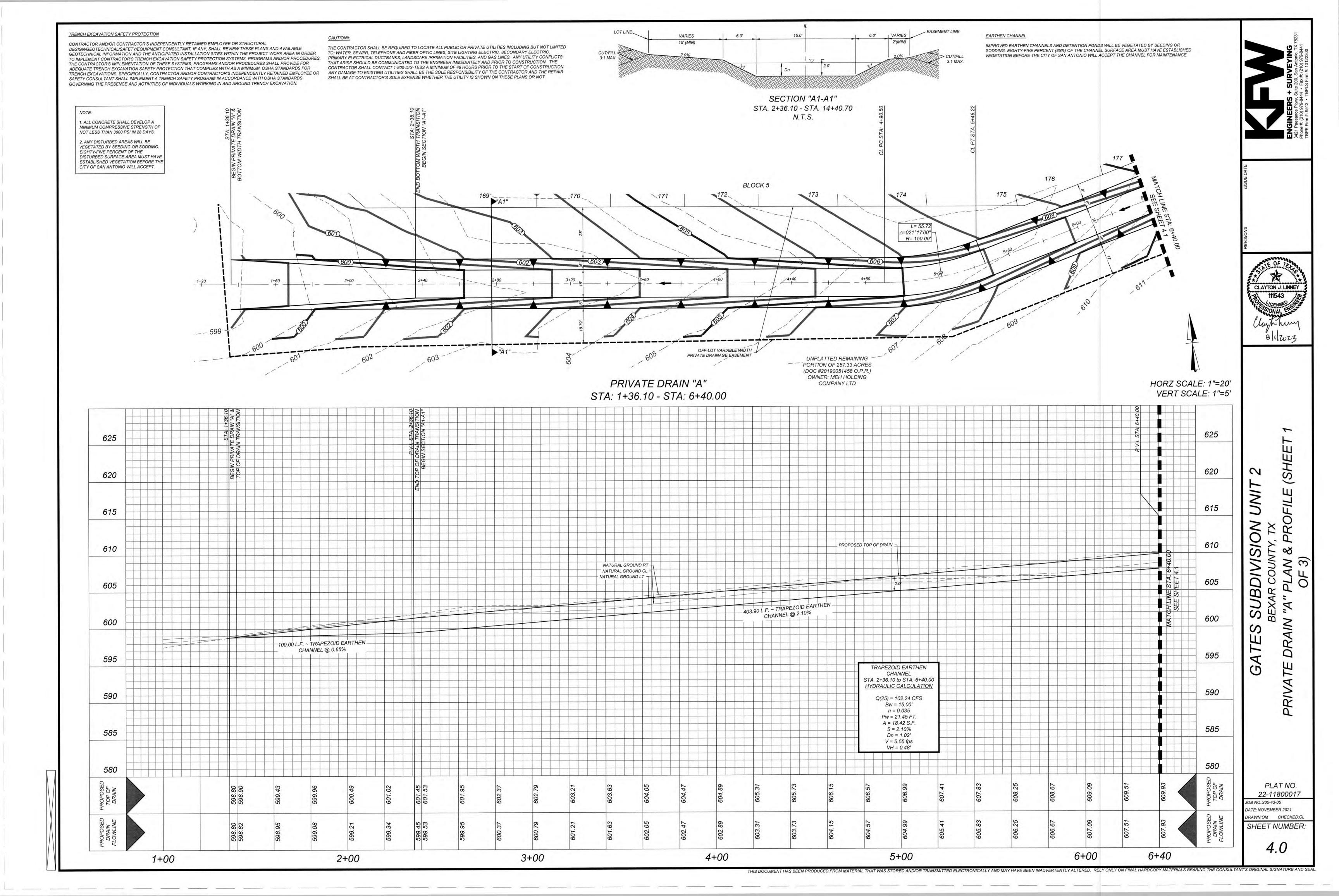
PLAN

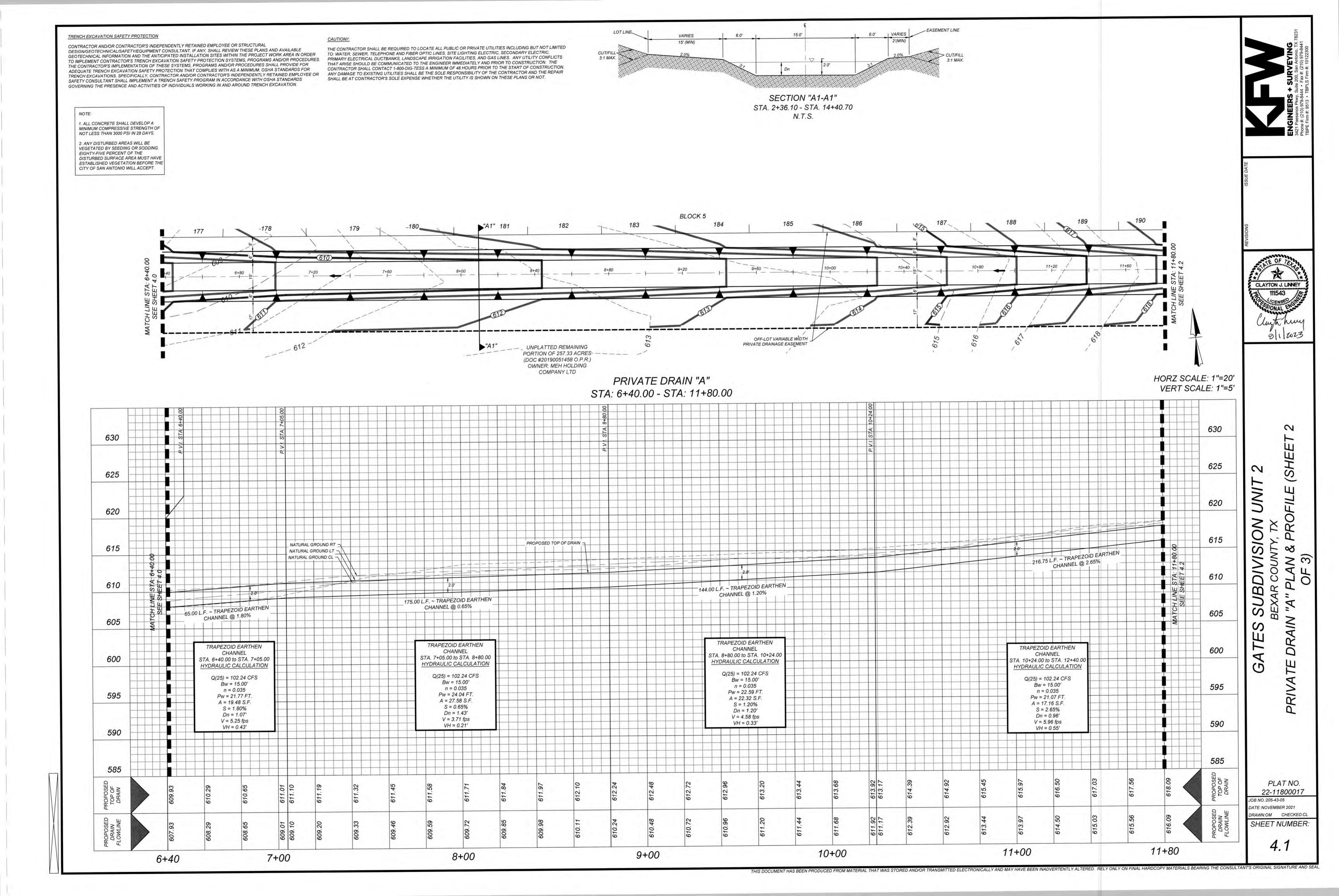
MASTER DRAINAGE

PLAT NO. 22-11800017 3 NO.:205-43-05

JOB NO.:205-43-05 DATE: NOVEMBER 2021 DRAWN:OM CHECKED:CL SHEET NUMBER:







#### TRENCH EXCAVATION SAFETY PROTECTION

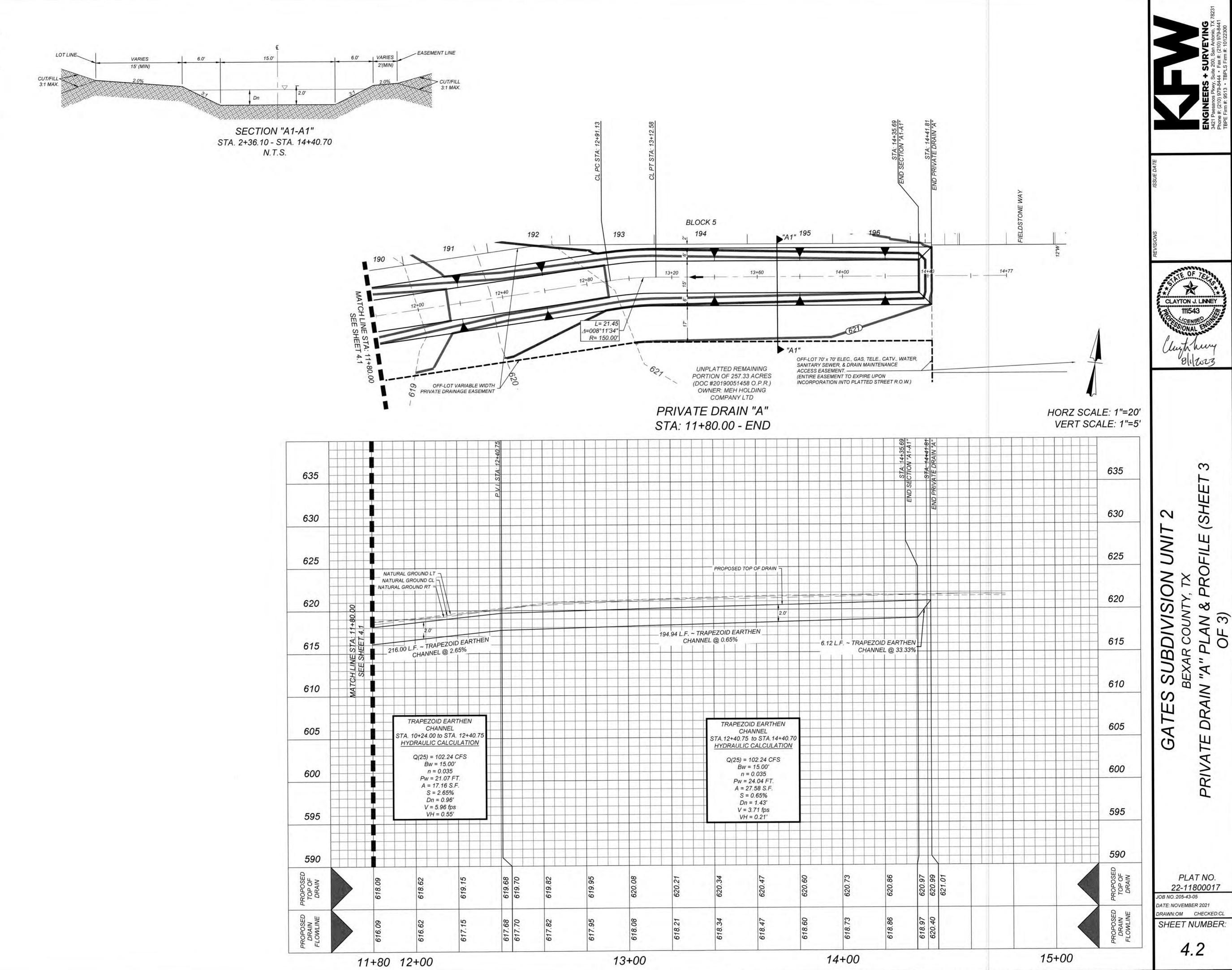
CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR STRUCTURAL DESIGN/GEOTECHNICAL/SAFETY/EQUIPMENT CONSULTANT, IF ANY, SHALL REVIEW THESE PLANS AND AVAILABLE GEOTECHNICAL INFORMATION AND THE ANTICIPATED INSTALLATION SITES WITHIN THE PROJECT WORK AREA IN ORDER TO IMPLEMENT CONTRACTOR'S TRENCH EXCAVATION SAFETY PROTECTION SYSTEMS, PROGRAMS AND/OR PROCEDURES. THE CONTRACTOR'S IMPLEMENTATION OF THESE SYSTEMS, PROGRAMS AND/OR PROCEDURES SHALL PROVIDE FOR ADEQUATE TRENCH EXCAVATION SAFETY PROTECTION THAT COMPLIES WITH AS A MINIMUM, OSHA STANDARDS FOR TRENCH EXCAVATIONS. SPECIFICALLY, CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR SAFETY CONSULTANT SHALL IMPLEMENT A TRENCH SAFETY PROGRAM IN ACCORDANCE WITH OSHA STANDARDS GOVERNING THE PRESENCE AND ACTIVITIES OF INDIVIDUALS WORKING IN AND AROUND TRENCH EXCAVATION.

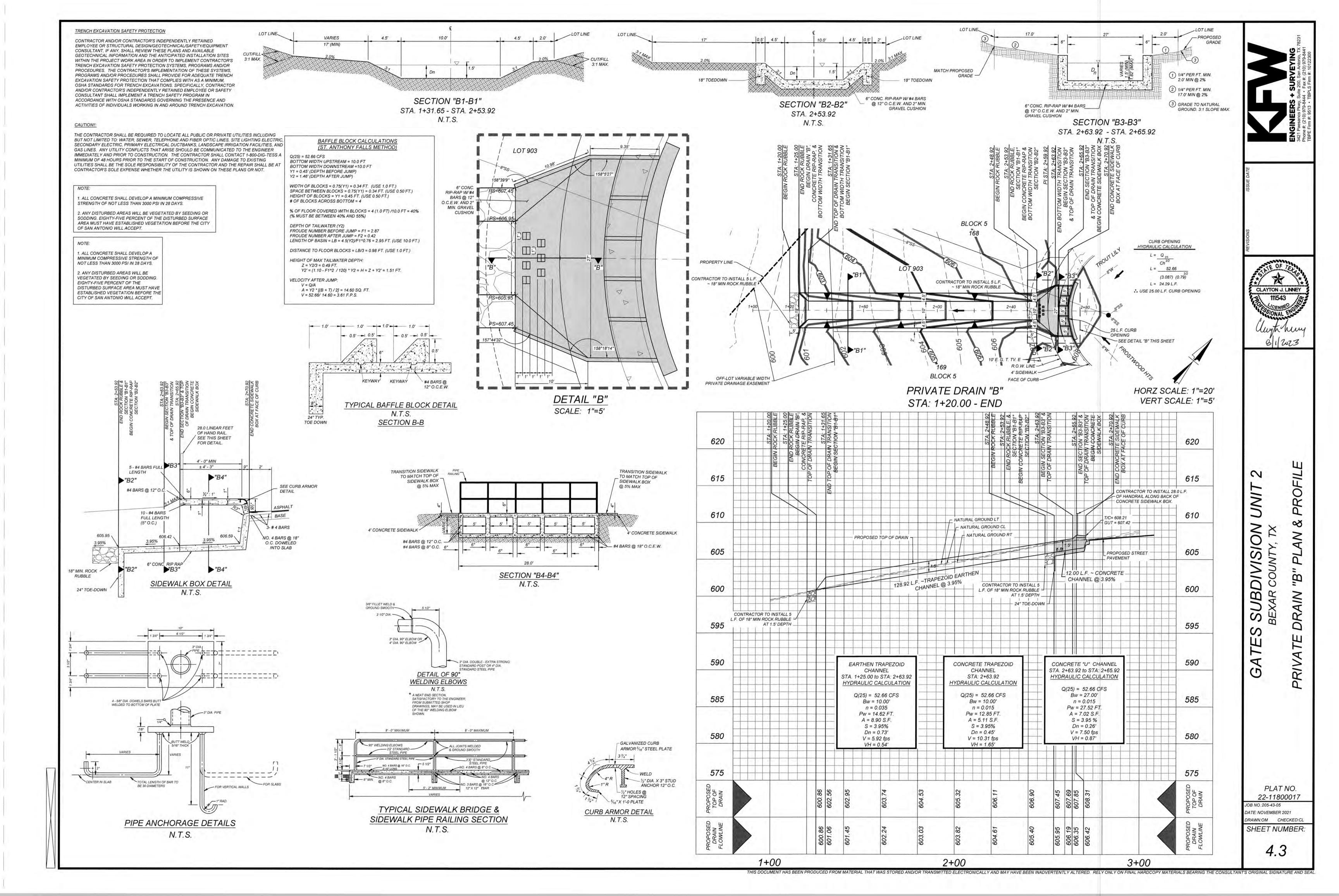
THE CONTRACTOR SHALL BE REQUIRED TO LOCATE ALL PUBLIC OR PRIVATE UTILITIES INCLUDING BUT NOT LIMITED TO: WATER, SEWER, TELEPHONE AND FIBER OPTIC LINES, SITE LIGHTING ELECTRIC, SECONDARY ELECTRIC, PRIMARY ELECTRICAL DUCTBANKS, LANDSCAPE IRRIGATION FACILITIES, AND GAS LINES. ANY UTILITY CONFLICTS THAT ARISE SHOULD BE COMMUNICATED TO THE ENGINEER IMMEDIATELY AND PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL CONTACT 1-800-DIG-TESS A MINIMUM OF 48 HOURS PRIOR TO THE START OF CONSTRUCTION. ANY DAMAGE TO EXISTING UTILITIES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND THE REPAIR SHALL BE AT CONTRACTOR'S SOLE EXPENSE WHETHER THE UTILITY IS SHOWN ON THESE PLANS OR NOT.

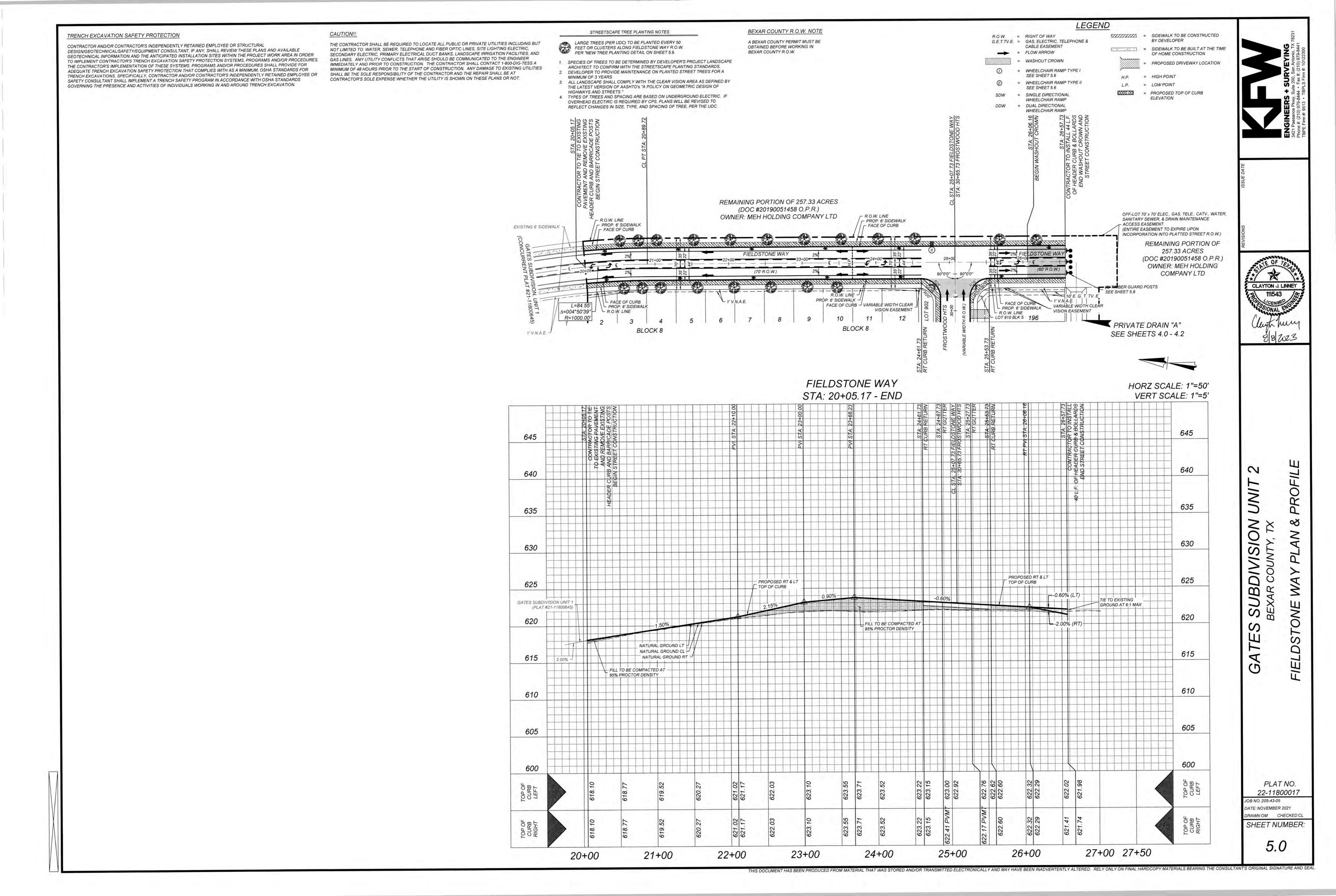
#### NOTE:

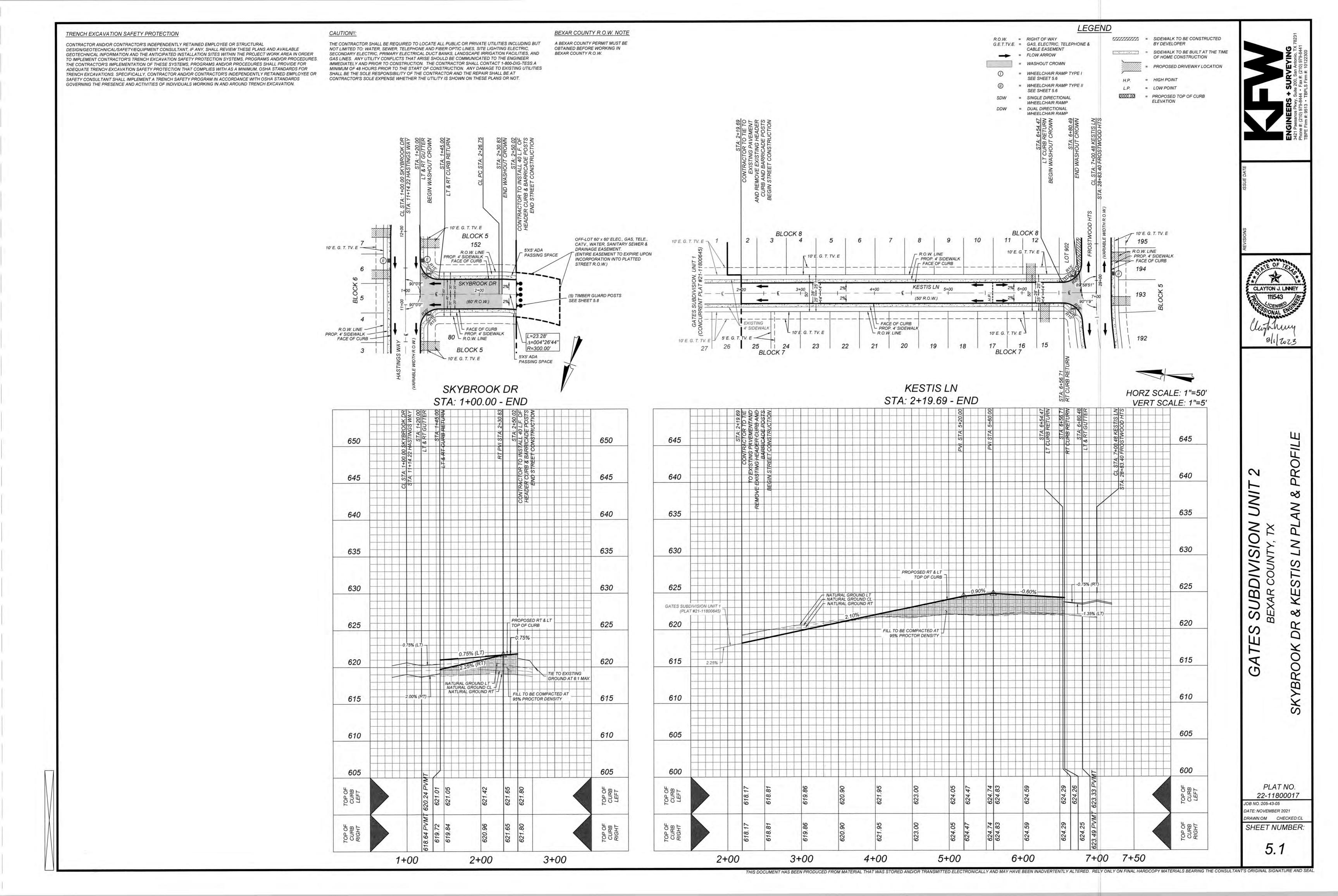
1. ALL CONCRETE SHALL DEVELOP A
MINIMUM COMPRESSIVE STRENGTH OF
NOT LESS THAN 3000 PSI IN 28 DAYS.

2. ANY DISTURBED AREAS WILL BE VEGETATED BY SEEDING OR SODDING. EIGHTY-FIVE PERCENT OF THE DISTURBED SURFACE AREA MUST HAVE ESTABLISHED VEGETATION BEFORE THE CITY OF SAN ANTONIO WILL ACCEPT.









TRENCH EXCAVATION SAFETY PROTECTION

CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR STRUCTURAL DESIGN/GEOTECHNICAL/SAFETY/EQUIPMENT CONSULTANT, IF ANY, SHALL REVIEW THESE PLANS AND AVAILABLE GEOTECHNICAL INFORMATION AND THE ANTICIPATED INSTALLATION SITES WITHIN THE PROJECT WORK AREA IN ORDER TO IMPLEMENT CONTRACTOR'S TRENCH EXCAVATION SAFETY PROTECTION SYSTEMS, PROGRAMS AND/OR PROCEDURES. THE CONTRACTOR'S IMPLEMENTATION OF THESE SYSTEMS, PROGRAMS AND/OR PROCEDURES SHALL PROVIDE FOR ADEQUATE TRENCH EXCAVATION SAFETY PROTECTION THAT COMPLIES WITH AS A MINIMUM, OSHA STANDARDS FOR TRENCH EXCAVATIONS. SPECIFICALLY, CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR SAFETY CONSULTANT SHALL IMPLEMENT A TRENCH SAFETY PROGRAM IN ACCORDANCE WITH OSHA STANDARDS GOVERNING THE PRESENCE AND ACTIVITIES OF INDIVIDUALS WORKING IN AND AROUND TRENCH EXCAVATION.

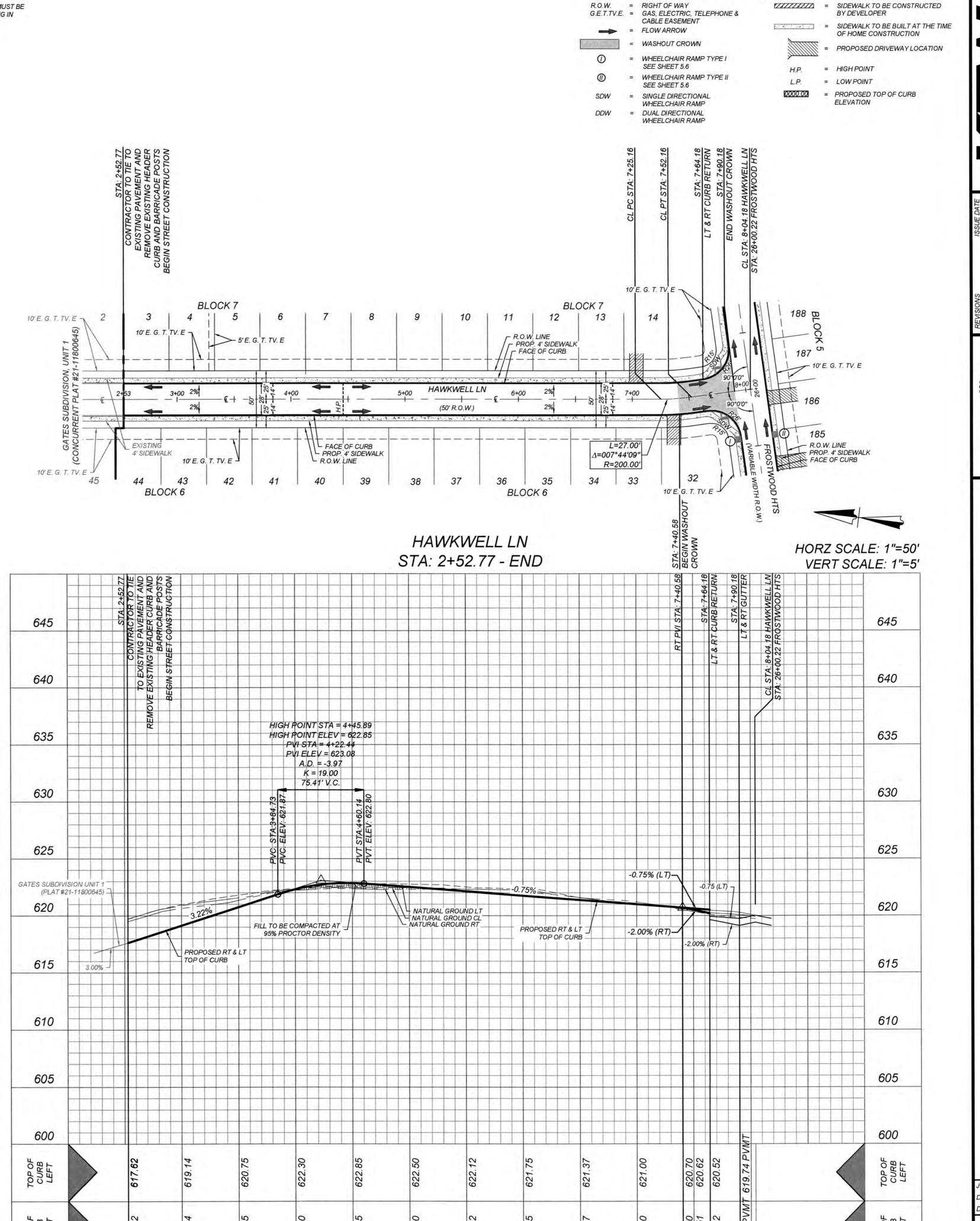
#### CAUTION!!:

THE CONTRACTOR SHALL BE REQUIRED TO LOCATE ALL PUBLIC OR PRIVATE UTILITIES INCLUDING BUT NOT LIMITED TO: WATER, SEWER, TELEPHONE AND FIBER OPTIC LINES, SITE LIGHTING ELECTRIC, SECONDARY ELECTRIC, PRIMARY ELECTRICAL DUCT BANKS, LANDSCAPE IRRIGATION FACILITIES, AND GAS LINES. ANY UTILITY CONFLICTS THAT ARISE SHOULD BE COMMUNICATED TO THE ENGINEER IMMEDIATELY AND PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL CONTACT 1-800-DIG-TESS A MINIMUM OF 48 HOURS PRIOR TO THE START OF CONSTRUCTION. ANY DAMAGE TO EXISTING UTILITIES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND THE REPAIR SHALL BE AT CONTRACTOR'S SOLE EXPENSE WHETHER THE UTILITY IS SHOWN ON THESE PLANS OR NOT.

#### BEXAR COUNTY R.O.W. NOTE

JT A BEXAR COUNTY PERMIT MUST BE OBTAINED BEFORE WORKING IN D BEXAR COUNTY R O W

2+50 3+00



5+00

4+00

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Phone #: (210) 979-8444 • Fax #: (210) 979-8441
TBPE Firm #: 9513 • TBPLS Firm #: 10122300

**LEGEND** 



GATES SUBDIVISION UNIT 2

BEXAR COUNTY, TX

HAMKINELL IN PLAN & PROFILE

PLAT NO. 22-11800017 JOB NO.:205-43-05

DATE: NOVEMBER 2021
DRAWN:OM CHECKED:CL
SHEET NUMBER:

5.2

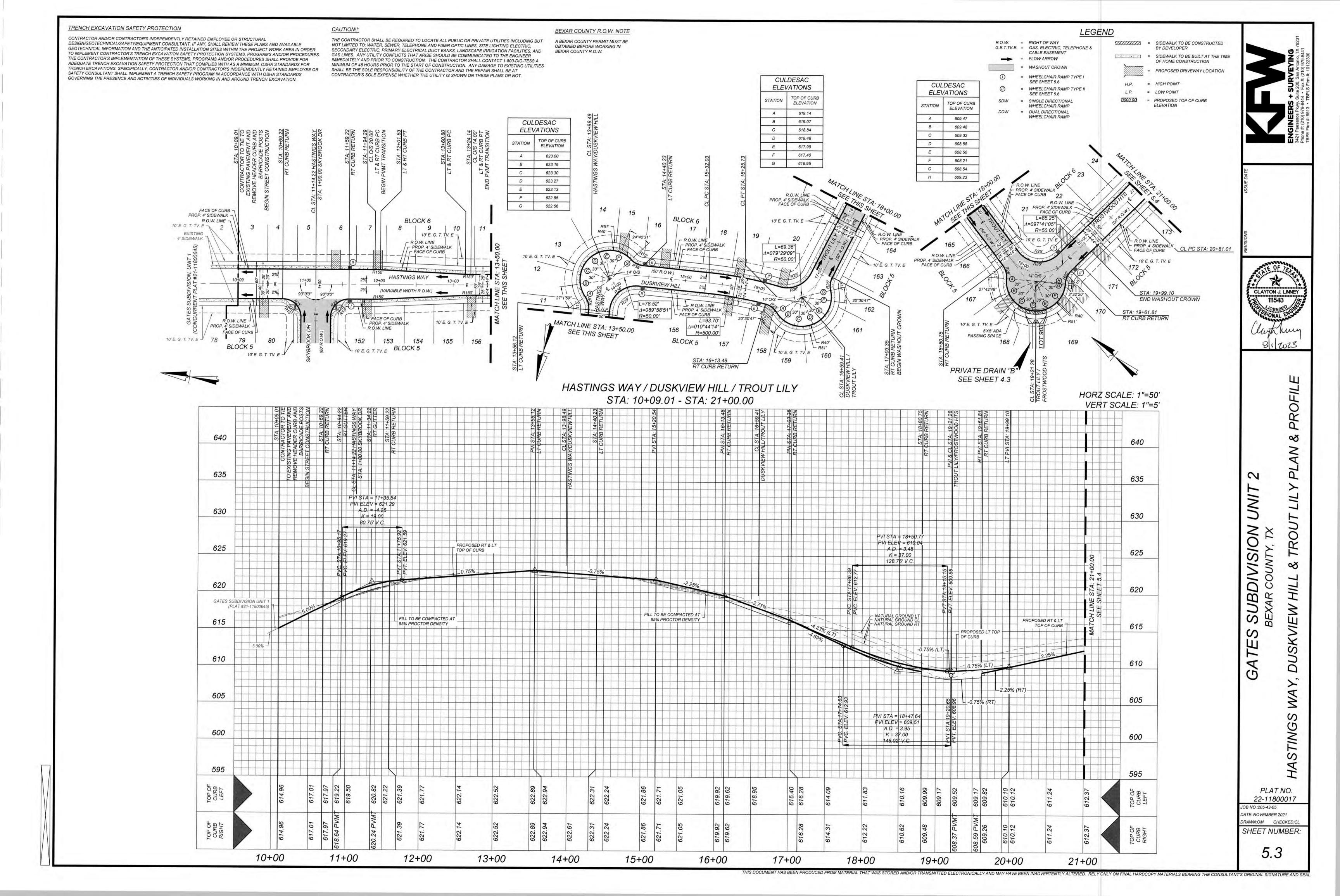
8+00 8+50

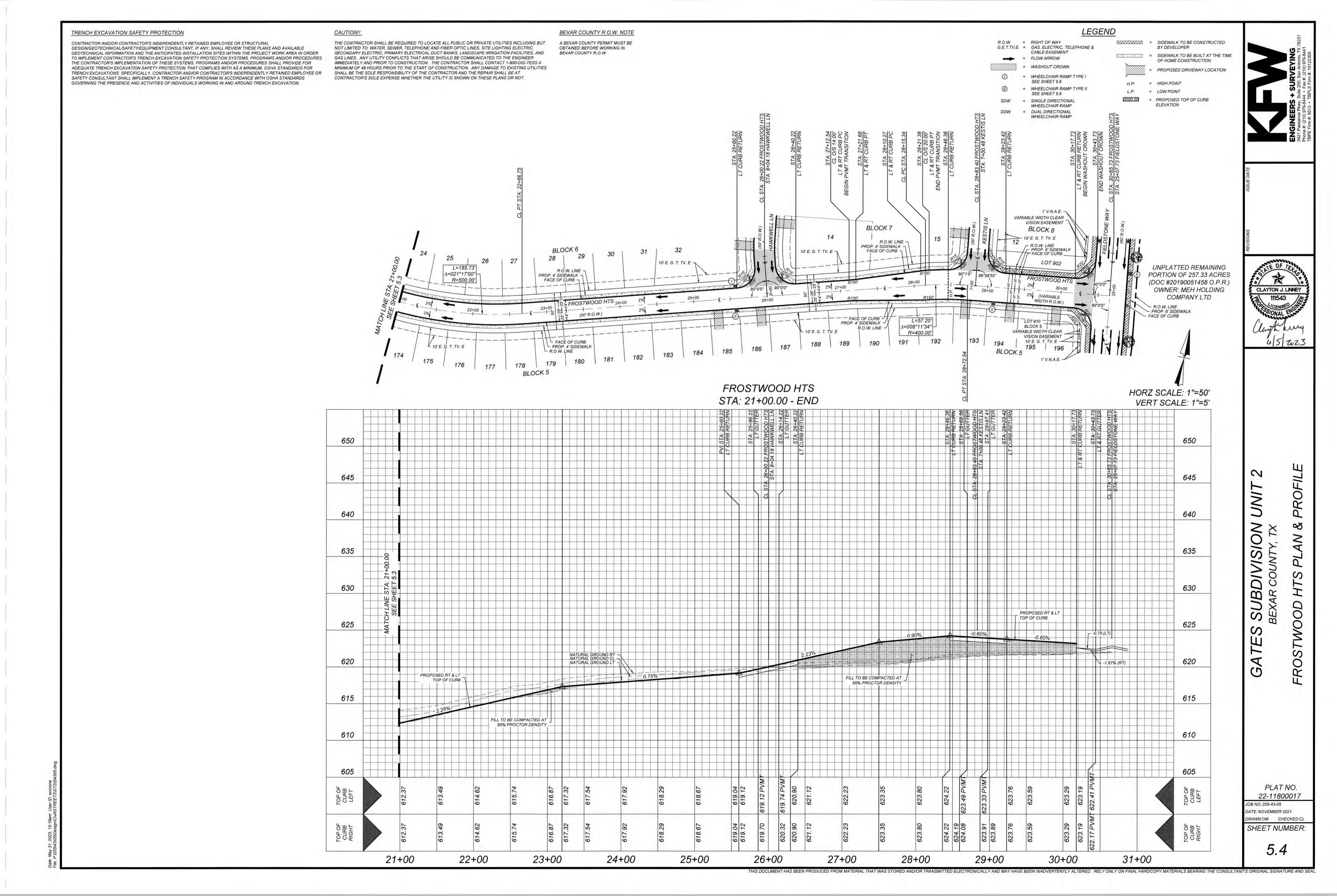
7+00

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6+00

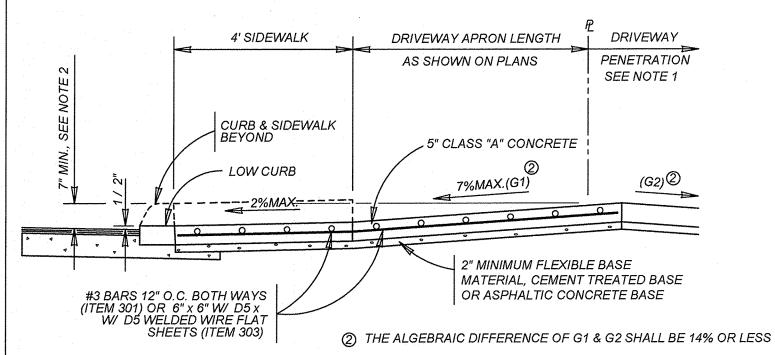
Feb 08, 2023, 11:02am UserID: ecorona >:\205\43\05\Design\Civi\STREETS\ST205430;





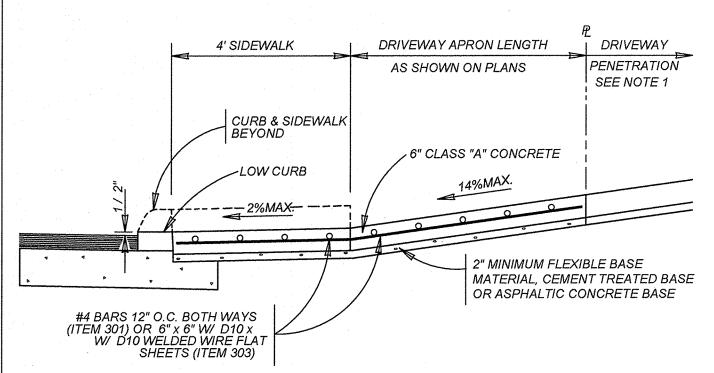
#### TYPICAL RESIDENTIAL DRIVEWAY SECTION

WITH SIDEWALK ABUTTING CURB ITEM 503.1



#### TYPICAL RESIDENTIAL DRIVEWAY SECTION

WHERE PROPERTY IS LOWER THAN STREET & SIDEWALK IS ABUTTING CURB



#### TYPICAL COMMERCIAL DRIVEWAY SECTION

WITH SIDEWALK ABUTTING CURB

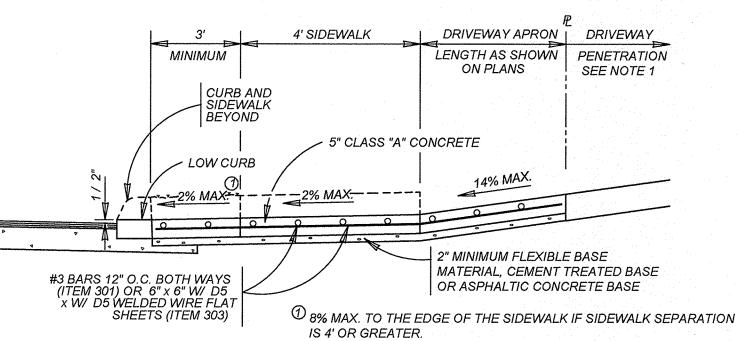
#### CONCRETE DRIVEWAY NOTES

- 1. DRIVEWAY PENETRATION REFERS TO A PORTION OF THE DRIVEWAY THAT MAY BE NECESSARY TO RECONSTRUCT WITHIN PRIVATE PROPERTY TO COMPLY WITH A MAXIMUM DRIVEWAY SLOPE. THIS PORTION OF THE DRIVEWAY SHALL BE PAID FOR UNDER THE FOLLOWING ITEMS AS MAY APPLY:

  A.) CONCRETE DRIVEWAY PAID FOR UNDER ITEM NO. 503.1 OR 503.2.
- B.) ASPHALTIC CONCRETE DRIVEWAY PAID FOR UNDER ITEM NO. 503.4 AND SHALL INCLUDE A MINIMUM OF 1" ASPHALT TYPE 'D' & 6" FLEXIBLE BASE C.) GRAVEL DRIVEWAY PAID FOR UNDER ITEM NO. 503.5 AND SHALL INCLUDE A MINIMUM OF 6" FLEXIBLE BASE
- 2. 7" MINIMUM HEIGHT WILL NOT NECESSARILY OCCUR AT THE PROPERTY LINE. IT MAY OCCUR WITHIN THE RIGHT OF WAY OR WITHIN THE DRIVEWAY PENETRATION ON PRIVATE PROPERTY.
- 3. THE PROPOSED DRIVEWAY SHOULD MATCH THE EXISTING WIDTH AT THE PROPERTY LINE BUT UNLESS AUTHORIZED BY THE CITY TRAFFIC ENGINEER, THE WIDTH SHALL BE WITHIN THE FOLLOWING VALUES:

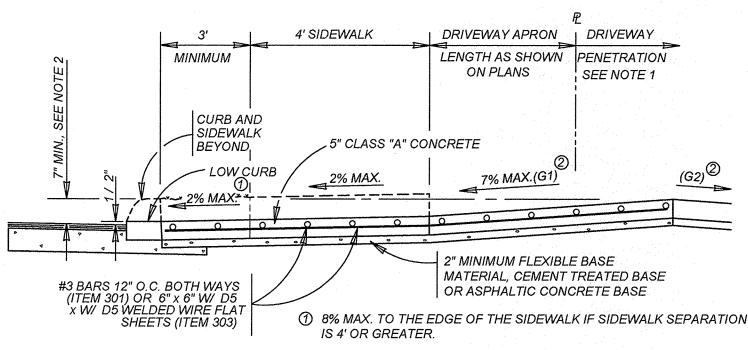
TYPE	MINIMUM	MAXIMUM
RESIDENTIAL	10'	20'
COMMERCIAL - ONE WAY	12'	20'
COMMERCIAL - TWO WAY	24'	30'

- 4. FOR LOCAL TYPE "A" STREETS, SIDEWALK SHALL HAVE A MINIMUM WIDTH OF 4' AND IF SEPARATED FROM THE CURB, THE SIDEWALK SHALL BE LOCATED A MINIMUM OF 3' FROM THE BACK OF CURB.
- 5. FOR OTHER THAN LOCAL TYPE "A" STREETS, THE SIDEWALK SHALL HAVE A MINIMUM WIDTH OF 4' AND SEPARATED A MINIMUM OF 3' FROM THE BACK OF CURB OR, AS AN OPTION, THE SIDEWALK SHALL HAVE A MINIMUM WIDTH OF 6' WHEN LOCATED AT THE BACK OF CURB.
- 6. DUMMY JOINTS PARALLEL TO THE CURB SHALL BE PLACED WHERE THE SIDEWALK MEETS THE DRIVEWAY. DUMMY JOINTS PERPENDICULAR TO THE CURB, AND WITHIN THE BOUNDARIES OF THE PARALLEL DUMMY JOINTS, SHALL BE PLACED AT INTERVALS EQUAL TO THE WIDTH OF THE SIDEWALK.
- 7. A MINIMUM OF TWO ROUND AND SMOOTH DOWEL BARS 3/8" IN DIAMETER AND 18" IN LENGTH SHALL BE SPACED 18" APART AT EACH EXPANSION JOINT.
- 8. SIDEWALK RAMP LENGTHS SHALL BE OF SUFFICIENT LENGTH TO MAINTAIN 8.33% (1:12) MAXIMUM SLOPE. WHERE SIDEWALKS CROSS DRIVEWAYS, SIDEWALK CROSS SLOPE SHALL NOT EXCEED 2%.
- 9. SIDEWALK RAMP SURFACE SHALL BE BRUSH FINISHED.



#### TYPICAL RESIDENTIAL DRIVEWAY SECTION

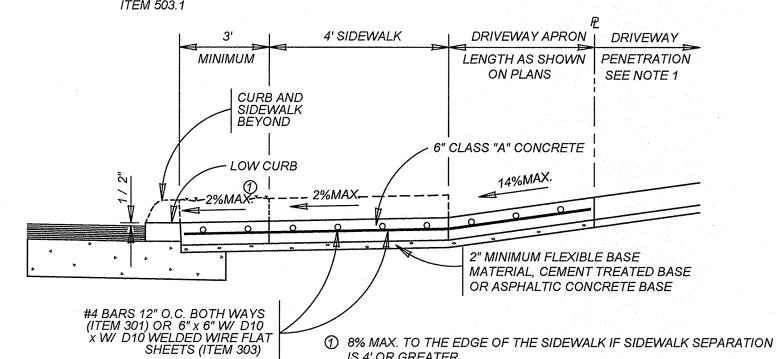
WITH SIDEWALK SEPARATED FROM CURB
ITEM 503.1



② THE ALGEBRAIC DIFFERENCE OF G1 & G2 SHALL BE 14% OR LESS

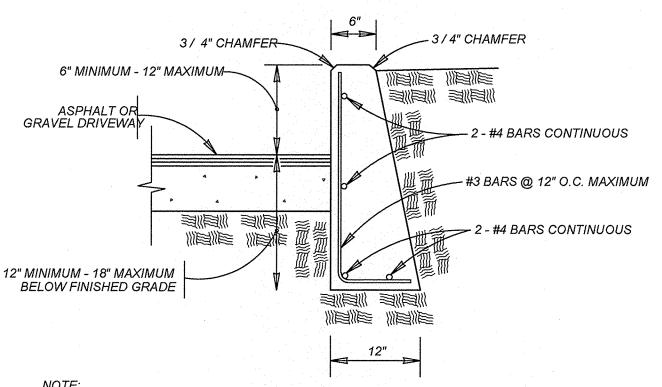
#### TYPICAL RESIDENTIAL DRIVEWAY SECTION

WHERE PROPERTY IS LOWER THAN STREET & SIDEWALK IS SEPARATED FROM CURB ITEM 503.1



#### TYPICAL COMMERCIAL DRIVEWAY SECTION

WITH SIDEWALK SEPARATED FROM CURB ITEM 503.2



NOTE:

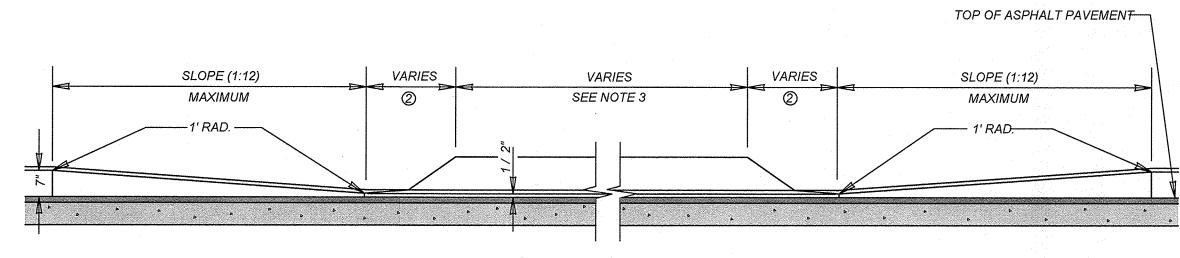
1. COST OF REINFORCEMENT TO BE INCLUDED IN UNIT COST OF ITEM 307.1.

2. CONCRETE RETAINING WALL COMBINATION TYPE SHALL BE USED FOR

#### DRIVEWAY - CONCRETE RETAINING WALL

ON COMPACTED SUBGRADE ITEM 307.1

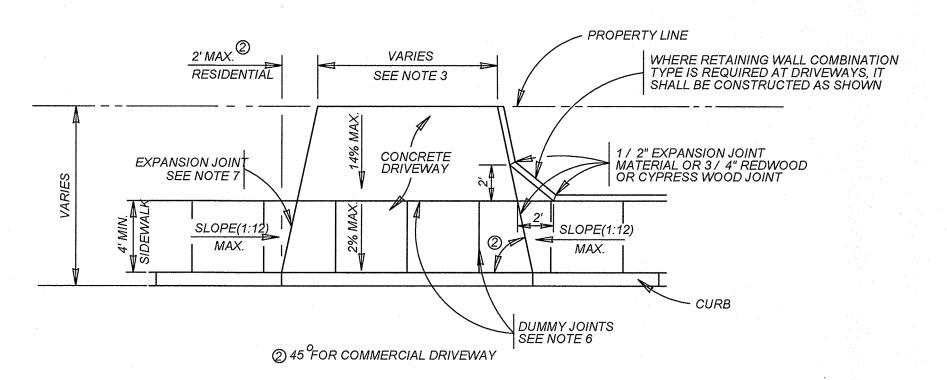
CONCRETE DRIVEWAYS.



② RESIDENTIAL : 2' MAXIMUM; COMMERCIAL: SEE PLAN VIEW

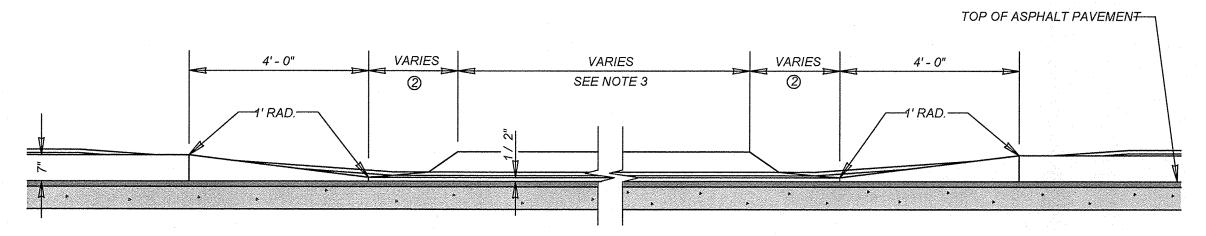
#### CURB PROFILE AT DRIVEWAY

WITH SIDEWALK ABUTTING CURB



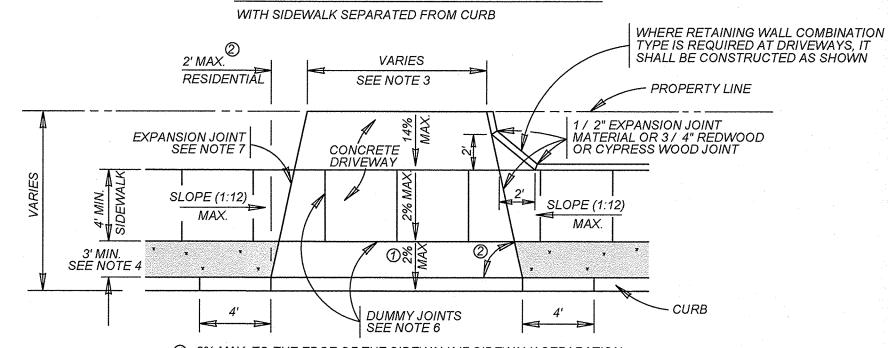
#### TYPICAL DRIVEWAY PLAN VIEW

WITH SIDEWALK ABUTTING CURB



② RESIDENTIAL: 2' MAXIMUM;

#### CURB PROFILE AT DRIVEWAY



8% MAX. TO THE EDGE OF THE SIDEWALK IF SIDEWALK SEPARATION
 IS 4' OR GREATER.

② 45 FOR COMMERCIAL DRIVEWAY

TYPICAL DRIVEWAY PLAN VIEW

WITH SIDEWALK SEPARATED FROM CURB

MAY 2009

CITY OF SAN ANTONIO

CAPITAL IMPROVEMENTS MANAGEMENT SERVICES DEPARTMENT

CONCRETE DRIVEWAY STANDARDS

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PLAT NO. 22-11800017 JOB NO. 205-43-05 DATE: NOVEMBER 2021

SUBDIVISION SAN ANTONIO. TX

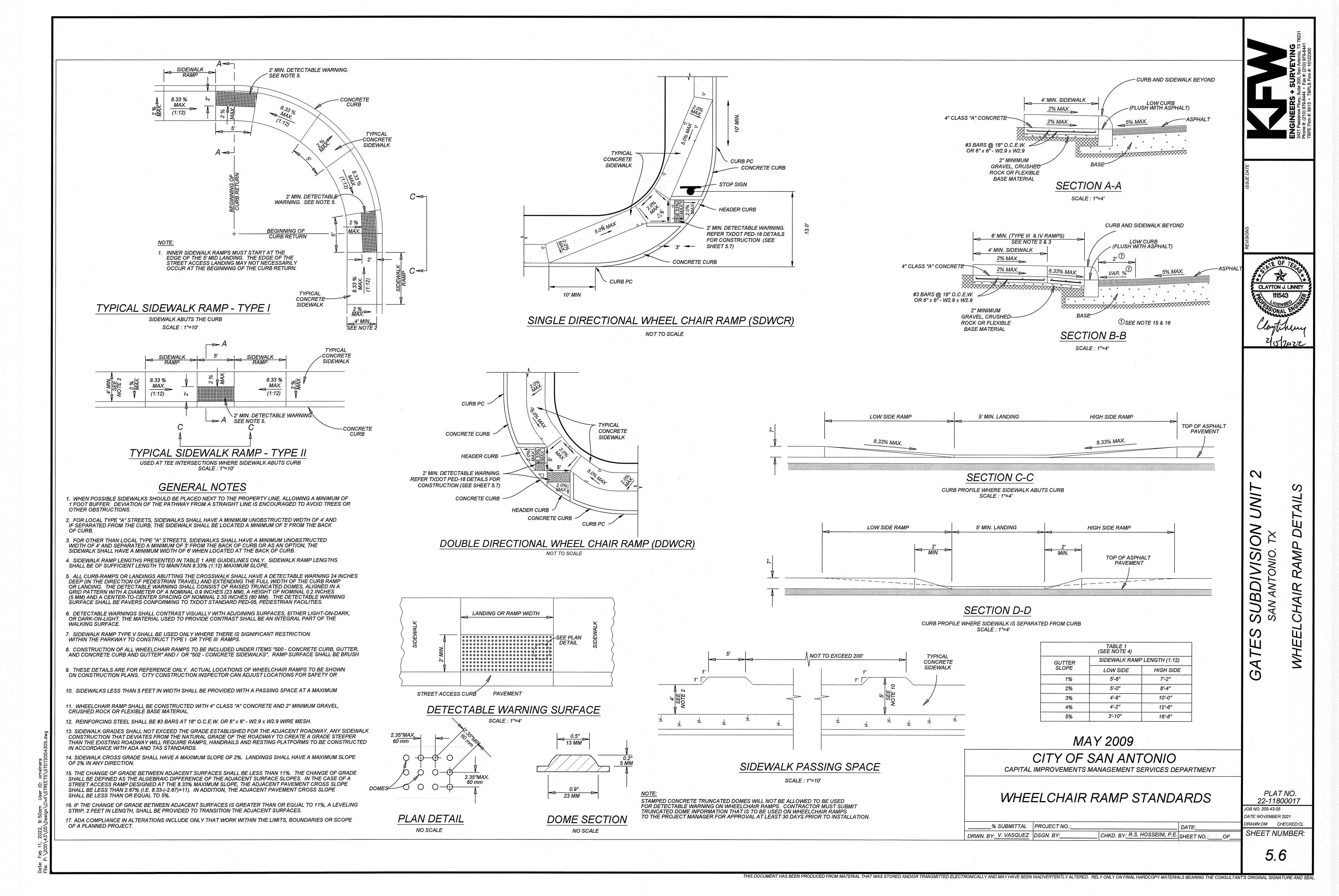
CLAYTON J. LINNEY

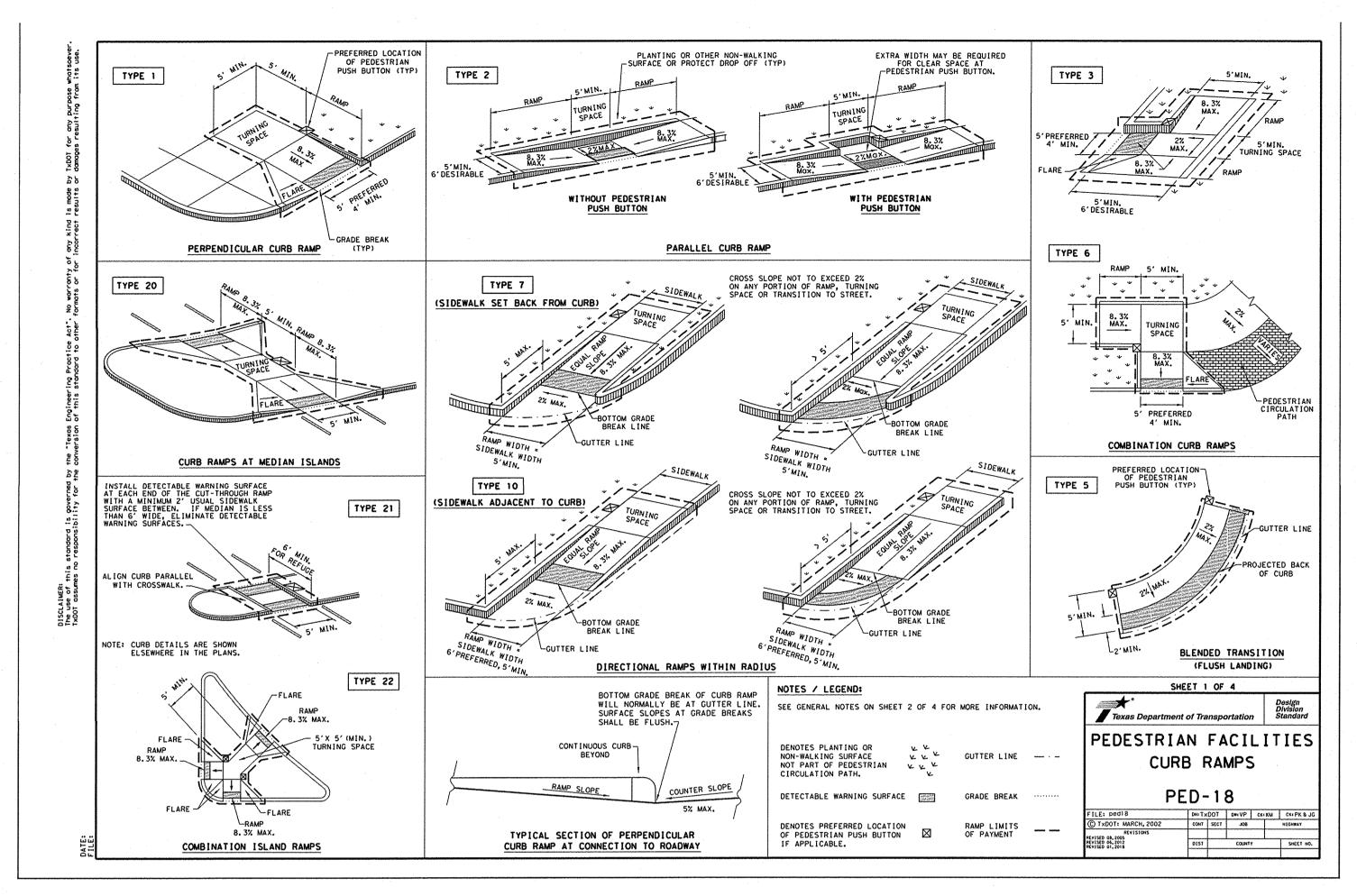
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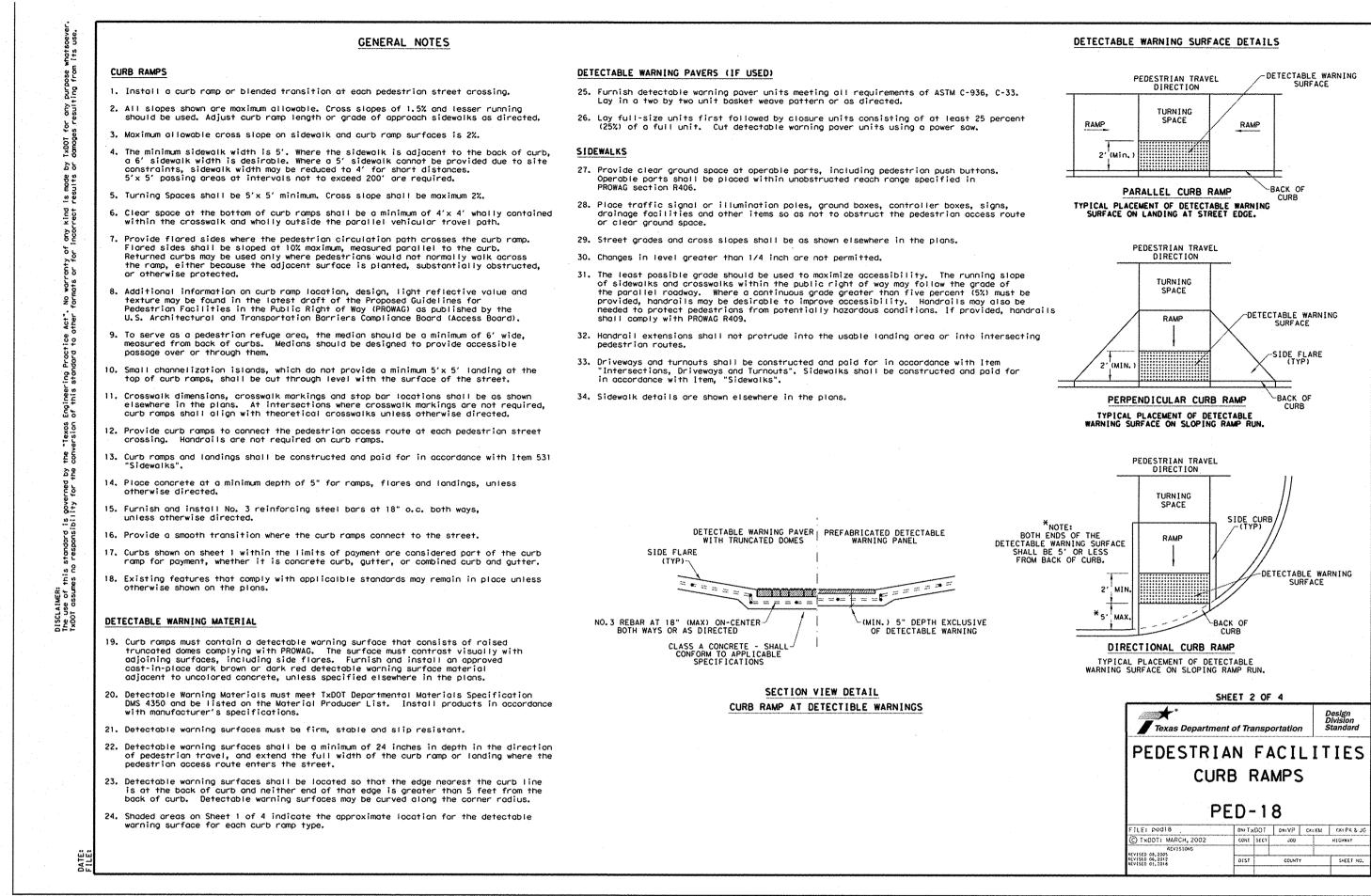
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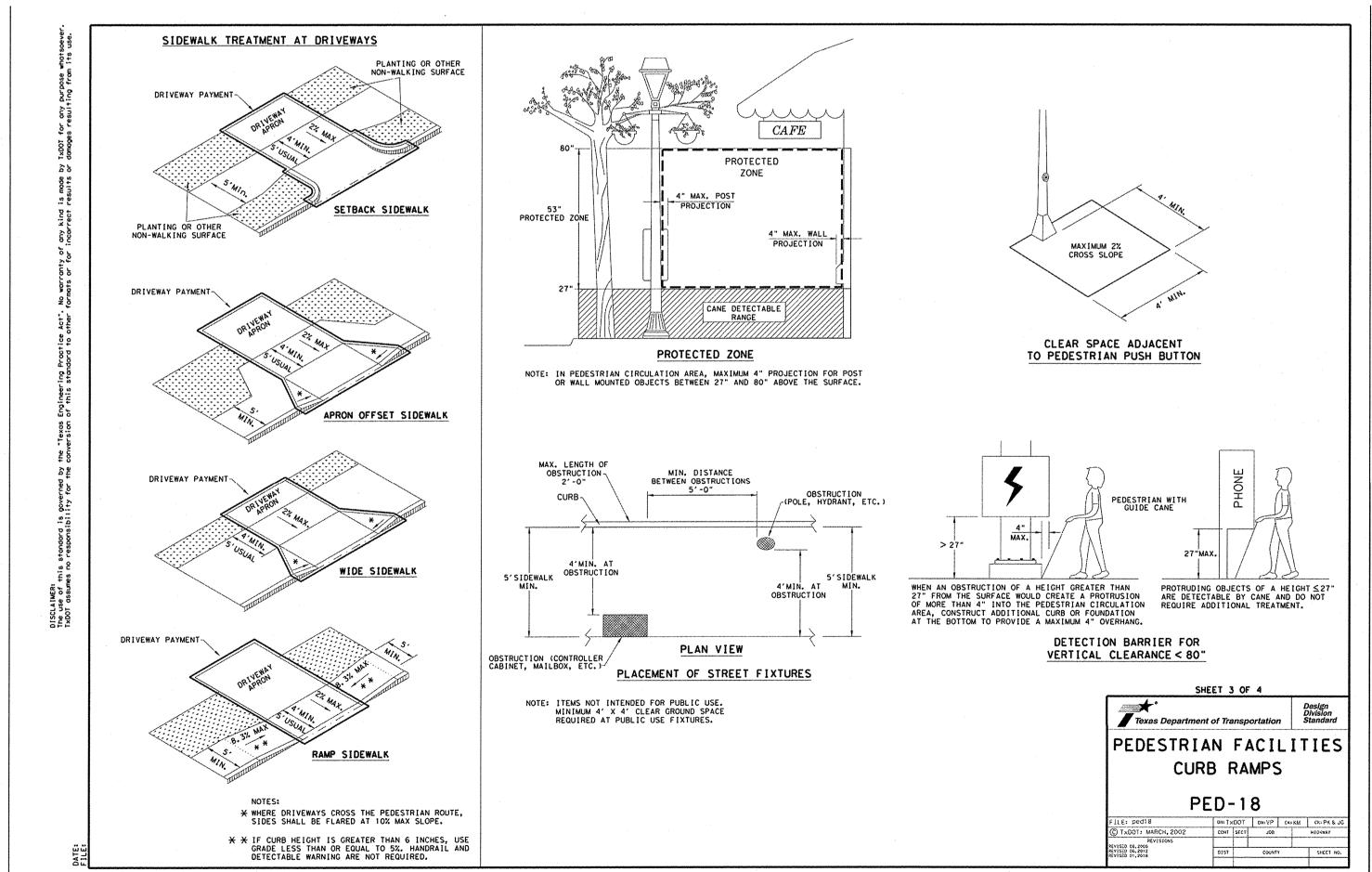
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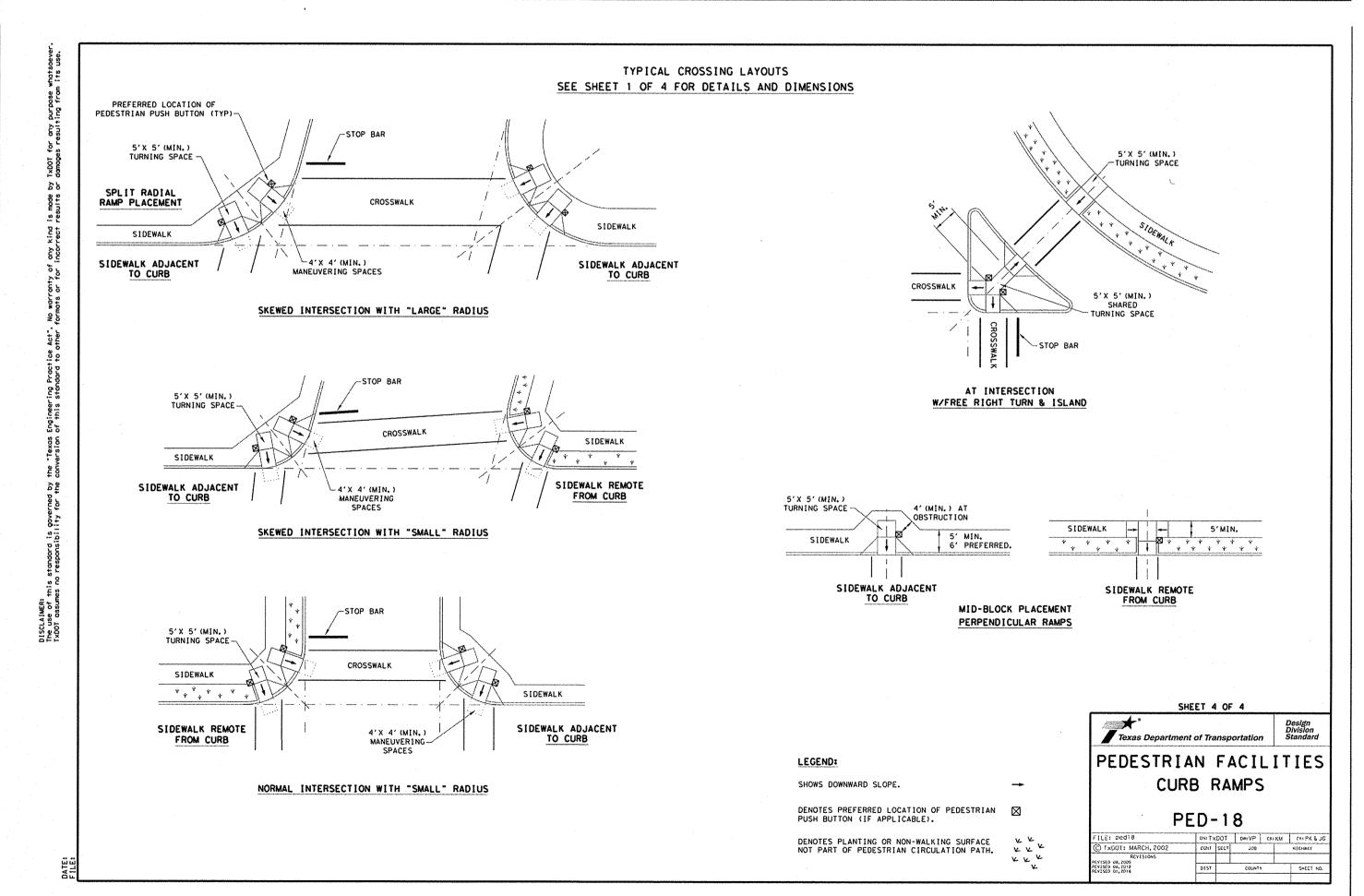
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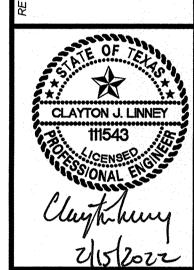






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3421 Paesanos Pkwy, Suite 200, San Antonio, TX 782
Phone #: (210) 979-8444 • Fax #: (210) 979-8441
TBPE Firm #: 9513 • TBPLS Firm #: 10122300

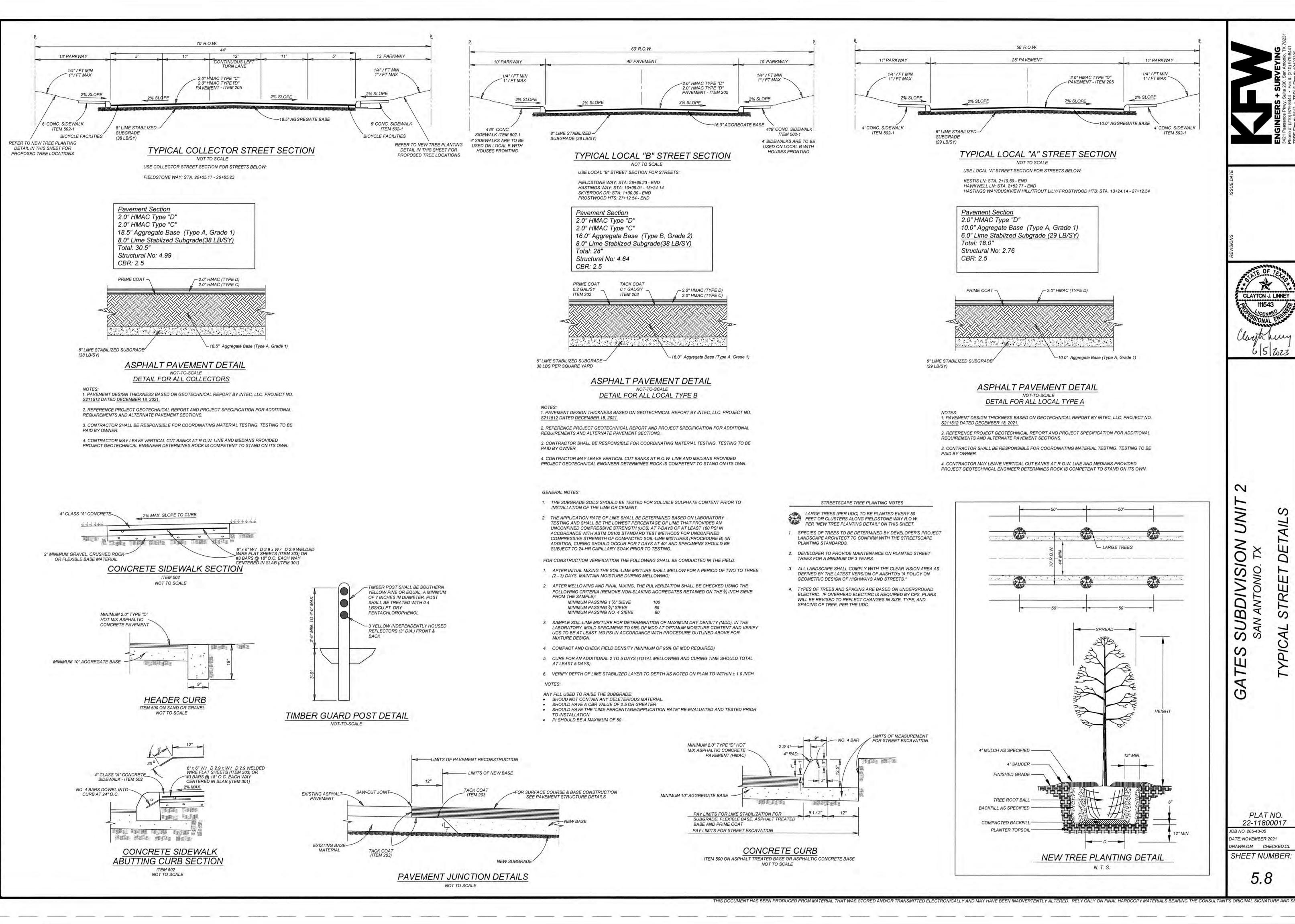


GATES SUBDIVISION UNIT 2

SAN ANTONIO. TX

TXDOT PEDESTRIAN CURB RAMP DETAIL

PLAT NO. 22-11800017 JOB NO. 205-43-05 DATE: NOVEMBER 2021 DRAWN:OM CHECKED:CL SHEET NUMBER.



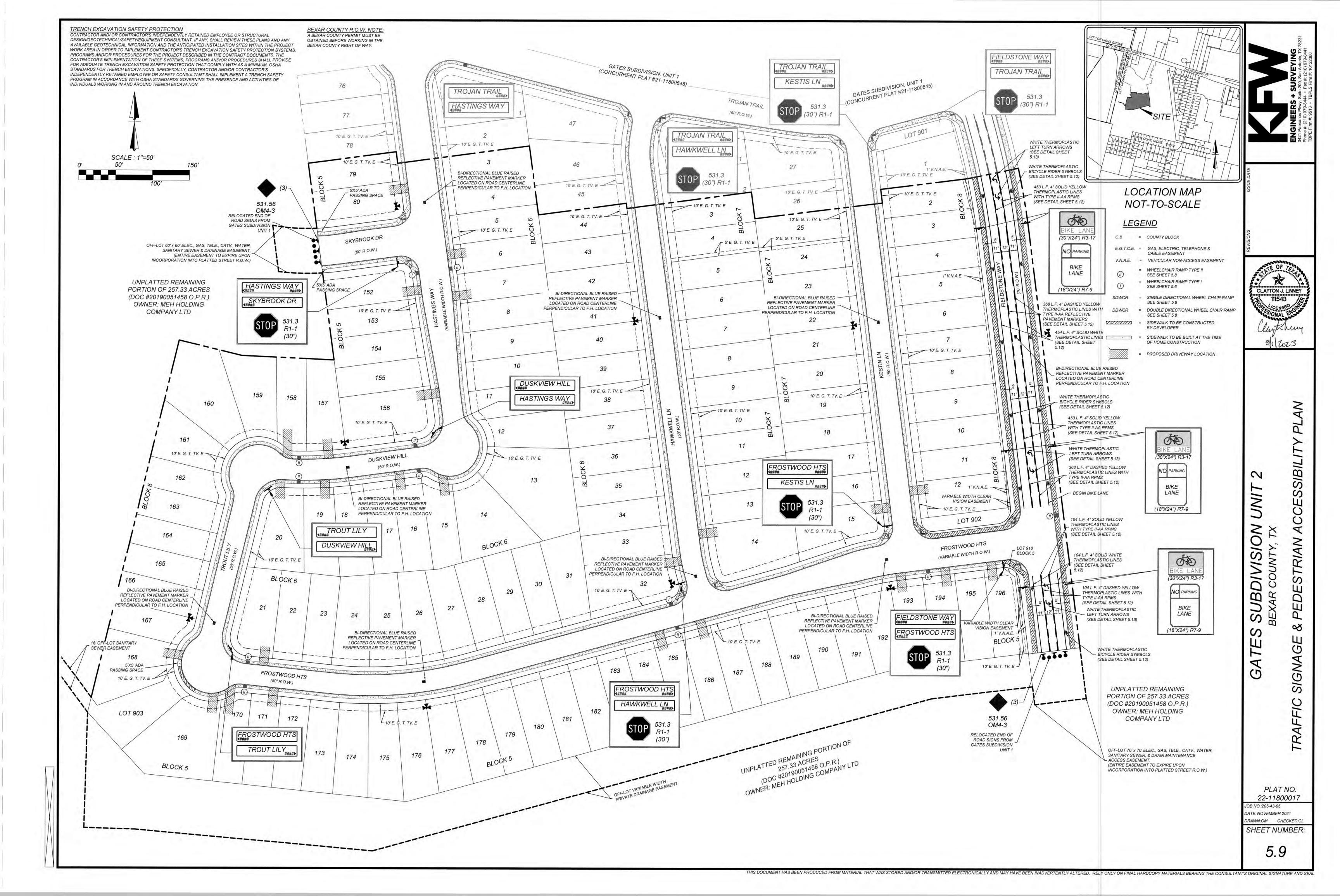
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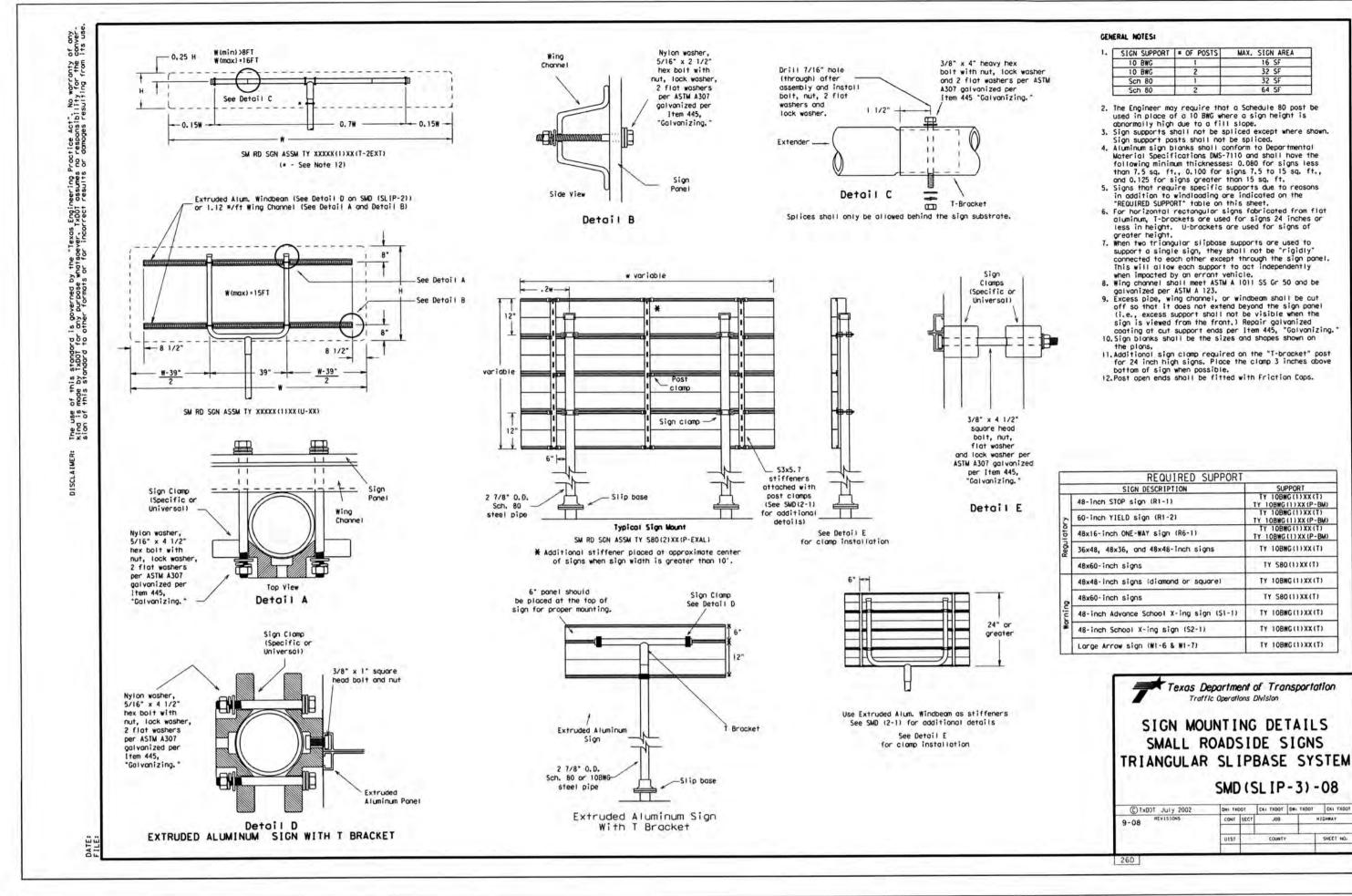
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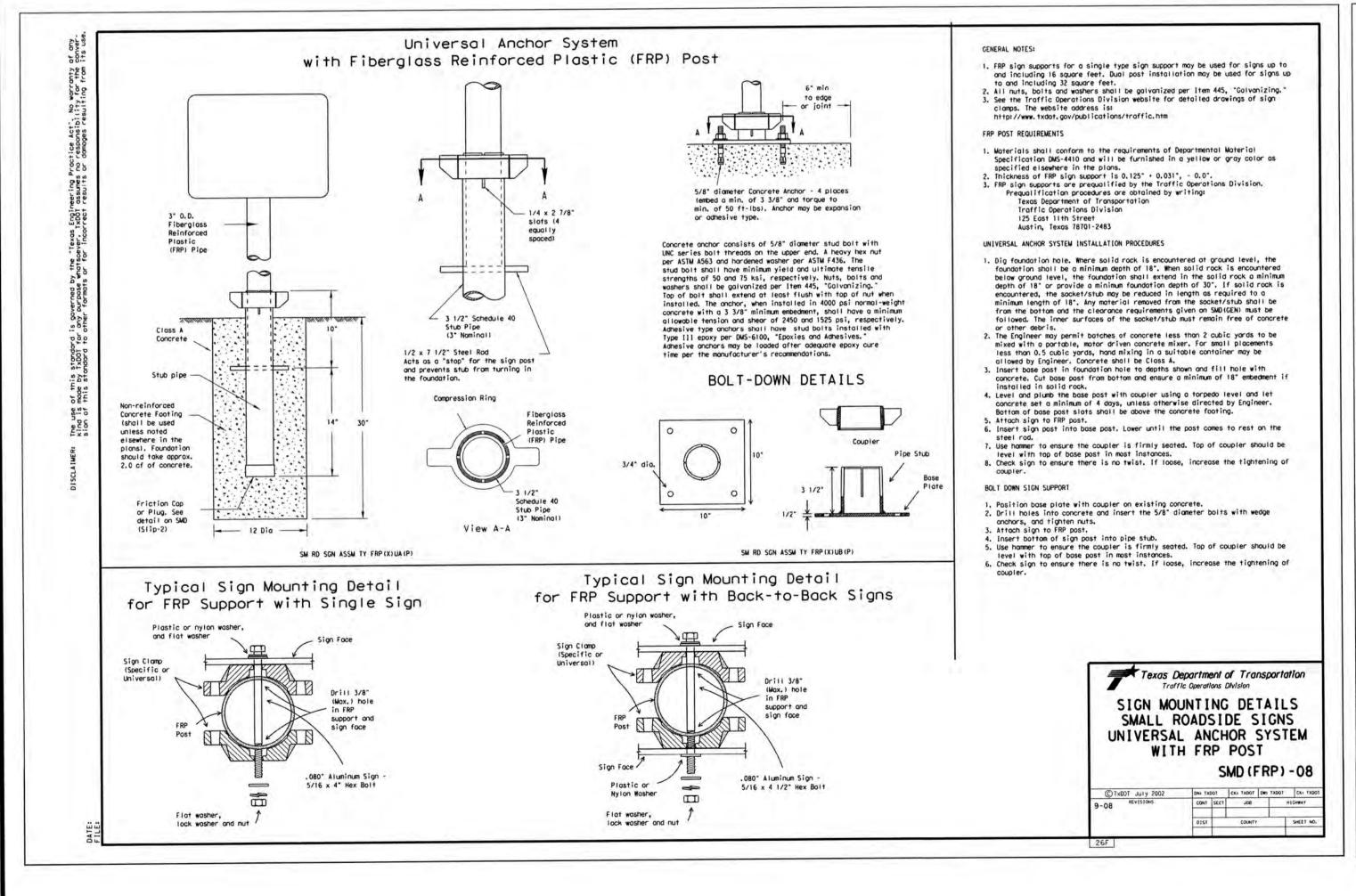
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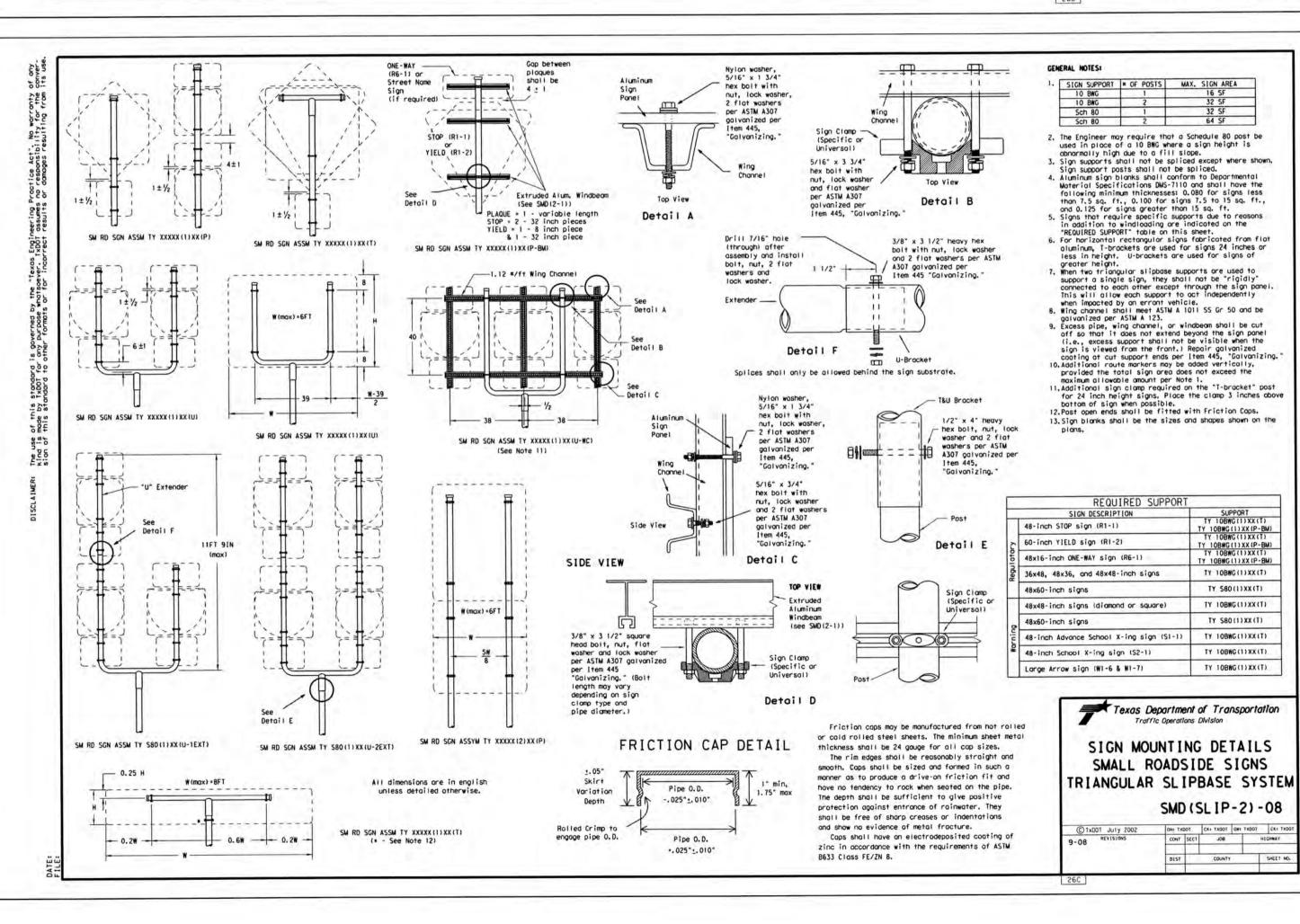
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CLAYTON J. LINNEY

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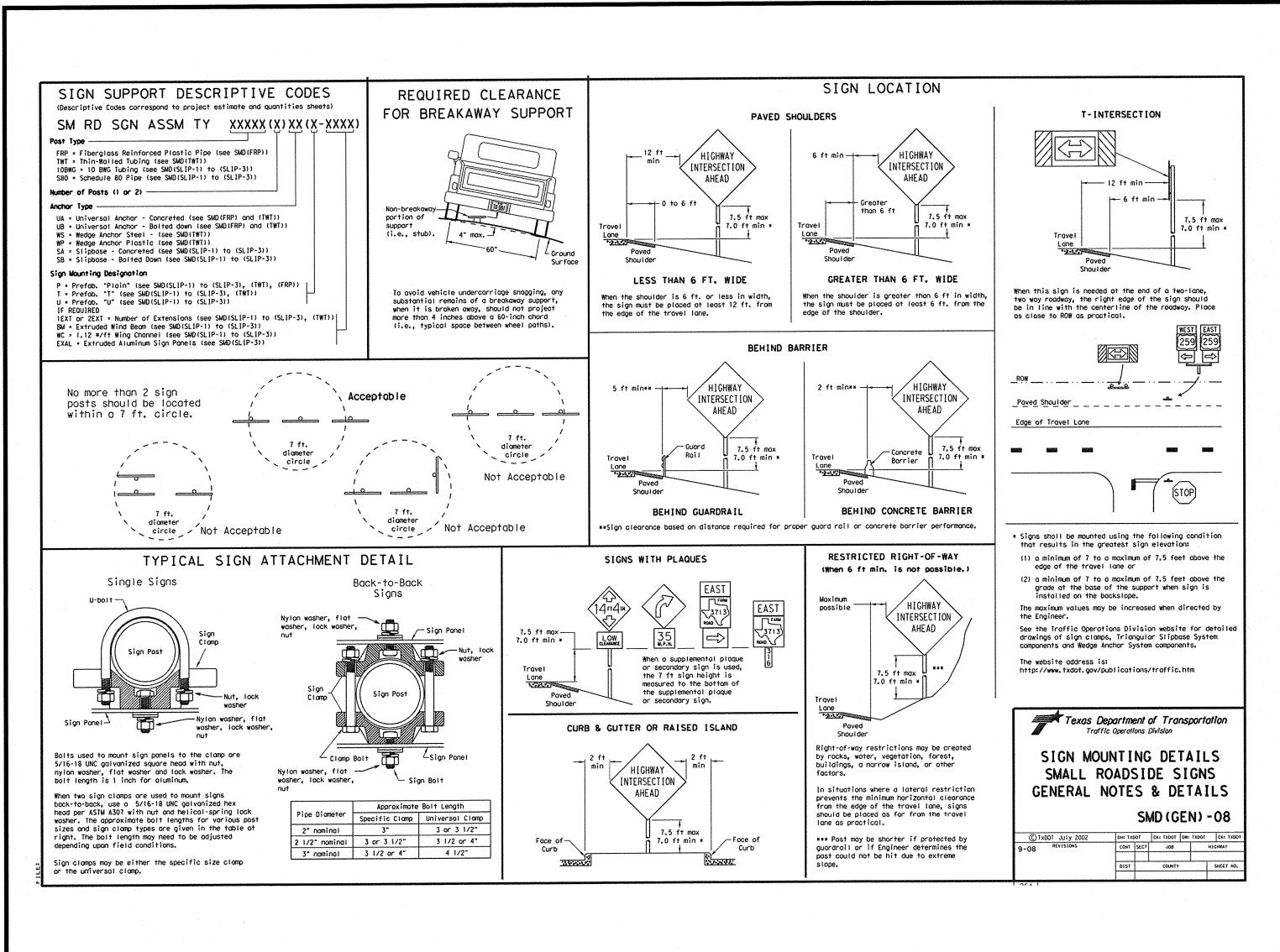
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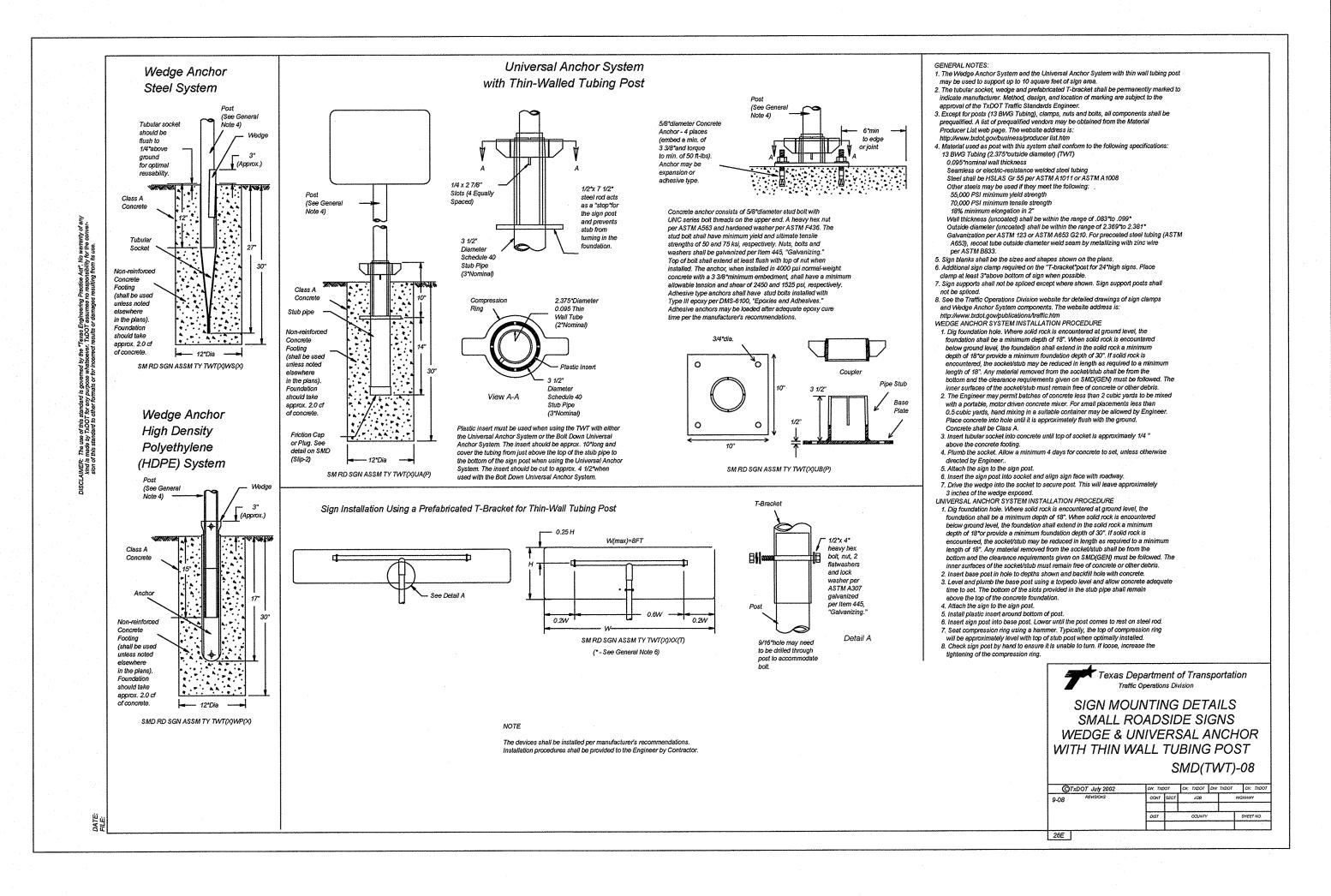
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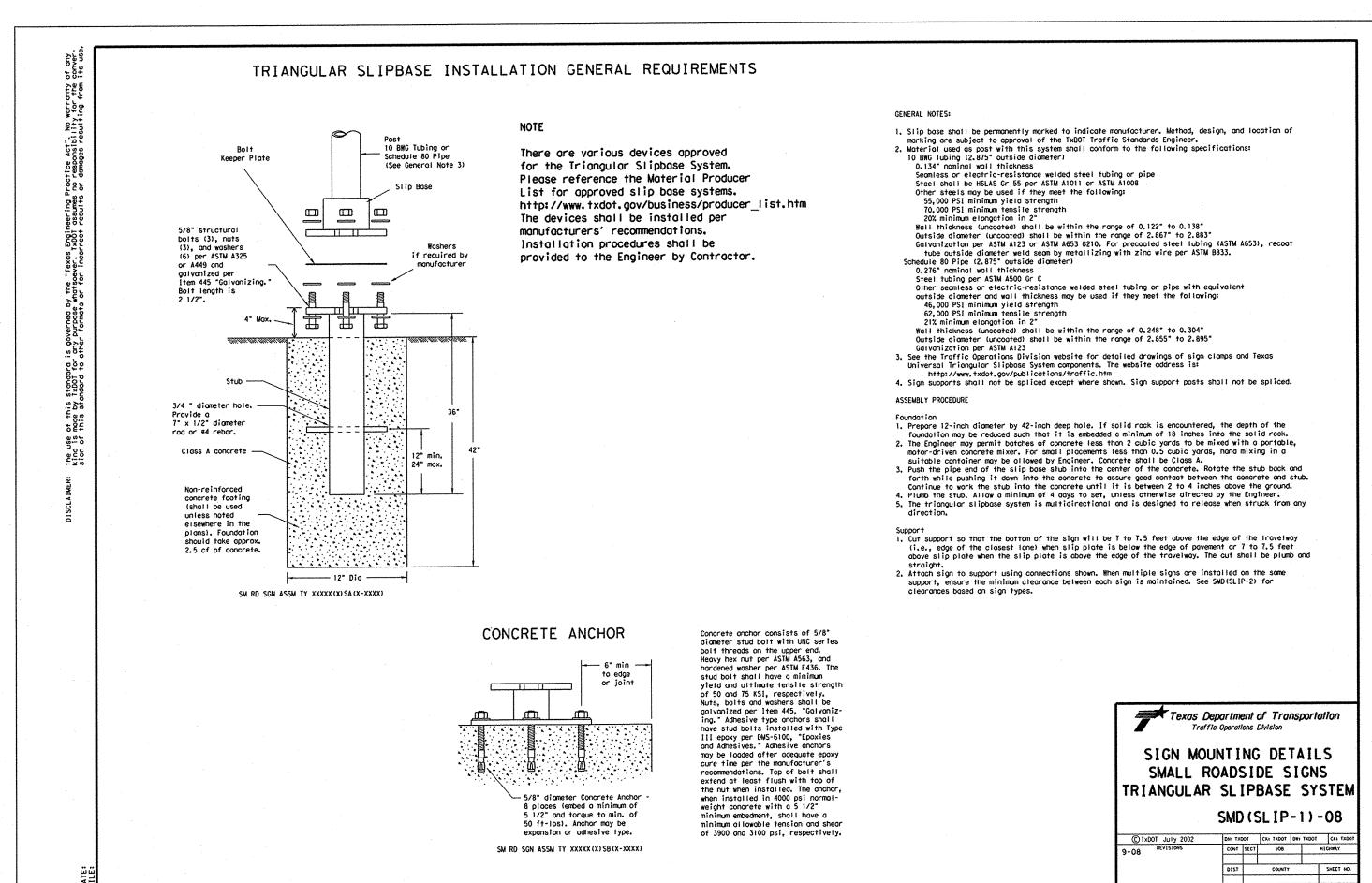
GATES SUBDIVISION UNIT 2

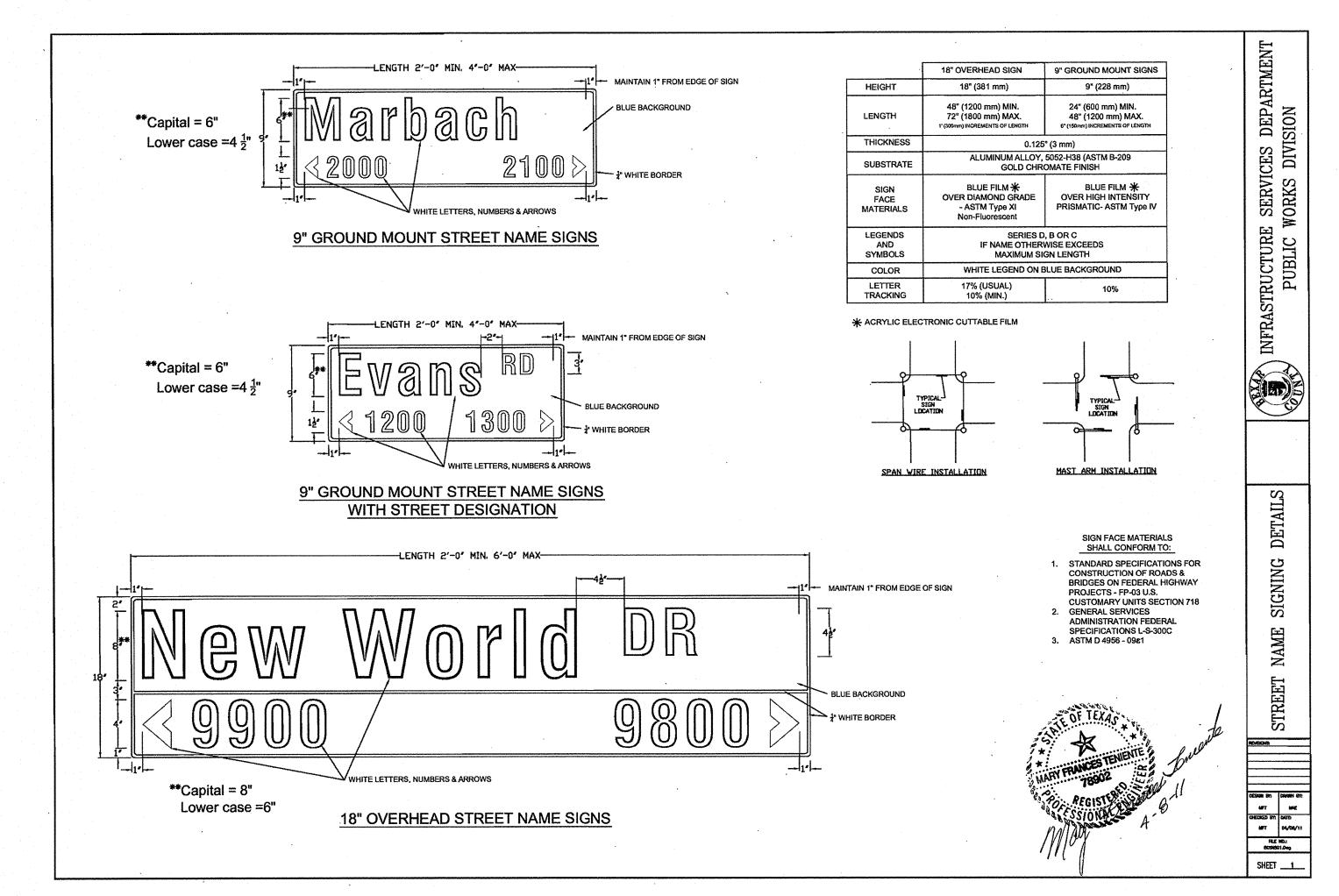
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GATES SUBDIVISION UNIT 2

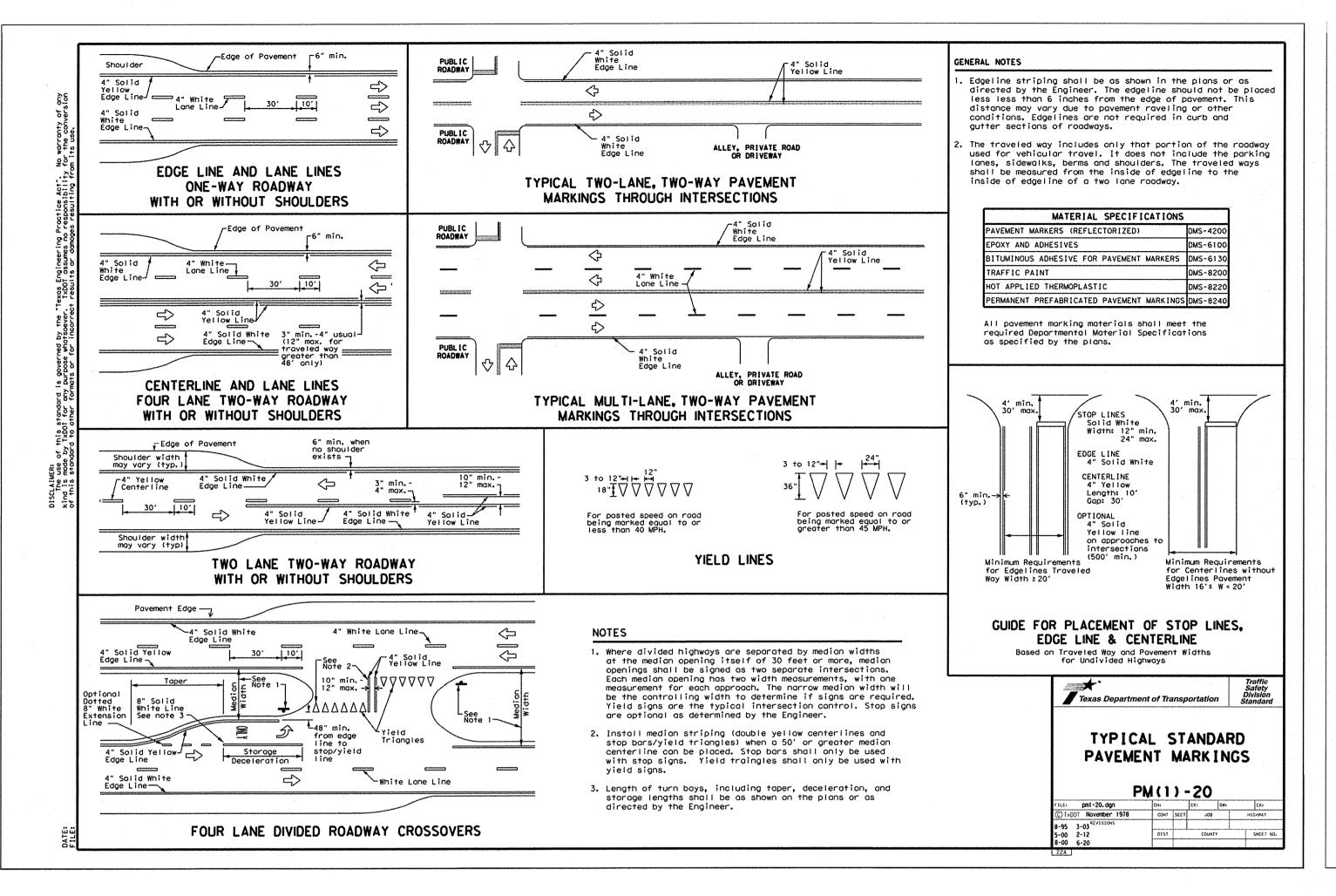
SAN ANTONIO. TX

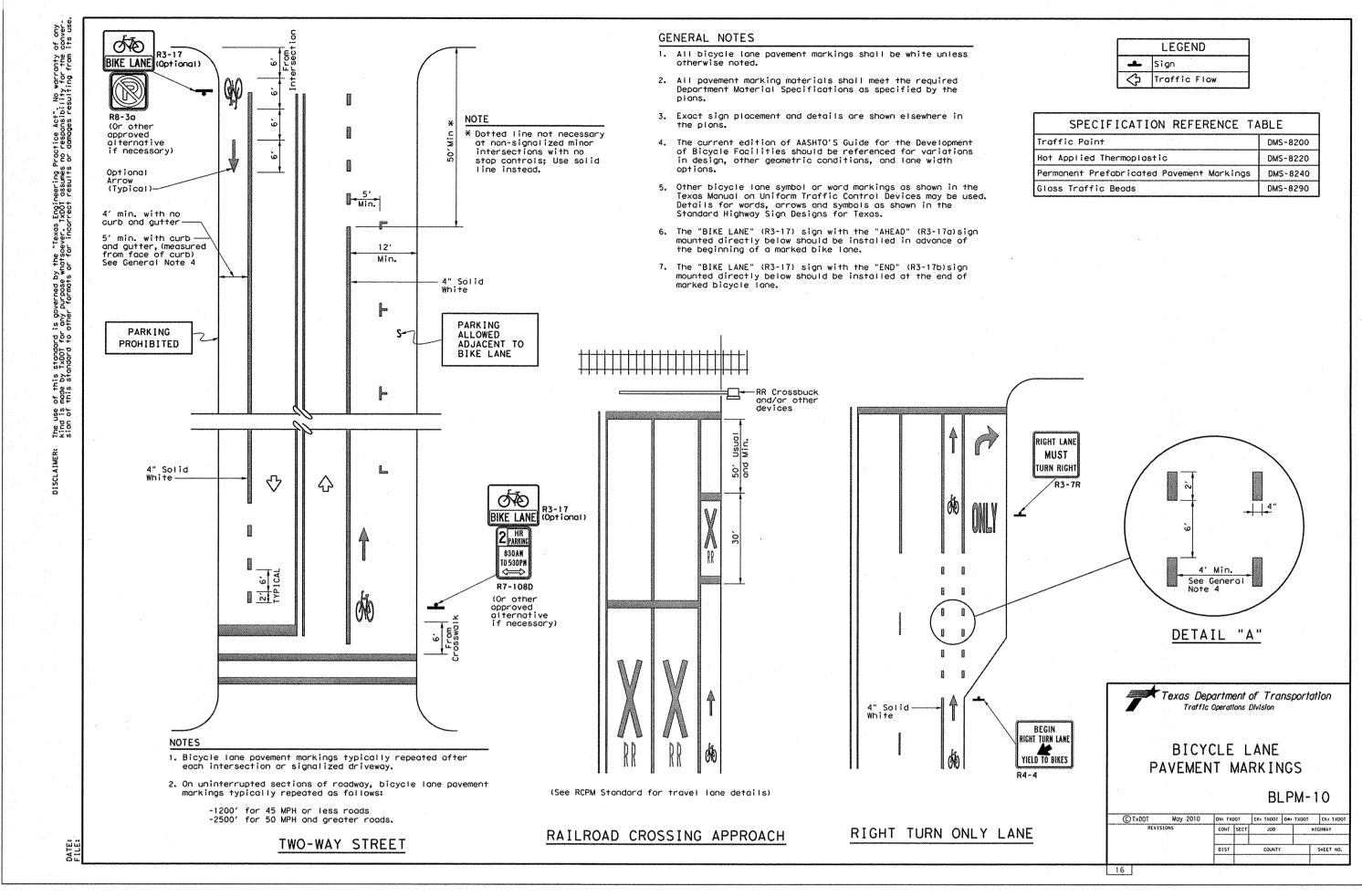
RAFFIC SIGNAGE DETAILS (SHEET 2 OF 2)

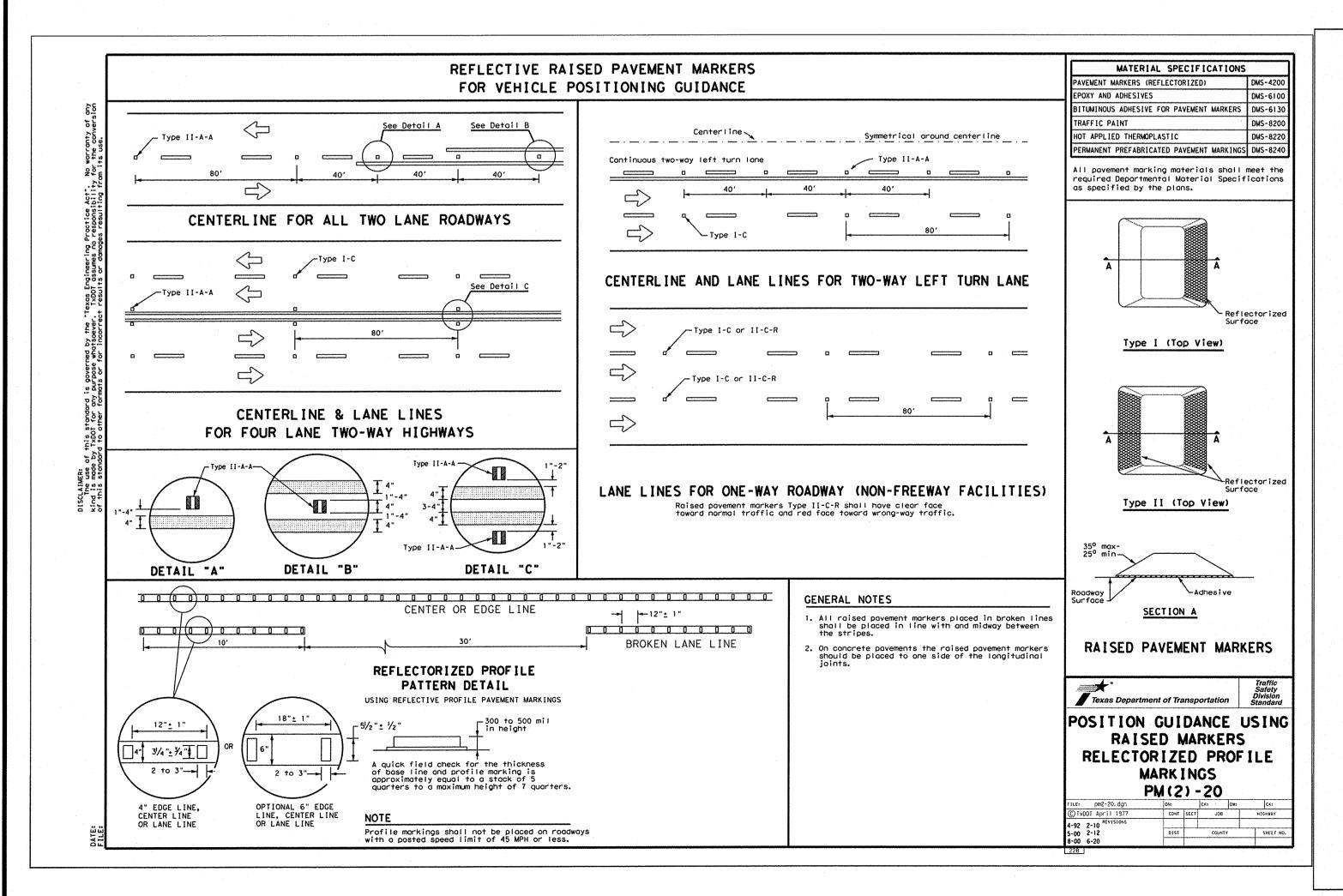
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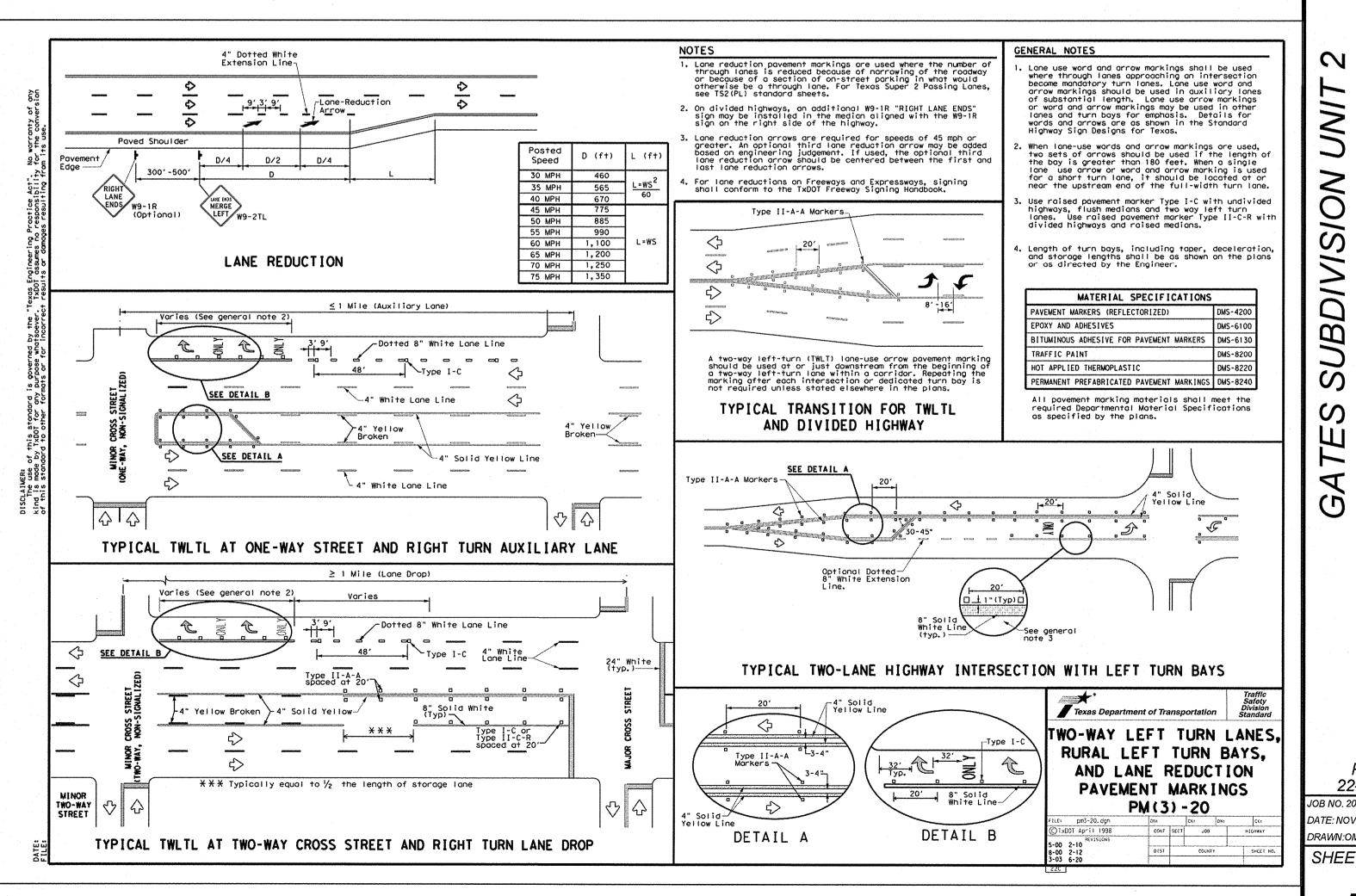
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SHEET NUMBER:



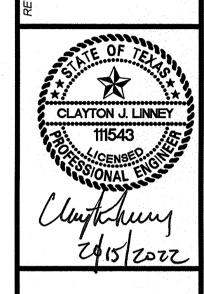






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SAN ANTONIO. TX PAVEMENT MARKING DETAILS (SHEET 1 OF 2)

PLAT NO. 22-11800017 NO. 205-43-05 E: NOVEMBER 2021

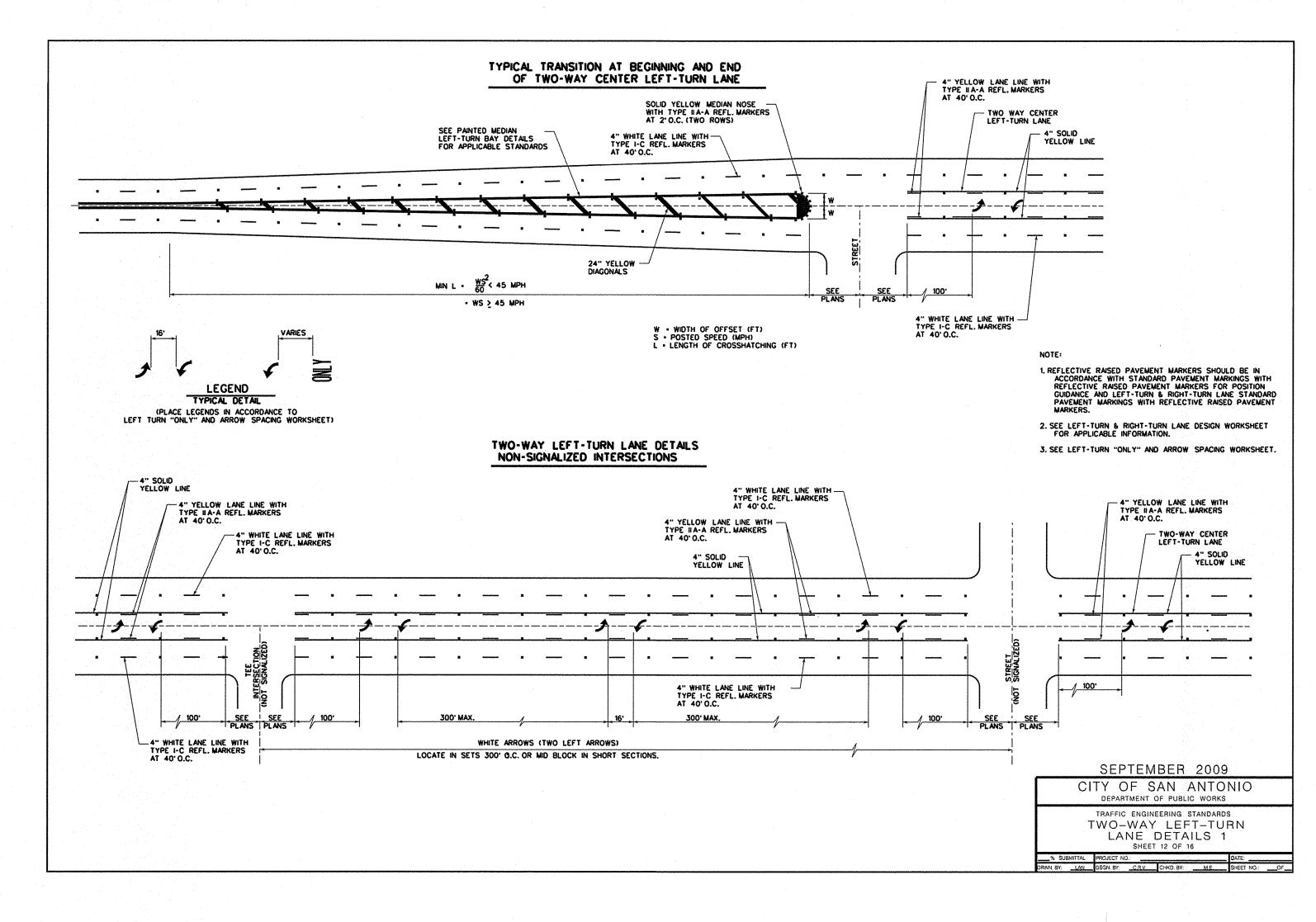
JOB NO. 205-43-05

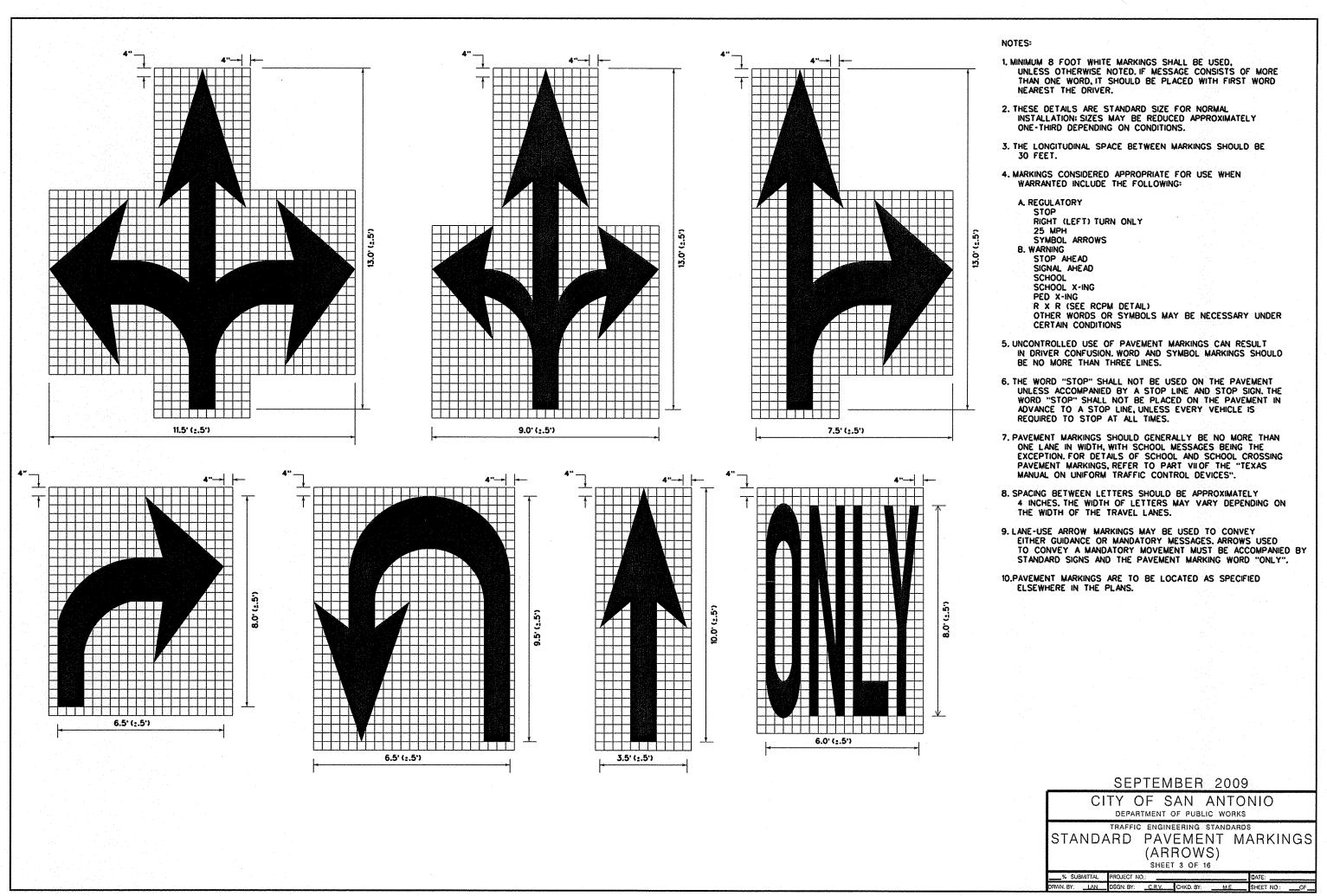
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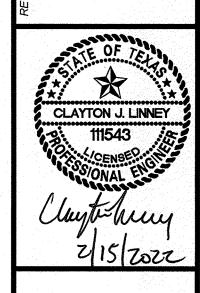
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THIS DOCUMENT HAS BEEN PRODUCED FROM MATERIAL THAT WAS STORED AND/OR TRANSMITTED ELECTRONICALLY AND MAY HAVE BEEN INADVERTENTLY ALTERED. RELY ONLY ON FINAL HARDCOPY MATERIALS BEARING THE CONSULTANT'S ORIGINAL SIGNATURE AND SEAL.





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GATES SUBDIVISION UNIT 2 SAN ANTONIO, TX

PLAT NO. 22-11800017 JOB NO. 205-43-05

DATE: NOVEMBER 2021 DRAWN:OM CHECKED:CL SHEET NUMBER:

#### GENERAL SEWER NOTES (REVISED JULY 2017)

1. All materials and construction procedures within the scope of this contract shall be approved by the San Antonio

Water System (SAWS) and comply with the Plans, Specifications, General
Conditions and with the following as applicable:

- A. Current Texas Commission on Environmental Quality (TCEQ) "Design Criteria for Domestic Waste water System", Texas Administrative Code (TAC) Title 30 Part 1 Chapter 217 and "Public Drinking Water", TAC Title 30 Part 1 Chapter 200
- Water", TAC Title 30 Part 1 Chapter 290.

  B. Current TXDOT "Standard Specifications for Construction of Highways, Streets and Drainage".

  C. Current "San Antonio Water System Standard Specifications for Water and Sanitary Sewer
- Construction .

  D. Current City of San Antonio "Standard Specifications for Public Works Construction".

  E. Current City of San Antonio "Utility Excavation Criteria Manual" (UECM).

2. The contractor shall not proceed with any pipe installation work until they obtain a copy of the approved Counter Permit or General Construction Permit (GCP) from the consultant and has been notified by SAWS Construction I nspection Division to proceed with the work and has arranged a meeting with the inspector and consultant for the work requirements. Work completed by the contractor without an approved Counter Permit and/or a GCP will be

 The Contractor shall obtain the SAWS Standard Details from the SAWS website, http://www.saws.org/business\_center/specs. Unless otherwise noted within the design plans.

subject to removal and replacement at the expense of the contractors and/or the developer.

4. The Contractor is to make arrangements with the SAWS Construction Inspection Division at (210) 233-2973, on notification procedures that will be used to notify affected home residents and/or property owners 48 hours prior to beginning any work.

5. Location and depth of existing utilities and service laterals shown on the plans are understood to be approximate. Actual locations and depths must be field verified by the Contractor at least 1 week prior to construction. It shall be the Contractor's responsibility to locate utility service lines as required for construction and to protect them during construction at no cost to SAWS.

6. The Contractor shall verify the exact location of underground utilities and drainage structures at least 1-2 weeks prior to construction whether shown on plans or not. Please allow up to 7 business days for locates requesting pipe location markers on SAWS facilities. The following contact information are supplied for verification purposes:

SAWS Utility Locates: http://www.saws.org/Service/Locates

COSA Drainage (210) 207-0724 or (210) 207-6026 COSA Traffic Signal Operations (210) 206-8480

COSA Traffic Signal Damages (210) 207-3951
Texas State Wide One Call Locator 1-800-545-6005 or 811

7. The Contractor shall be responsible for restoring existing fences, curbs, streets, driveways, sidewalks, landscaping and structures to its original or better condition if damages are made as a result of the project's construction.

All work in Texas Department of Transportation (TxDOT) and/or Bexar County right-of-way shall be done in accordance with respective construction specifications and permit requirements.

The Contractor shall comply with City of San Antonio or other governing municipality's tree ordinances when excavating near trees.

10. The Contractor shall not place any waste materials in the 100-year Flood Plain without first obtaining an approved Flood Plain Permit.

11. Holiday Work: Contractors will not be allowed to perform SAWS work on SAWS recognized holidays. Request should be sent to constworkreq@saws.org. Weekend Work: Contractors are required to notify the SAWS Inspection Construction Department 48 hours in advance to request weekend work. Request should be sent to constworkreq@saws.org. Any and all SAWS utility work installed without holiday/weekend approval will be subject to be uncovered for proper inspection.

12. Compaction note (Item 804): The contractor shall be responsible for meeting the compaction requirements on all trench backfill and for paying for the tests performed by a third party. Compaction tests will be done at one location point randomly selected, or as indicated by the SAWS Inspector and/or the test administrator, per each 12-inch loose lift per 400 linear feet at a minimum. This project will not be accepted and finalized by SAWS without this requirement being met and verified by providing all necessary documented test results.

13. A copy of all testing reports shall be forwarded to SAWS Construction Inspection Division.

#### Sewer Notes:

1. The Contractor is responsible for ensuring that no Sanitary Sewer Overflow (SSO) occurs as a result of their work. All contractor personnel responsible for SSO prevention and control shall be trained on proper response. Should an SSO occur, the contractor shall:

- Identify the soruce of the SSO and notify SAWS Emergency Operations Center (EOC) immediately at (210) 233-2014. Provide the address of the spill and en estimated volume or flow B. Attempt to eliminate the source of the SSO
- C. Contain sewage from the SSO to the extent of preventing a possible contamination of waterways.
   D. Clean up spill site (return contained sewage to the collection system if possible) and properly dispose of contaminated soil/materials.
- E. Clean the affected sewer mains and remove any debris.
   F. Meet all post-SSO requirements as per the EPA Consent Decree, including line cleaning and

Should the Contractor fail to address an SSO immediately and to SAWS satisfaction, they will be responsible for all cost incurred by SAWS, including any fines from EPA, TCEQ and/or any other Federal, State or Local Agencies.

televising the affected sewer mains (at SAWS direction) within 24 hours.

No separate measurement or payment shall be made for this work. All work shall be done according to guidelines

2. If bypass pumping is required, the Contractor shall perform such work in accordance with SAWS Standard

and it is the responsibility of the Contractor sequence the work accordingly.

Specification for Water and Sanitary Sewer Construction, Item No. 864, "Bypass Pumping".

3. Prior to tie-ins, any shutdowns of existing force mains of any size must be coordinated with the SAWS Construction Inspection Division at (210) 233-2973 at least one week in advance of the shutdown. The Contractor must also provide a sequence of work as related to the tie-ins: this is at no additional cost to SAWS or the project

4. Sewer pipe where water line crosses shall be 160 psi and meet the requirements of ASTM D2241, TAC 217.53 and TCEQ 290.44(Ee)(4)(B). Contractor shall center a 20' joint of 160 psi pressure rated PVC at the proposed water crossing.

ELEVATIONS POSTED FOR TOP OF MANHOLES ARE FOR REFERENCE ONLY: It shall be the responsibility
of the Contractor to make allowances and adjustments for top of manholes to match the finished grade of the
project's improvements. (NSPI).

6. Spills, Overflows, or Discharges of Wastewater: All Spills, overflows, or discharges of wastewater, recycled water, petroleum products, or chemicals must be reported immediately to the SAWS Inspector assigned to the Counter Permit or GEneral Construction Permit (GCP). This requirement applies to every spill, overflow, or discharge regardless of size.

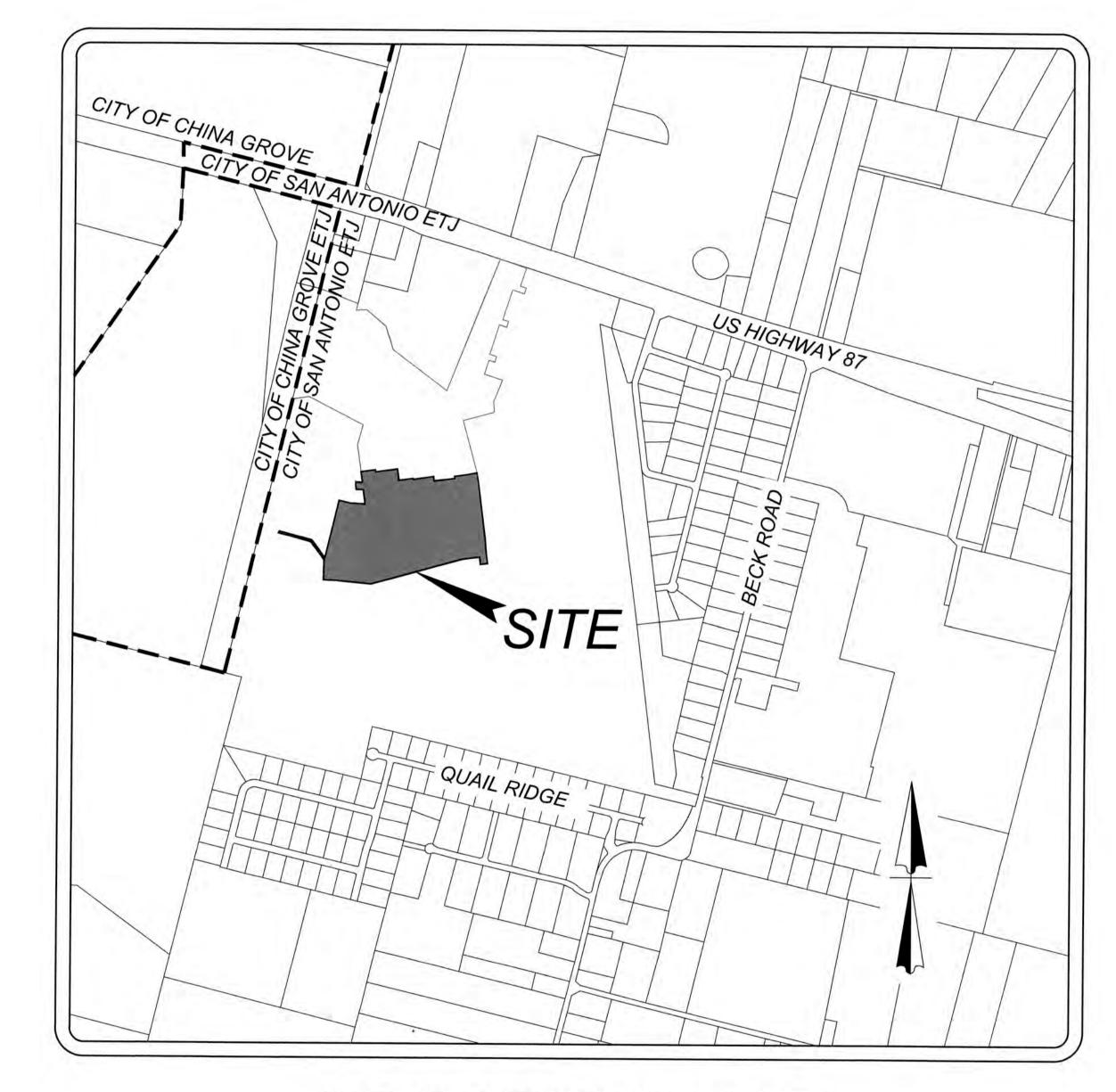
7. Manhole and all pipe testing (including the TV inspection) must be performed and passed prior to Final Field Acceptance by SAWS Construction Inspection Division, as per SAWS Specifications For Water and Sanitary Sewer Construction.

8. All PVC pipe over 14 feet of cover shall be extra strength with minimum pipe stiffness of 115 psi.

		SHEET	REVISION LOG
DATE	SHEET NO.	REVISION NO.	DESCRIPTION
10/17/2023	6.1,6.3	1	REVISED SEWER MAIN & ADDED MANHOLE

## GATES SUBDIVISION UNIT 2

## BEXAR COUNTY, TX SANITARY SEWER IMPROVEMENTS



## OVERALL SANITARY SEWER PLAN (SHEET 1 OF 2) 6.1 OVERALL SANITARY SEWER PLAN (SHEET 2 OF 2) 6.2 LINE "A" PLAN & PROFILE 6.3 LINE "B" PLAN & PROFILE 6.4 LINE "C", "D", & "E" PLAN & PROFILE 6.5 LINE "F" & "G" PLAN & PROFILE 6.6

Sheet Title

SANITARY SEWER COVER SHEET

SHEET LIST TABLE

Sheet Number

### LOCATION MAP NOT-TO-SCALE

OWNER INFORMATION
MEH HOLDING COMPANY LTD
5210 THOUSAND OAKS, SUITE 1318
SAN ANTONIO, TX 78233

 DEVELOPER'S NAME: MEH HOLDING COMPANY LTD

 DEVELOPER'S ADDRESS: 1202 W. BITTERS RD, BUILDING 1, SUITE 1200

 CITY: SAN ANTONIO
 STATE: TEXAS
 ZIP: 78258

 PHONE#:
 FAX#:
 TOTAL ACREAGE: 22.38 AC.

 SAWS BLOCK MAP#: 216562, 216560, 214560
 TOTAL EDU'S: 123

 TOTAL LINEAR FOOTAGE OF PIPE: 3,827.5 L.F. ~ 8" SDR 26
 PLAT NO.: 22-11800017

 NUMBER OF LOTS: 123
 SAWS JOB#: 22-1547

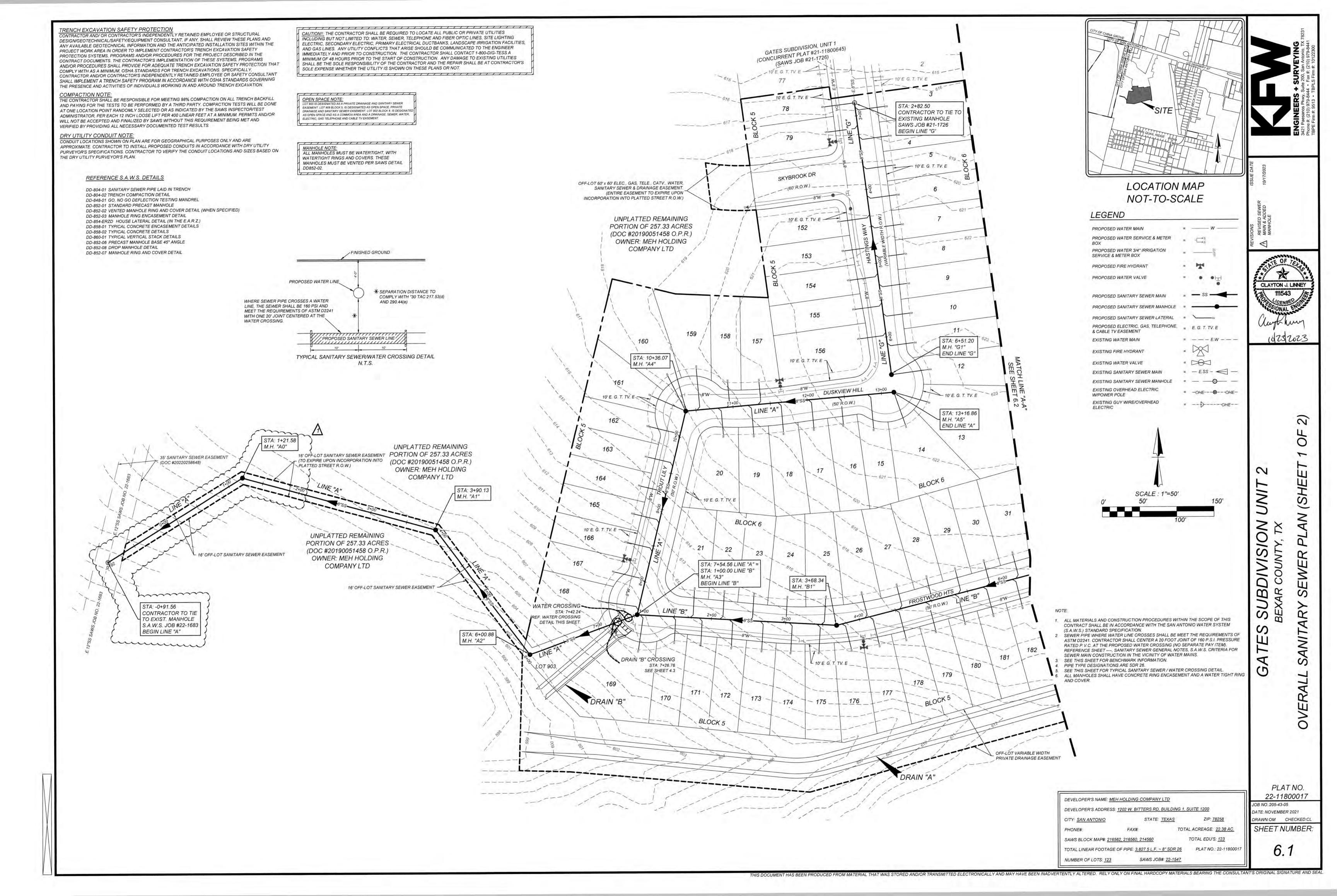




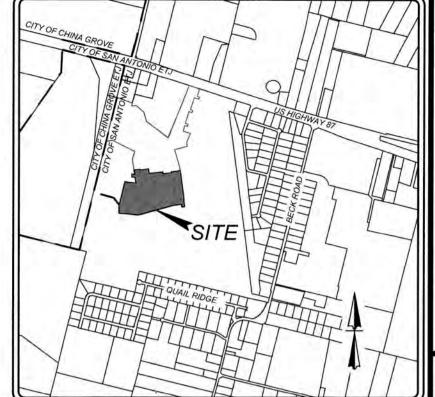
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PLAT NO. 22-11800017

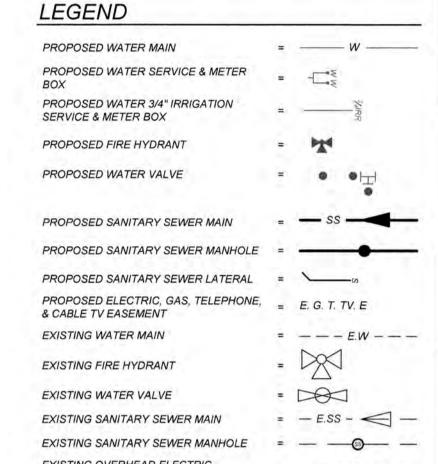
SHEET 6.0



TRENCH EXCAVATION SAFETY PROTECTION
CONTRACTOR AND/ OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR STRUCTURAL DRY UTILITY CONDUIT NOTE: REFERENCE S.A.W.S. DETAILS THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEETING 98% COMPACTION ON ALL TRENCH BACKFILL CONDUIT LOCATIONS SHOWN ON PLAN ARE FOR GEOGRAPHICAL PURPOSES ONLY AND ARE AND PAYING FOR THE TESTS TO BE PERFORMED BY A THIRD PARTY. COMPACTION TESTS WILL BE DONE DESIGN/GEOTECHNICAL/SAFETY/EQUIPMENT CONSULTANT, IF ANY, SHALL REVIEW THESE PLANS AND APPROXIMATE. CONTRACTOR TO INSTALL PROPOSED CONDUITS IN ACCORDANCE WITH DRY UTILITY ANY AVAILABLE GEOTECHNICAL INFORMATION AND THE ANTICIPATED INSTALLATION SITES WITHIN THE AT ONE LOCATION POINT RANDOMLY SELECTED OR AS INDICATED BY THE SAWS INSPECTOR/TEST PURVEYOR'S SPECIFICATIONS. CONTRACTOR TO VERIFY THE CONDUIT LOCATIONS AND SIZES BASED ON DD-804-01 SANITARY SEWER PIPE LAID IN TRENCH PROJECT WORK AREA IN ORDER TO IMPLEMENT CONTRACTOR'S TRENCH EXCAVATION SAFETY ADMINISTRATOR, PER EACH 12 INCH LOOSE LIFT PER 400 LINEAR FEET AT A MINIMUM. PERMITS AND/OR THE DRY UTILITY PURVEYOR'S PLAN. DD-804-02 TRENCH COMPACTION DETAIL PROTECTION SYSTEMS, PROGRAMS AND/OR PROCEDURES FOR THE PROJECT DESCRIBED IN THE WILL NOT BE ACCEPTED AND FINALIZED BY SAWS WITHOUT THIS REQUIREMENT BEING MET AND DD-848-01 GO, NO GO DEFLECTION TESTING MANDREL CONTRACT DOCUMENTS. THE CONTRACTOR'S IMPLEMENTATION OF THESE SYSTEMS, PROGRAMS VERIFIED BY PROVIDING ALL NECESSARY DOCUMENTED TEST RESULTS. DD-852-01 STANDARD PRECAST MANHOLE AND/OR PROCEDURES SHALL PROVIDE FOR ADEQUATE TRENCH EXCAVATION SAFETY PROTECTION THAT DD-852-02 VENTED MANHOLE RING AND COVER DETAIL (WHEN SPECIFIED) COMPLY WITH AS A MINIMUM, OSHA STANDARDS FOR TRENCH EXCAVATIONS. SPECIFICALLY, DD-852-03 MANHOLE RING ENCASEMENT DETAIL CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR SAFETY CONSULTANT DD-854-ERZD HOUSE LATERAL DETAIL (IN THE E.A.R.Z.) SHALL IMPLEMENT A TRENCH SAFETY PROGRAM IN ACCORDANCE WITH OSHA STANDARDS GOVERNING DD-858-01 TYPICAL CONCRETE ENCASEMENT DETAILS THE PRESENCE AND ACTIVITIES OF INDIVIDUALS WORKING IN AND AROUND TRENCH EXCAVATION. DD-858-02 TYPICAL CONCRETE DETAILS DD-860-01 TYPICAL VERTICAL STACK DETAILS SCALE: 1"=50" DD-852-06 PRECAST MANHOLE BASE 45° ANGLE 150' DD-852-08 DROP MANHOLE DETAIL DD-852-07 MANHOLE RING AND COVER DETAIL GATES SUBDIVISION, UNIT 1 (CONCURRENT PLAT #21-11800645) GATES SUBDIVISION, UNIT 1 (SAWS JOB #21-1726) (CONCURRENT PLAT #21-11800645) DEVELOPER'S NAME: MEH HOLDING COMPANY LTD (SAWS JOB #21-1726) DEVELOPER'S ADDRESS: 1202 W. BITTERS RD, BUILDING 1, SUITE 1200 CITY: SAN ANTONIO STATE: TEXAS ZIP: 78258 FAX#: PHONE#: TOTAL ACREAGE: 22.38 AC. 10' E. G. T. TV. E SAWS BLOCK MAP#: 216562, 216560, 214560 TOTAL EDU'S: 123 LEGEND TOTAL LINEAR FOOTAGE OF PIPE: 3,827.5 L.F. ~ 8" SDR 26 PLAT NO.: 22-11800017 STA: 2+20.00 - 619 -STA: 2+63.15 NUMBER OF LOTS: 123 SAWS JOB#: 22-1547 CONTRACTOR TO TIE TO CONTRACTOR TO TIE T EXISTING MANHOLE EXISTING MANHOLE SAWS JOB #22-1726 SAWS JOB #22-1726 BEGIN LINE "D" 5' E. G. T. TV. E BEGIN LINE "F" 5' E. G. T. TV. E -23 22 20 EXISTING OVERHEAD ELECTRIC STA: 4+67.98 1' V.N.A.E. W/POWER POLE M.H. "D1" EXISTING GUY WIRE/OVERHEAD 10' E. G. T. TV. E END LINE "D" 10' E. G. T. TV. E ELECTRIC UNPLATTED REMAINING STA: 5+17.12 10' E. G. T. TV. E PORTION OF 257.33 ACRES STA: 2+99.55 M.H. "F1" (DOC #20190051458 O.P.R.) M.H. "C1" END LINE "F END LINE "C" OWNER: MEH HOLDING COMPANY LTD STA: 3+59.00 11 M.H. "E1" END LINE "E" 1. ALL MATERIALS AND CONSTRUCTION PROCEDURES WITHIN THE SCOPE OF THIS (S.A.W.S.) STANDARD SPECIFICATION. 10' E. G. T. TV. E 35 3. SEE THIS SHEET FOR BENCHMARK INFORMATION. 13 4. PIPE TYPE DESIGNATIONS ARE SDR 26. STA: 11+89.58 M.H. "B4" WATER CROSSING END LINE "B" STA: 10+57.98 REF. WATER CROSSING DETAIL THIS SHEET. FROSTWOOD HTS 10' E. G. T. TV. E 10' E. G. T. TV. E (VARIABLE WIDTH R.O.V. WATER CROSSING STA: 7+75.19 REF. WATER CROSSING DETAIL THIS SHEET. --BLOCK 5 PROPOSED WATER LINE 1' V.N.A.E. STA: 10+77.12 LINE "B" = STA: 1+00.00 LINE "C" M.H. "B3" WHERE SEWER PIPE CROSSES A WATER LINE, THE SEWER SHALL BE 160 PSI AND BEGIN LINE "C" MEET THE REQUIREMENTS OF ASTM D2241 BLOCK 510' E. G. T. TV. E WITH ONE 20' JOINT CENTERED AT THE 193 WATER CROSSING. STA: 7+85.31 LINE "B" = 190 STA: 1+00.00 LINE "E" M.H. "B2" BEGIN LINE "E" OFF-LOT 60' x 60' ELEC., GAS, TELE., CATV., WATER. SANITARY SEWER & DRAINAGE EASEMENT. (ENTIRE EASEMENT TO EXPIRE UPON OFF-LOT VARIABLE WIDTH INCORPORATION INTO PLATTED STREET R.O.W.) PRIVATE DRAINAGE EASEMENT DRAIN "A" PORTION OF 257.33 ACRES (DOC #20190051458 O.P.R.) OWNER: MEH HOLDING
COMPANY LTD DD852-02. 



#### LOCATION MAP NOT-TO-SCALE

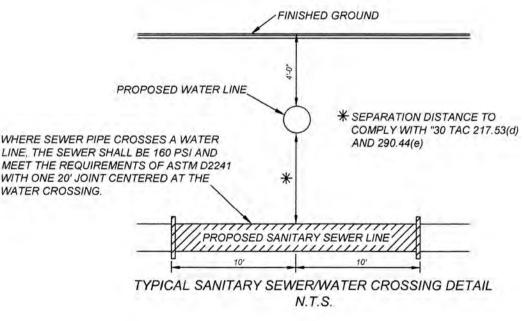


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CONTRACT SHALL BE IN ACCORDANCE WITH THE SAN ANTONIO WATER SYSTEM 2. SEWER PIPE WHERE WATER LINE CROSSES SHALL BE MEET THE REQUIREMENTS OF ASTM D2241. CONTRACTOR SHALL CENTER A 20 FOOT JOINT OF 160 P.S.I. PRESSURE RATED P.V.C. AT THE PROPOSED WATER CROSSING (NO SEPARATE PAY ITEM). REFERENCE SHEET ----, SANITARY SEWER GENERAL NOTES, S.A.W.S. CRITERIA FOR SEWER MAIN CONSTRUCTION IN THE VICINITY OF WATER MAINS.

5. SEE THIS SHEET FOR TYPICAL SANITARY SEWER / WATER CROSSING DETAIL. 6. ALL MANHOLES SHALL HAVE CONCRETE RING ENCASEMENT AND A WATER TIGHT RING



ALL MANHOLES MUST BE WATERTIGHT, WITH WATERTIGHT RINGS AND COVERS. THESE MANHOLES MUST BE VENTED PER SAWS DETAIL

CAUTION!!: THE CONTRACTOR SHALL BE REQUIRED TO LOCATE ALL PUBLIC OR PRIVATE UTILITIES INCLUDING BUT NOT LIMITED TO: WATER, SEWER, TELEPHONE AND FIBER OPTIC LINES, SITE LIGHTING ELECTRIC. SECONDARY ELECTRIC, PRIMARY ELECTRICAL DUCTBANKS, LANDSCAPE IRRIGATION FACILITIES, AND GAS LINES. ANY UTILITY CONFLICTS THAT ARISE SHOULD BE COMMUNICATED TO THE ENGINEER IMMEDIATELY AND PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL CONTACT 1-800-DIG-TESS A MINIMUM OF 48 HOURS PRIOR TO THE START OF CONSTRUCTION. ANY DAMAGE TO EXISTING UTILITIES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND THE REPAIR SHALL BE AT CONTRACTOR'S SOLE EXPENSE WHETHER THE UTILITY IS SHOWN ON THESE PLANS OR NOT. 

PLAT NO. 22-11800017 JOB NO.:205-43-05

CLAYTON J. LINNEY

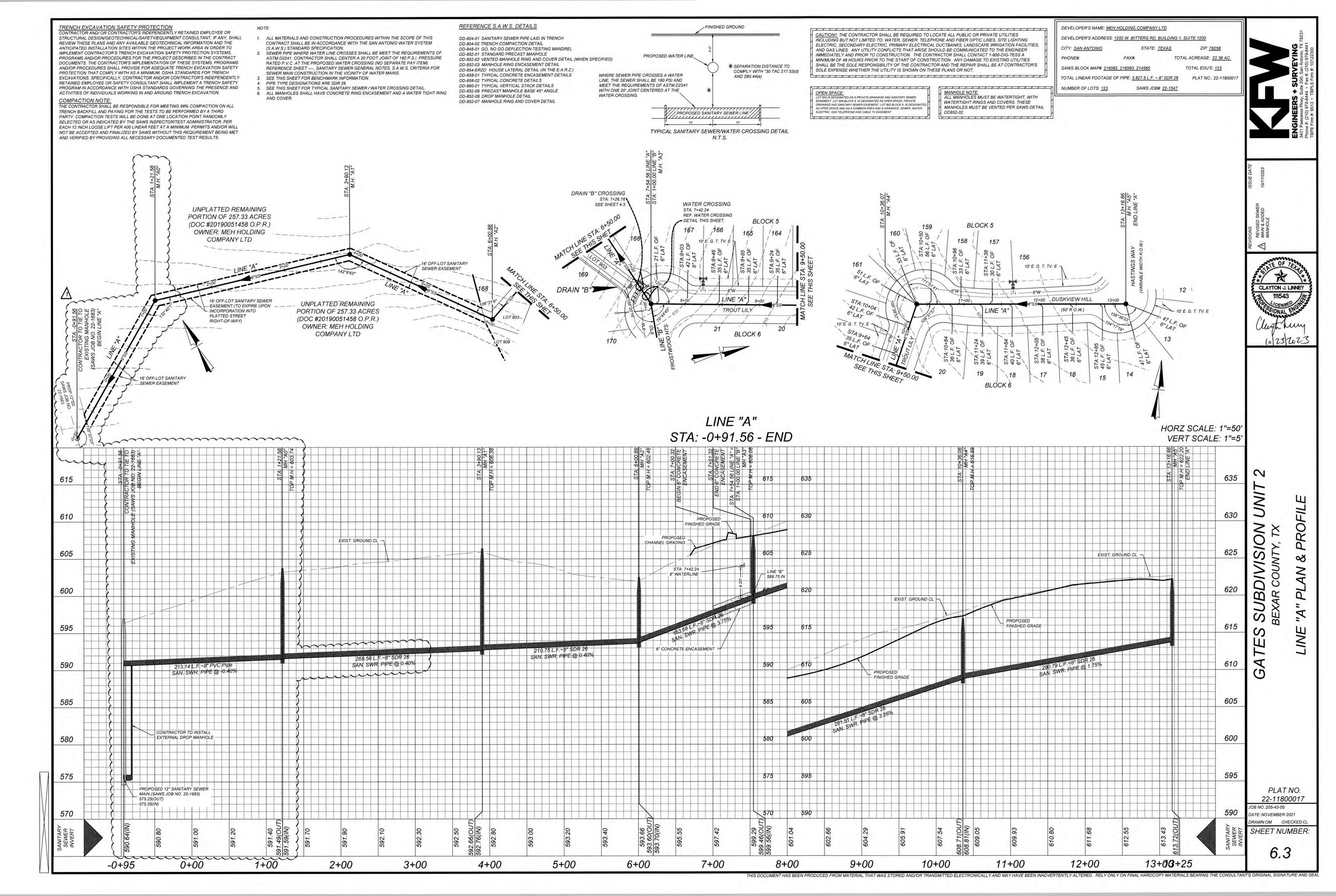
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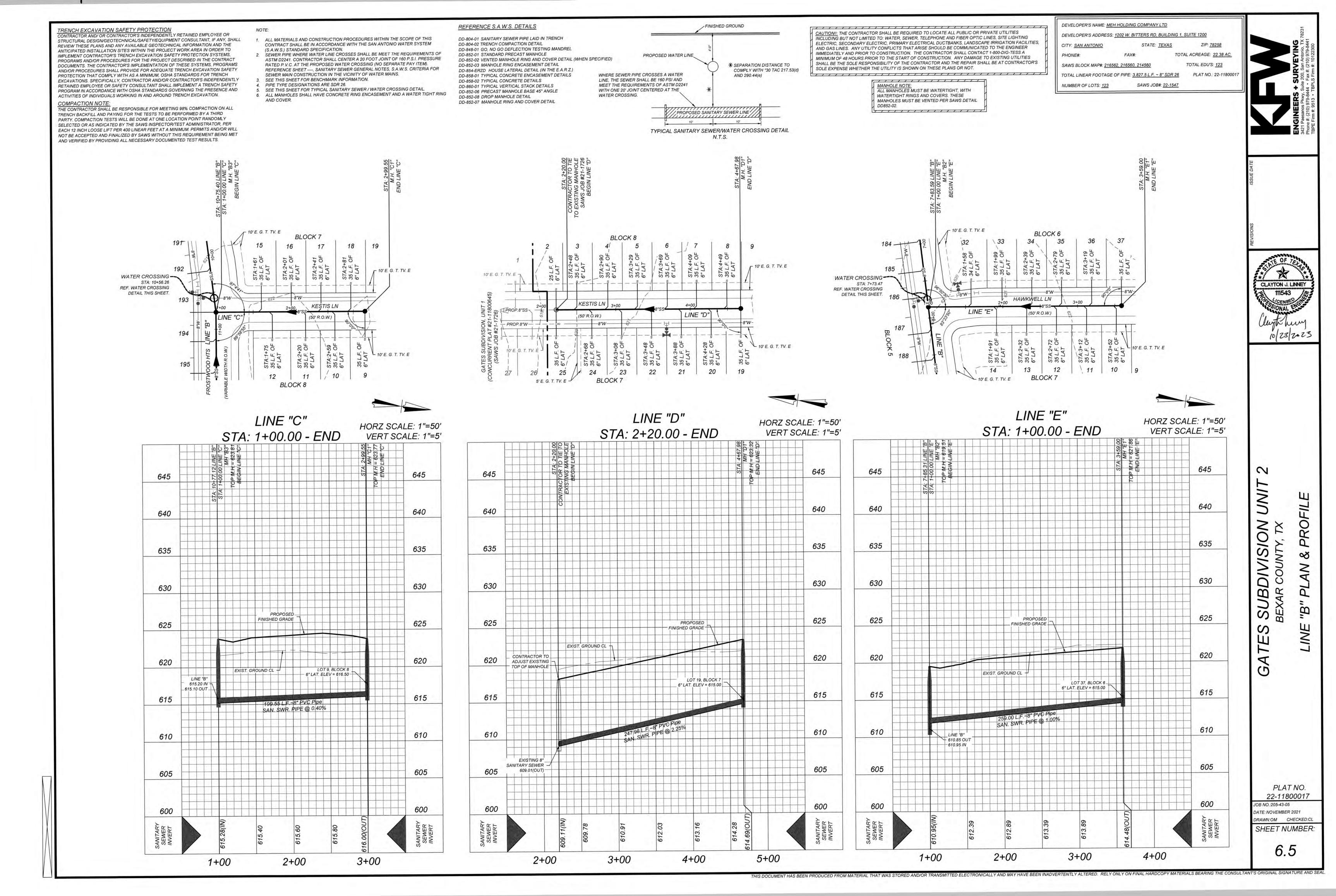
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ATE: NOVEMBER 2021 DRAWN:OM CHECKED:CL SHEET NUMBER:



REFERENCE S.A.W.S. DETAILS DEVELOPER'S NAME: MEH HOLDING COMPANY LTD FINISHED GROUND CONTRACTOR AND/ OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR CAUTION!!: THE CONTRACTOR SHALL BE REQUIRED TO LOCATE ALL PUBLIC OR PRIVATE UTILITIES STRUCTURAL DESIGN/GEOTECHNICAL/SAFETY/EQUIPMENT CONSULTANT, IF ANY, SHALL DEVELOPER'S ADDRESS: 1202 W. BITTERS RD, BUILDING 1, SUITE 1200 1. ALL MATERIALS AND CONSTRUCTION PROCEDURES WITHIN THE SCOPE OF THIS DD-804-01 SANITARY SEWER PIPE LAID IN TRENCH INCLUDING BUT NOT LIMITED TO: WATER, SEWER, TELEPHONE AND FIBER OPTIC LINES, SITE LIGHTING REVIEW THESE PLANS AND ANY AVAILABLE GEOTECHNICAL INFORMATION AND THE CONTRACT SHALL BE IN ACCORDANCE WITH THE SAN ANTONIO WATER SYSTEM ELECTRIC, SECONDARY ELECTRIC, PRIMARY ELECTRICAL DUCTBANKS, LANDSCAPE IRRIGATION FACILITIES, DD-804-02 TRENCH COMPACTION DETAIL ZIP: <u>78258</u> ANTICIPATED INSTALLATION SITES WITHIN THE PROJECT WORK AREA IN ORDER TO CITY: SAN ANTONIO (S.A.W.S.) STANDARD SPECIFICATION. DD-848-01 GO, NO GO DEFLECTION TESTING MANDREL AND GAS LINES. ANY UTILITY CONFLICTS THAT ARISE SHOULD BE COMMUNICATED TO THE ENGINEER IMPLEMENT CONTRACTOR'S TRENCH EXCAVATION SAFETY PROTECTION SYSTEMS, 2. SEWER PIPE WHERE WATER LINE CROSSES SHALL BE MEET THE REQUIREMENTS OF DD-852-01 STANDARD PRECAST MANHOLE IMMEDIATELY AND PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL CONTACT 1-800-DIG-TESS A PROPOSED WATER LINE TOTAL ACREAGE: 22.38 AC. PROGRAMS AND/OR PROCEDURES FOR THE PROJECT DESCRIBED IN THE CONTRACT PHONE#: ASTM D2241. CONTRACTOR SHALL CENTER A 20 FOOT JOINT OF 160 P.S.I. PRESSURE DD-852-02 VENTED MANHOLE RING AND COVER DETAIL (WHEN SPECIFIED) MINIMUM OF 48 HOURS PRIOR TO THE START OF CONSTRUCTION. ANY DAMAGE TO EXISTING UTILITIES DOCUMENTS. THE CONTRACTOR'S IMPLEMENTATION OF THESE SYSTEMS, PROGRAMS RATED P.V.C. AT THE PROPOSED WATER CROSSING (NO SEPARATE PAY ITEM). SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND THE REPAIR SHALL BE AT CONTRACTOR'S DD-852-03 MANHOLE RING ENCASEMENT DETAIL TOTAL EDU'S: 123 \* SEPARATION DISTANCE TO SAWS BLOCK MAP#: 216562, 216560, 214560 AND/OR PROCEDURES SHALL PROVIDE FOR ADEQUATE TRENCH EXCAVATION SAFETY REFERENCE SHEET ----, SANITARY SEWER GENERAL NOTES, S.A.W.S. CRITERIA FOR DD-854-ERZD HOUSE LATERAL DETAIL (IN THE E.A.R.Z.) SOLE EXPENSE WHETHER THE UTILITY IS SHOWN ON THESE PLANS OR NOT. COMPLY WITH "30 TAC 217.53(d) PROTECTION THAT COMPLY WITH AS A MINIMUM, OSHA STANDARDS FOR TRENCH SEWER MAIN CONSTRUCTION IN THE VICINITY OF WATER MAINS. DD-858-01 TYPICAL CONCRETE ENCASEMENT DETAILS WHERE SEWER PIPE CROSSES A WATER PLAT NO.: 22-1180001 TOTAL LINEAR FOOTAGE OF PIPE: 3,827.5 L.F. ~ 8" SDR 26 EXCAVATIONS. SPECIFICALLY, CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY AND 290.44(e) SEE THIS SHEET FOR BENCHMARK INFORMATION. LINE, THE SEWER SHALL BE 160 PSI AND DD-858-02 TYPICAL CONCRETE DETAILS RETAINED EMPLOYEE OR SAFETY CONSULTANT SHALL IMPLEMENT A TRENCH SAFETY PIPE TYPE DESIGNATIONS ARE SDR 26. DD-860-01 TYPICAL VERTICAL STACK DETAILS MEET THE REQUIREMENTS OF ASTM D2241 SAWS JOB#: 22-1547 PROGRAM IN ACCORDANCE WITH OSHA STANDARDS GOVERNING THE PRESENCE AND NUMBER OF LOTS: 123 5. SEE THIS SHEET FOR TYPICAL SANITARY SEWER / WATER CROSSING DETAIL. DD-852-06 PRECAST MANHOLE BASE 45° ANGLE WITH ONE 20' JOINT CENTERED AT THE MANHOLE NOTE: ALL MANHOLES MUST BE WATERTIGHT, WITH ACTIVITIES OF INDIVIDUALS WORKING IN AND AROUND TRENCH EXCAVATION. 6. ALL MANHOLES SHALL HAVE CONCRETE RING ENCASEMENT AND A WATER TIGHT RING WATER CROSSING. DD-852-08 DROP MANHOLE DETAIL AND COVER. COMPACTION NOTE: THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEETING 98% COMPACTION ON ALL EASEMENT. LOT 909 BLOCK 5, IS DESIGNATED AS OPEN SPACE, PRIVATE
DRAINAGE AND SANITARY SEWER EASEMENT. LOT 902 BLOCK 8, IS DESIGNATED
AS OPEN SPACE AND AS A COMMON AREA AND A DRAINAGE, SEWER, WATER,

WATERTIGHT RINGS AND COVERS. THESE
MANHOLES MUST BE VENTED PER SAWS DETAIL DD-852-07 MANHOLE RING AND COVER DETAIL TRENCH BACKFILL AND PAYING FOR THE TESTS TO BE PERFORMED BY A THIRD /PROPOSED SANITARY SEWER LINE ELECTRIC, GAS TELEPHONE AND CABLE TV EASEMENT. DD852-02. PARTY. COMPACTION TESTS WILL BE DONE AT ONE LOCATION POINT RANDOMLY SELECTED OR AS INDICATED BY THE SAWS INSPECTOR/TEST ADMINISTRATOR, PER EACH 12 INCH LOOSE LIFT PER 400 LINEAR FEET AT A MINIMUM. PERMITS AND/OR WILL TYPICAL SANITARY SEWER/WATER CROSSING DETAIL NOT BE ACCEPTED AND FINALIZED BY SAWS WITHOUT THIS REQUIREMENT BEING MET AND VERIFIED BY PROVIDING ALL NECESSARY DOCUMENTED TEST RESULTS. N.T.S. WATER CROSSING STA: 10+57.98 REF. WATER CROSSING DETAIL THIS SHEET. --LOT 902 BLOCK 7 WATER CROSSING STA: 7+75.19\_ REF. WATER CROSSING FROSTWOOD HTS DETAIL THIS SHEET. --CLAYTON J. LINNEY STA:10: 36 L.F. 6" LAT STA:11-40 L.F. 6" LAT 40 - 10' E. G. T. TV. E 196 192 191 190 10' E. G. T. TV. E BLOCK 5 184 178 LINE "B" HORZ SCALE: 1"=50' STA: 1+00.00 - END VERT SCALE: 1"=5" S PRO SUBDIVISION 630 PROPOSED NISHED GRADE "B" PL 8" WATERLINE 620 LOT 193, BLOCK 5 LOT 196, BLOCK 5 6" LAT. ELEV = 616.00 -GATE 6" LAT. ELEV = 615.50 PROPOSED STA: 7+75.19 8" WATERLINE FINISHED GRADE 615 EXIST. GROUND CL -20 L.F. ~ 160 PSI PRESSURE RATED 615.20 IN 610 605 605 600 600 PLAT NO. 22-11800017 595 JOB NO.:205-43-05 DATE: NOVEMBER 2021 DRAWN:OM CHECKED:CL SHEET NUMBER 6.4 12+00 8+00 9+00 10+00 11+00 5+00 6+00 7+00 1+00 4+00 2+00 3+00



TRENCH EXCAVATION SAFETY PROTECTION CONTRACTOR AND/ OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR STRUCTURAL DESIGN/GEOTECHNICAL/SAFETY/EQUIPMENT CONSULTANT, IF ANY, SHALL REVIEW THESE PLANS AND ANY AVAILABLE GEOTECHNICAL INFORMATION AND THE ANTICIPATED INSTALLATION SITES WITHIN THE PROJECT WORK AREA IN ORDER TO

IMPLEMENT CONTRACTOR'S TRENCH EXCAVATION SAFETY PROTECTION SYSTEMS, PROGRAMS AND/OR PROCEDURES FOR THE PROJECT DESCRIBED IN THE CONTRACT DOCUMENTS. THE CONTRACTOR'S IMPLEMENTATION OF THESE SYSTEMS, PROGRAMS AND/OR PROCEDURES SHALL PROVIDE FOR ADEQUATE TRENCH EXCAVATION SAFETY PROTECTION THAT COMPLY WITH AS A MINIMUM, OSHA STANDARDS FOR TRENCH EXCAVATIONS. SPECIFICALLY, CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR SAFETY CONSULTANT SHALL IMPLEMENT A TRENCH SAFETY PROGRAM IN ACCORDANCE WITH OSHA STANDARDS GOVERNING THE PRESENCE AND

PIPE TYPE DESIGNATIONS ARE SDR 26.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEETING 98% COMPACTION ON ALL TRENCH BACKFILL AND PAYING FOR THE TESTS TO BE PERFORMED BY A THIRD PARTY. COMPACTION TESTS WILL BE DONE AT ONE LOCATION POINT RANDOMLY SELECTED OR AS INDICATED BY THE SAWS INSPECTOR/TEST ADMINISTRATOR, PER EACH 12 INCH LOOSE LIFT PER 400 LINEAR FEET AT A MINIMUM. PERMITS AND/OR WILL NOT BE ACCEPTED AND FINALIZED BY SAWS WITHOUT THIS REQUIREMENT BEING MET AND VERIFIED BY PROVIDING ALL NECESSARY DOCUMENTED TEST RESULTS.

ACTIVITIES OF INDIVIDUALS WORKING IN AND AROUND TRENCH EXCAVATION.

- 1. ALL MATERIALS AND CONSTRUCTION PROCEDURES WITHIN THE SCOPE OF THIS CONTRACT SHALL BE IN ACCORDANCE WITH THE SAN ANTONIO WATER SYSTEM (S.A.W.S.) STANDARD SPECIFICATION.
- SEWER PIPE WHERE WATER LINE CROSSES SHALL BE MEET THE REQUIREMENTS OF ASTM D2241. CONTRACTOR SHALL CENTER A 20 FOOT JOINT OF 160 P.S.I. PRESSURE RATED P.V.C. AT THE PROPOSED WATER CROSSING (NO SEPARATE PAY ITEM). REFERENCE SHEET ----, SANITARY SEWER GENERAL NOTES, S.A.W.S. CRITERIA FOR SEWER MAIN CONSTRUCTION IN THE VICINITY OF WATER MAINS.
- SEE THIS SHEET FOR BENCHMARK INFORMATION.
- SEE THIS SHEET FOR TYPICAL SANITARY SEWER / WATER CROSSING DETAIL. ALL MANHOLES SHALL HAVE CONCRETE RING ENCASEMENT AND A WATER TIGHT RING
- AND COVER.

#### REFERENCE S.A.W.S. DETAILS DD-804-01 SANITARY SEWER PIPE LAID IN TRENCH DD-804-02 TRENCH COMPACTION DETAIL DD-848-01 GO, NO GO DEFLECTION TESTING MANDREL DD-852-01 STANDARD PRECAST MANHOLE PROPOSED WATER LINE DD-852-02 VENTED MANHOLE RING AND COVER DETAIL (WHEN SPECIFIED) DD-852-03 MANHOLE RING ENCASEMENT DETAIL \* SEPARATION DISTANCE TO COMPLY WITH "30 TAC 217.53(d) DD-854-ERZD HOUSE LATERAL DETAIL (IN THE E.A.R.Z.) DD-858-01 TYPICAL CONCRETE ENCASEMENT DETAILS WHERE SEWER PIPE CROSSES A WATER AND 290.44(e) DD-858-02 TYPICAL CONCRETE DETAILS LINE, THE SEWER SHALL BE 160 PSI AND DD-860-01 TYPICAL VERTICAL STACK DETAILS MEET THE REQUIREMENTS OF ASTM D2241 WITH ONE 20' JOINT CENTERED AT THE DD-852-06 PRECAST MANHOLE BASE 45° ANGLE DD-852-08 DROP MANHOLE DETAIL WATER CROSSING. DD-852-07 MANHOLE RING AND COVER DETAIL //PROPOSED SANITARY SEWER LINE

TYPICAL SANITARY SEWER/WATER CROSSING DETAIL

N.T.S.

CAUTION!!: THE CONTRACTOR SHALL BE REQUIRED TO LOCATE ALL FUBLIC ON THIS STEELIGHTING INCLUDING BUT NOT LIMITED TO: WATER, SEWER, TELEPHONE AND FIBER OPTIC LINES, SITE LIGHTING INCLUDING BUT NOT LIMITED TO: WATER, SEWER, TELEPHONE AND FIBER OPTIC LINES, SITE LIGHTING INCLUDING BUT NOT LIMITED TO: WATER, SEWER, TELEPHONE AND FIBER OPTIC LINES, SITE LIGHTING INCLUDING BUT NOT LIMITED TO: WATER, SEWER, TELEPHONE AND FIBER OPTIC LINES, SITE LIGHTING INCLUDING BUT NOT LINES, SITE LIGHTING BUT NOT L ELECTRIC, SECONDARY ELECTRIC, PRIMARY ELECTRICAL DUCTBANKS, LANDSCAPE IRRIGATION FACILITIES, AND GAS LINES. ANY UTILITY CONFLICTS THAT ARISE SHOULD BE COMMUNICATED TO THE ENGINEER IMMEDIATELY AND PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL CONTACT 1-800-DIG-TESS A MINIMUM OF 48 HOURS PRIOR TO THE START OF CONSTRUCTION. ANY DAMAGE TO EXISTING UTILITIES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND THE REPAIR SHALL BE AT CONTRACTOR'S SOLE EXPENSE WHETHER THE UTILITY IS SHOWN ON THESE PLANS OR NOT. 

> ALL MANHOLES MUST BE WATERTIGHT, WITH WATERTIGHT RINGS AND COVERS. THESE MANHOLES MUST BE VENTED PER SAWS DETAIL DD852-02. 7111111111111111

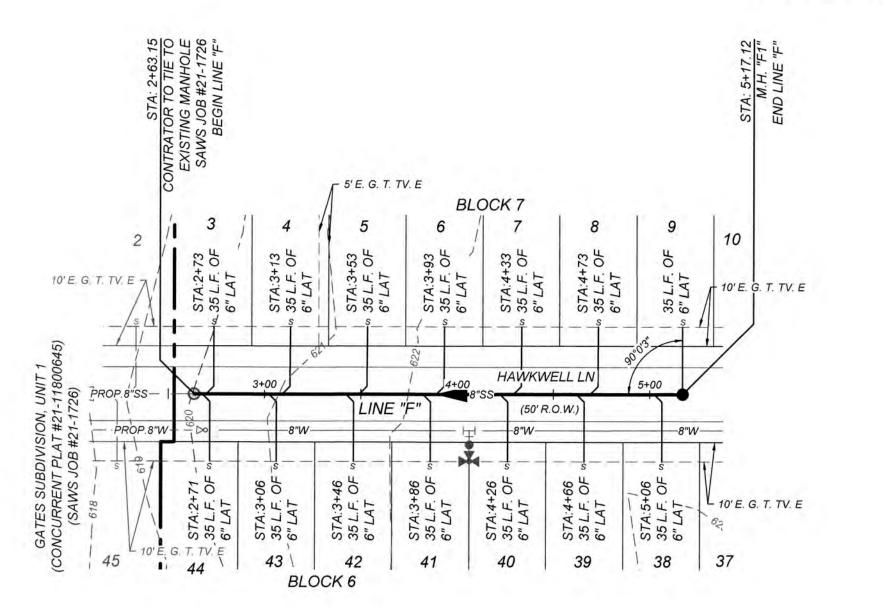
DEVELOPER'S NAME: MEH HOLDING COMPANY LTD

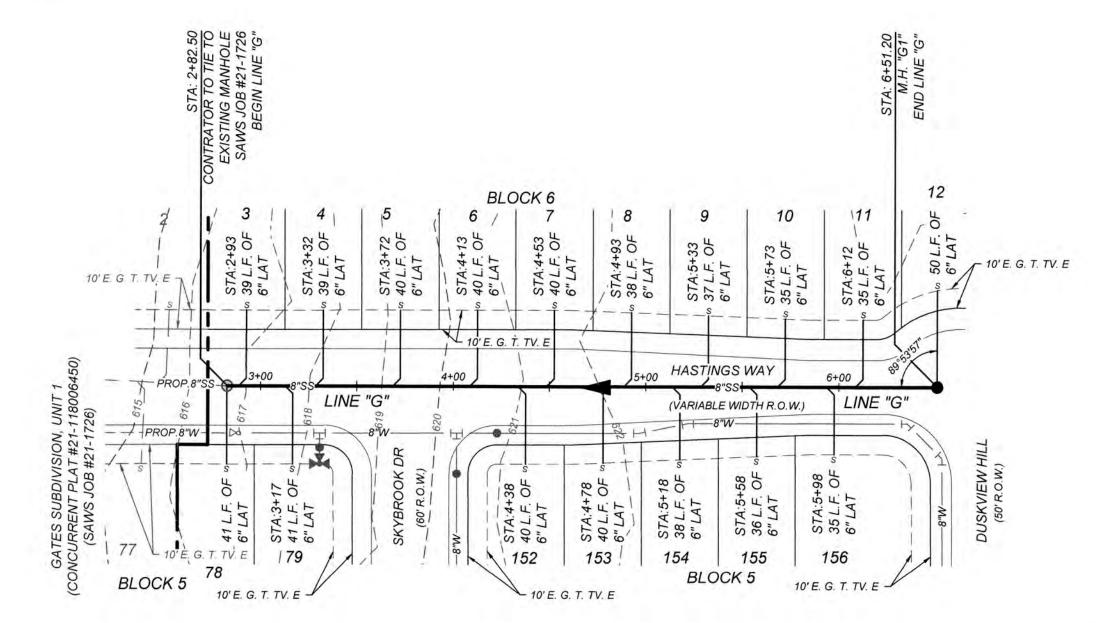
PHONE#:

DEVELOPER'S ADDRESS: 1202 W. BITTERS RD, BUILDING 1, SUITE 1200 ZIP: 78258

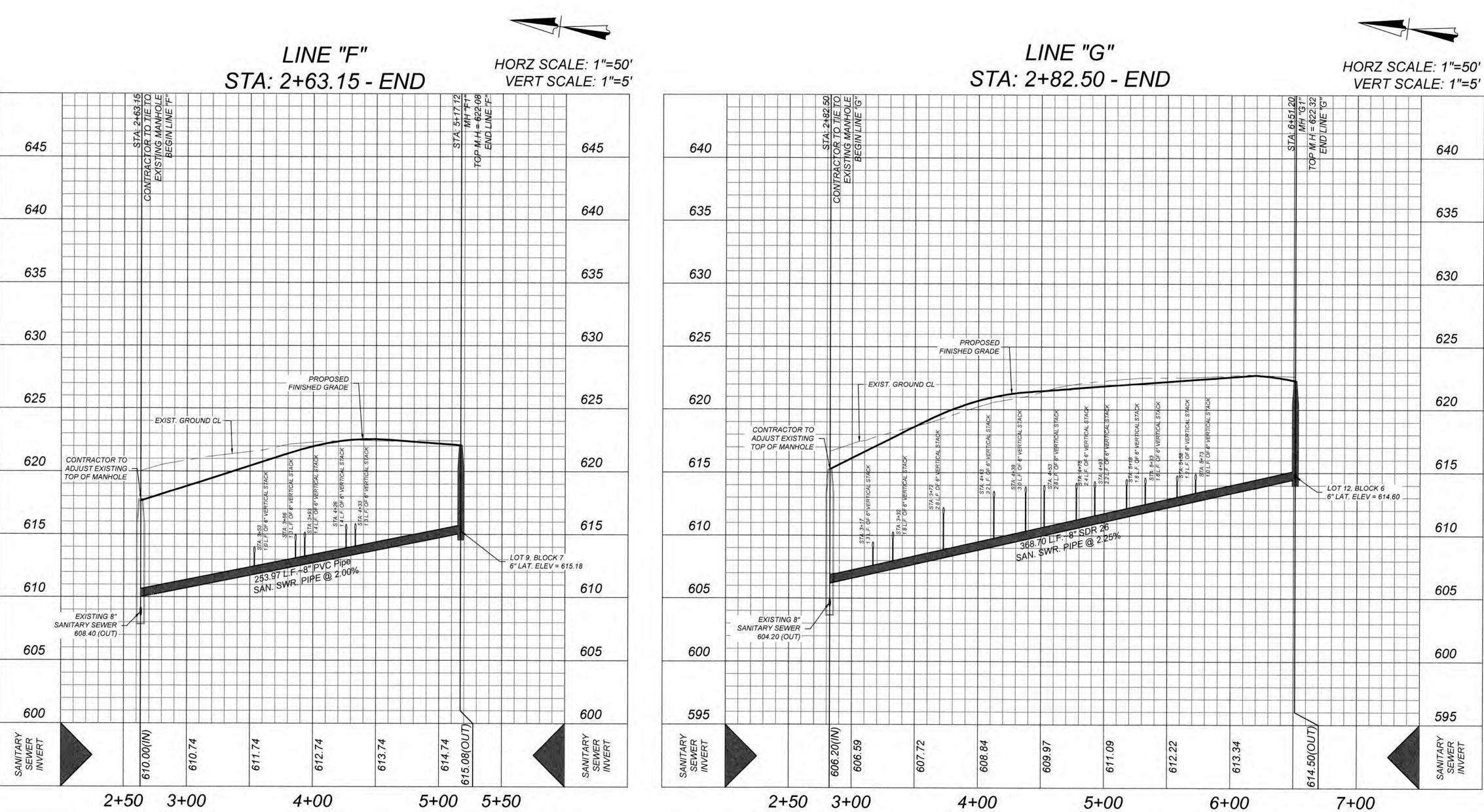
TOTAL ACREAGE: 22.38 AC.

SAWS BLOCK MAP#: 216562, 216560, 214560 TOTAL EDU'S: 123 TOTAL LINEAR FOOTAGE OF PIPE: 3,827.5 L.F. ~ 8" SDR 26 PLAT NO.: 22-11800017 SAWS JOB#: <u>22-1547</u> NUMBER OF LOTS: 123





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CLAYTON J. LINNEY

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BEXAR COUNTY, TX PLAN GATE

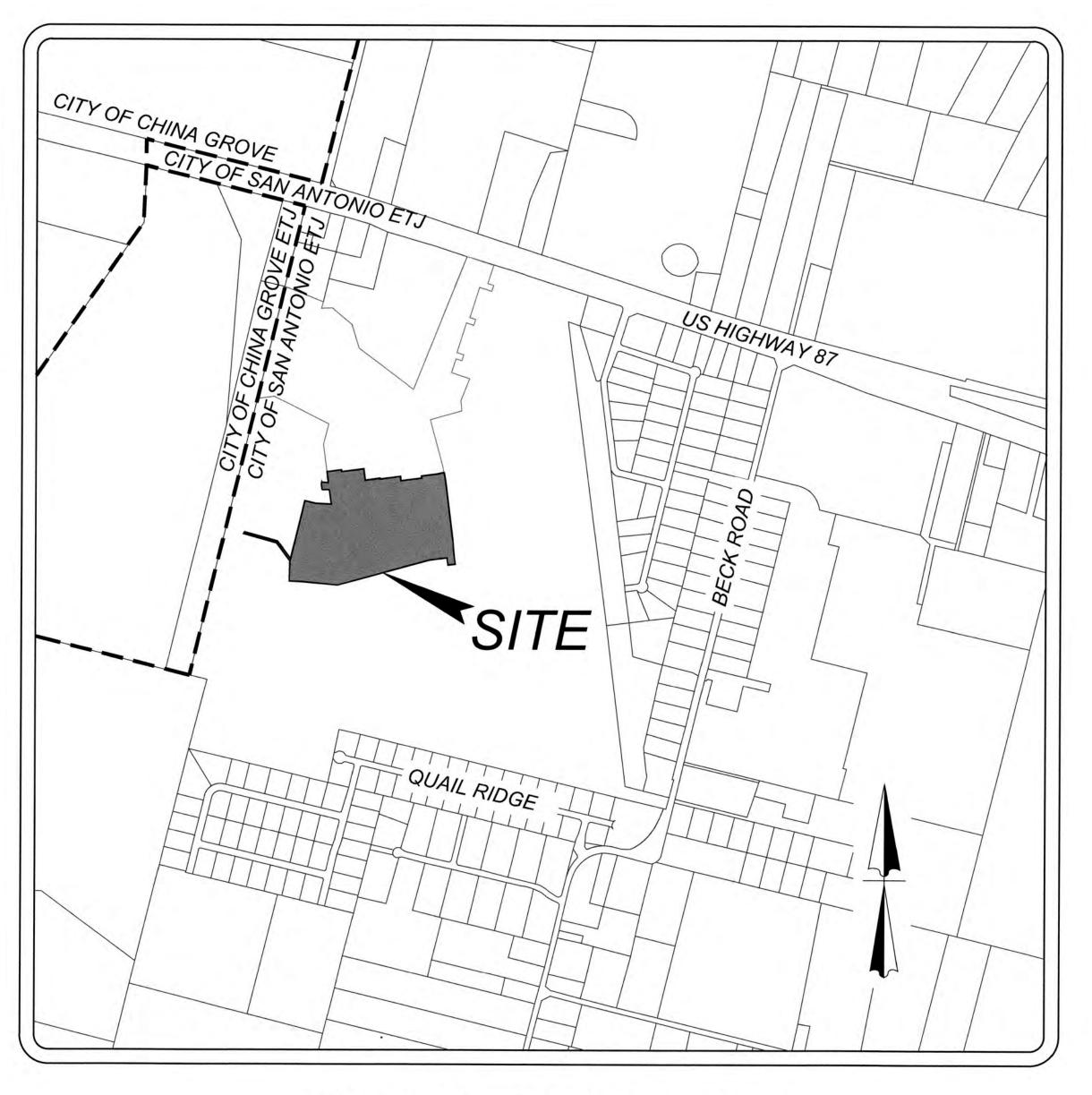
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PLAT NO. 22-11800017 JOB NO.:205-43-05

DATE: NOVEMBER 2021 RAWN:OM CHECKED:CL SHEET NUMBER:

### GATES SUBDIVISION UNIT 2

## BEXAR COUNTY, TX WATER IMPROVEMENTS



LOCATION MAP NOT-TO-SCALE

SHEET REVISION LOG

ADDED GATE VALVES

DESCRIPTION

SHEE REVISION T NO. NO.

OWNER INFORMATION
MEH HOLDING COMPANY LTD
5210 THOUSAND OAKS, SUITE 1318
SAN ANTONIO, TX 78233

#### SHEET LIST TABLE

Sheet Title	Sheet Number
WATER DISTRIBUTION COVER SHEET	7.0
WATER DISTRIBUTION PLAN (SHEET 1 OF 2)	7.1
WATER DISTRIBUTION PLAN (SHEET 2 OF 2)	7.2
WATER DISTRIBUTION PLAN DETAILS (SHEET 1 OF 4)	7.3
WATER DISTRIBUTION PLAN DETAILS (SHEET 2 OF 4)	7.4
WATER DISTRIBUTION PLAN DETAILS (SHEET 3 OF 4)	7.5
WATER DISTRIBUTION PLAN DETAILS (SHEET 4 OF 4)	7.6





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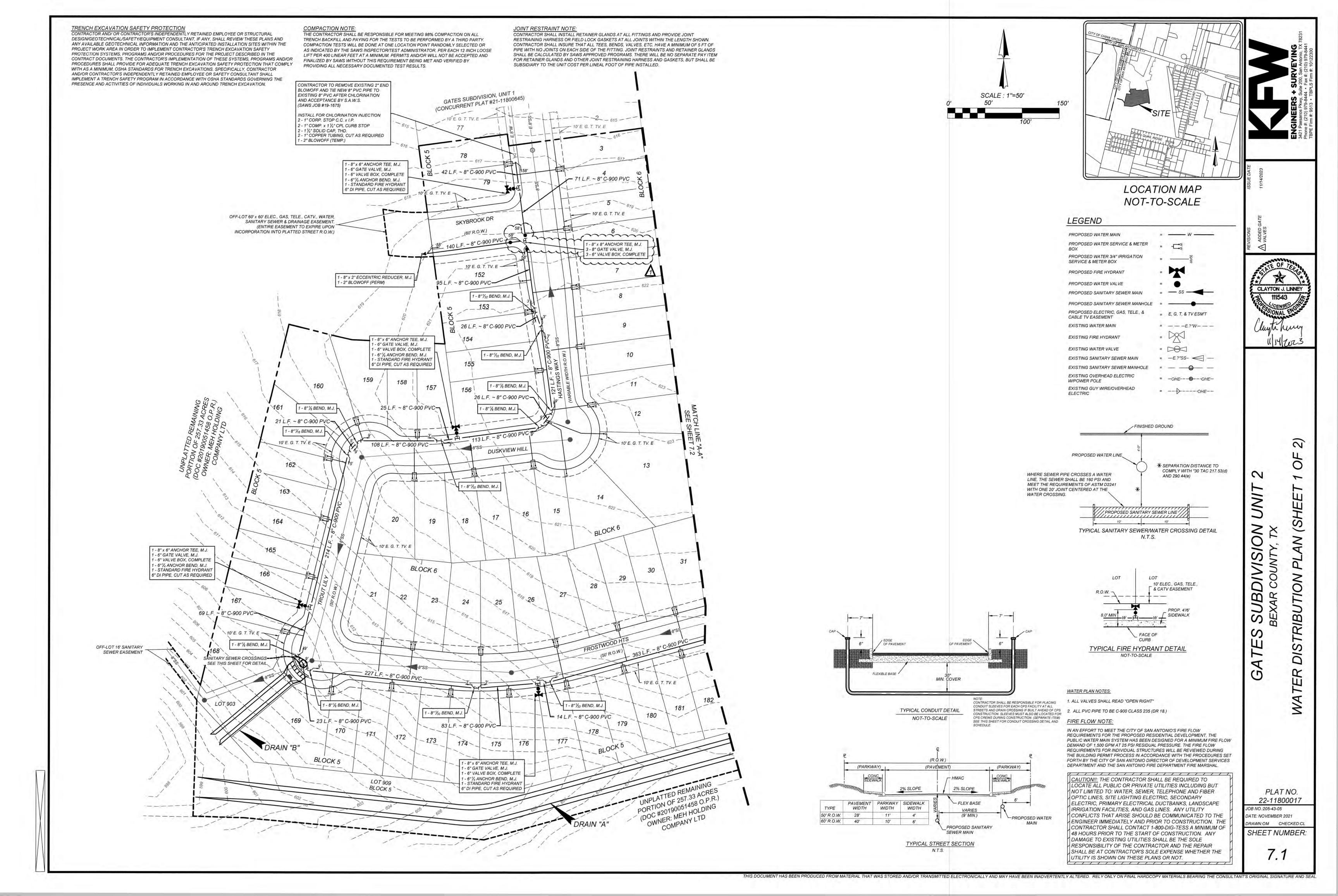
### TON J. LINNEY

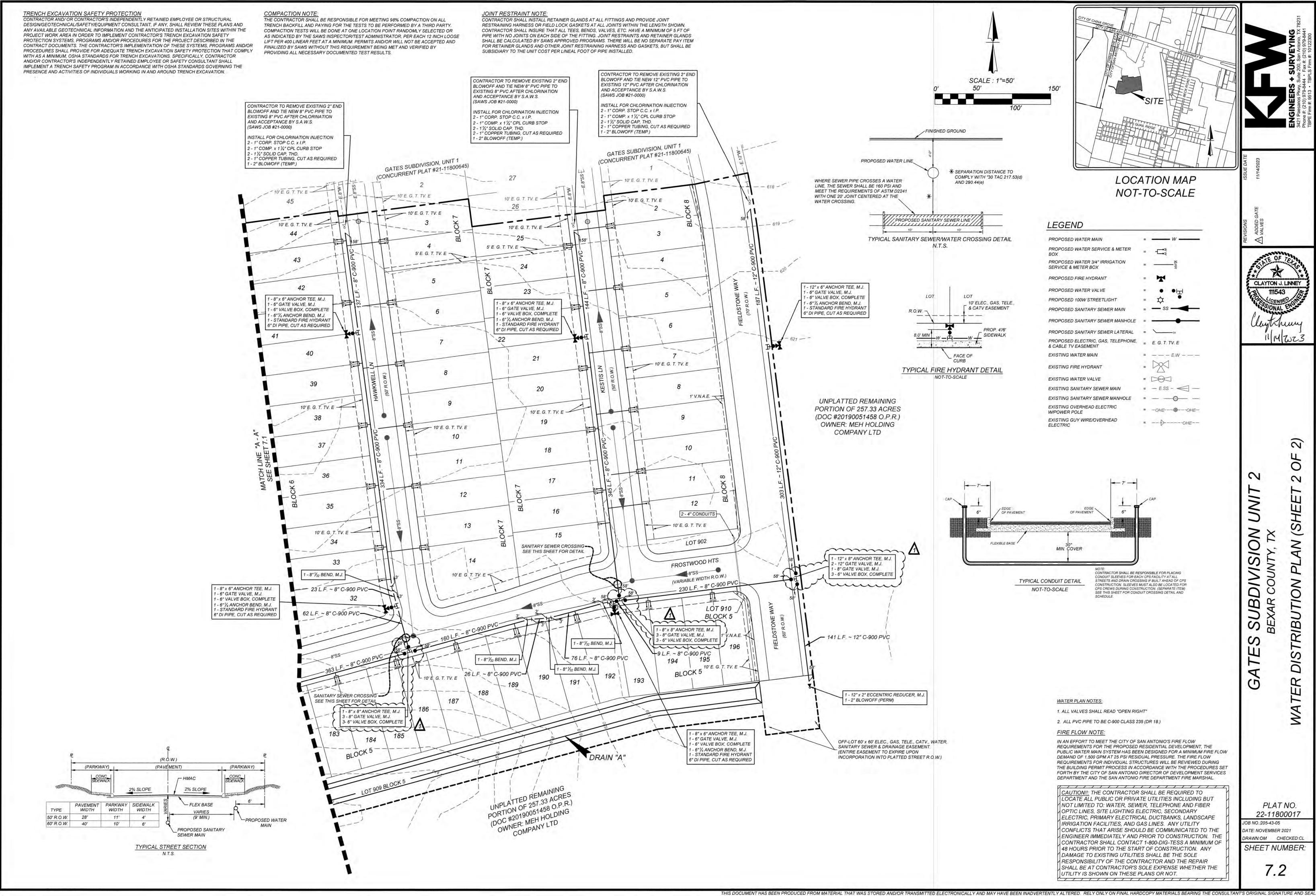
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PLAT NO. 22-11800017 JOB NO.:205-43-05

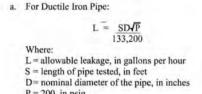
DATE: NOVEMBER 2021 DRAWN:OM CHECKED:CL SHEET NUMBER:

#### 3.0 CONSTRUCTION METHODS

- A. For Ductile Iron Pipe, make hydrostatic pressure and leakage tests on entire pipeline in accordance with AWWA Standard C600-99, Section 5.2.
- B. For PVC Pipe, make hydrostatic pressure and leakage tests on entire pipeline in accordance with AWWA Standard C605-94, Section 7.3.
- C. Furnish all labor equipment, including test pump with regulated by-pass meters and gauges required for conducting pipeline tests. Furnish equipment and necessary piping as required to transport water used in testing from source to test location.
- D. Schedule time and sequence of testing, subject to observation and approval by the Owner or Owner's Representative. Provide adequate labor, tools and equipment to operate valves and to locate and repair leaks discovered during the initial filling of the pipeline prior to actual testing or during the course of the tests.
- E. After the pipe has been laid and backfilled and the backfill has been jetted or otherwise consolidated, subject all newly laid pipe or valved section thereof to the hydrostatic pressure specified below for the particular type of pipe. The duration of each pressure test shall be at least four hours, unless otherwise specified or noted on the plans. Disconnect all meters, fixtures, devices or appliances which are connected to the pipeline system and which might be damaged if subjected to the specified test pressure. Cap or plug the ends of the branch lines during the testing procedures.
- F. Fill each valved (capped or plugged) section of pipe slowly with water and expel all air. If permanent air vents are not located at all high points, install corporation or blow-of cocks at such points so that the air can be expelled as filling takes place. After verification that all air has been expelled, close the cocks and keep the pipe filled until tested. Examine unexposed pipe, fittings, valves, hydrants and joints while under test pressure. Visible leaks shall be stopped. Remove and replace cracked or defective pipe, fittings, valves or hydrants discovered during testing. Replacement shall be with sound material. Repeat the test until specified requirements are achieved.
- G. Where any section of a pipeline is proved with concrete thrust blocking, do not make hydrostatic pressure test until at least five days have elapsed after installation

#### H. Pressure/Leakage Tests

- 1. The duration of the hydrostatic test shall be a minimum of four (4) hours.
- 2. The pipeline shall be tested so that the pressure at the highest point in the test section is not less than 200 psi.
- 3. The maximum allowable leakage is the number of gallons per hour as determined by the following formulas.



P = 200, in psig

#### b. For PVC Pipe: L = allowable leakage, in gallons per hour

#### N= number of joints in the length of pipeline tested D= nominal diameter of the pipe, in inches P = 200, in psig

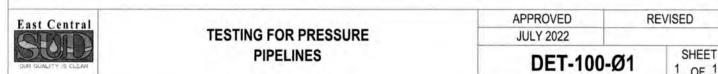
- H. Final Acceptance 1. No pipe installation will be accepted until known leaks have been repaired, whether or not leakage is within allowable limits. Locate and repair leaks at no
- 2. The Contractor will certify that all required pressure and leakage tests have been successfully completed before the pipeline is accepted.

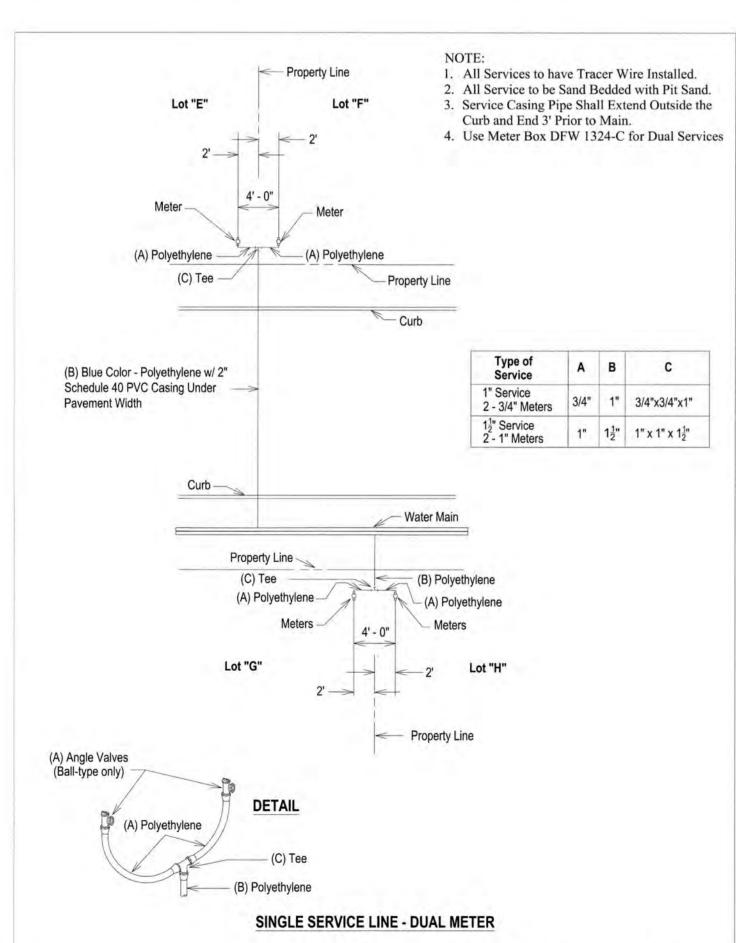
#### Special Project Requirements

Water Source. Obtaining water for testing purposes shall be the responsibility of the Contractor. The Contractor shall provide all equipment and labor required to transport water from the source to the test point.

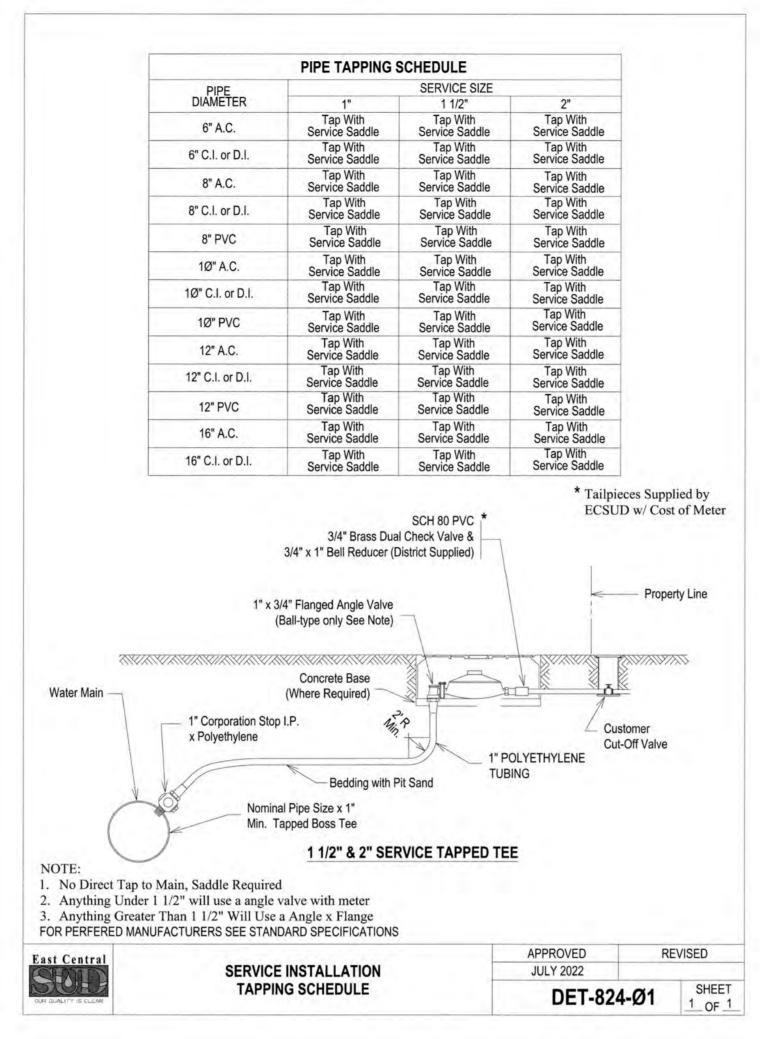
#### 4.0 MEASUREMENT AND PAYMENT

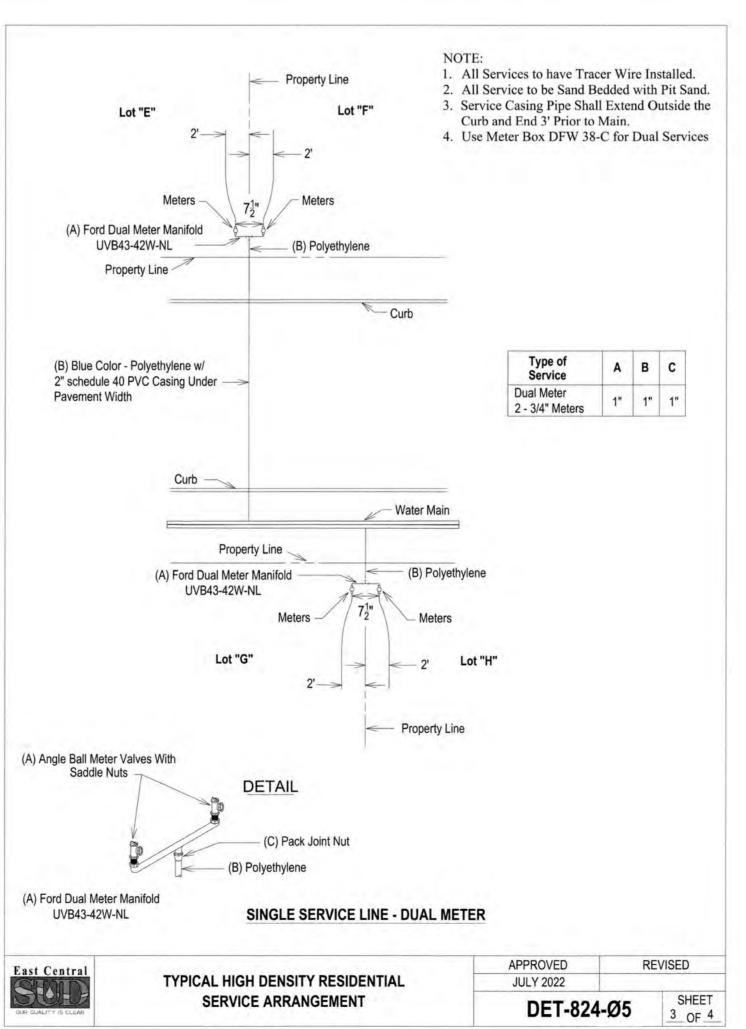
No direct measurement or payment will be made for the work to be done or the equipment to be furnished under this item, but shall be considered subsidiary to the particular items of work for which unit prices are required in the proposal.

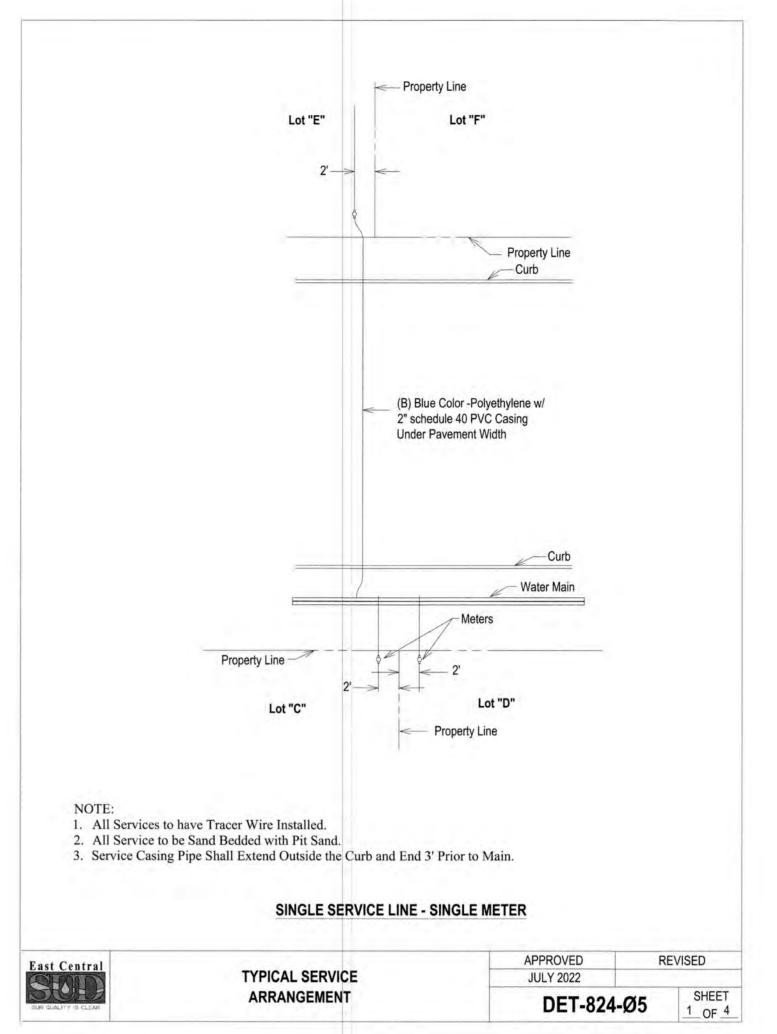


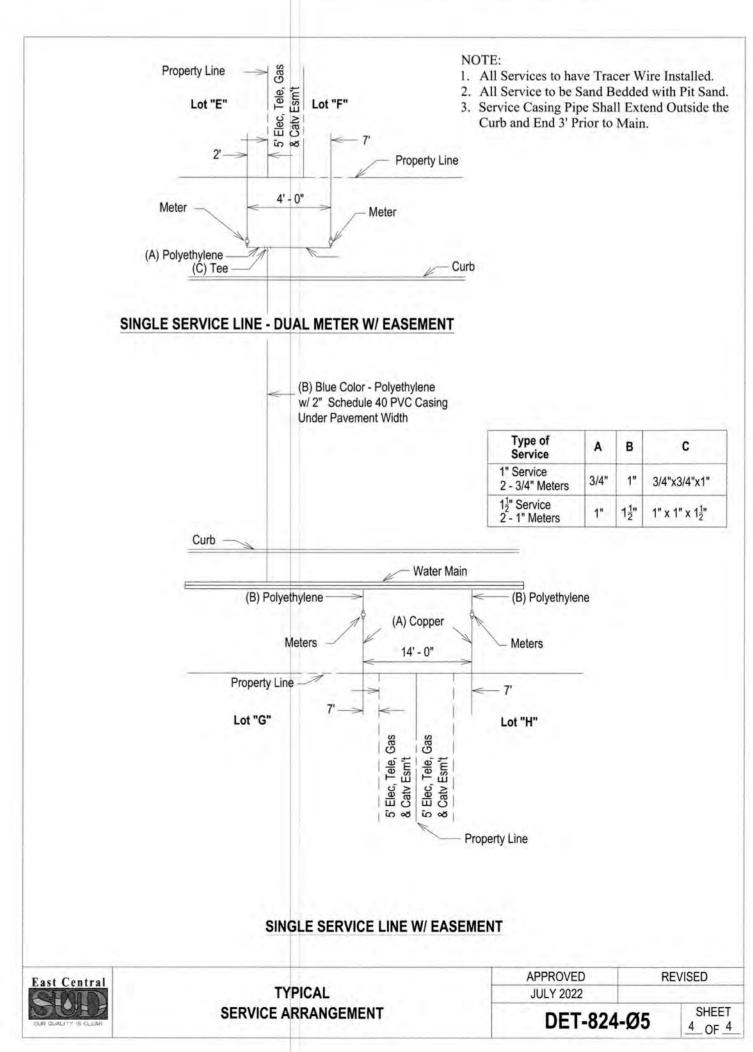


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YOU BOKE	TYPICAL RURAL	JULY 2022		
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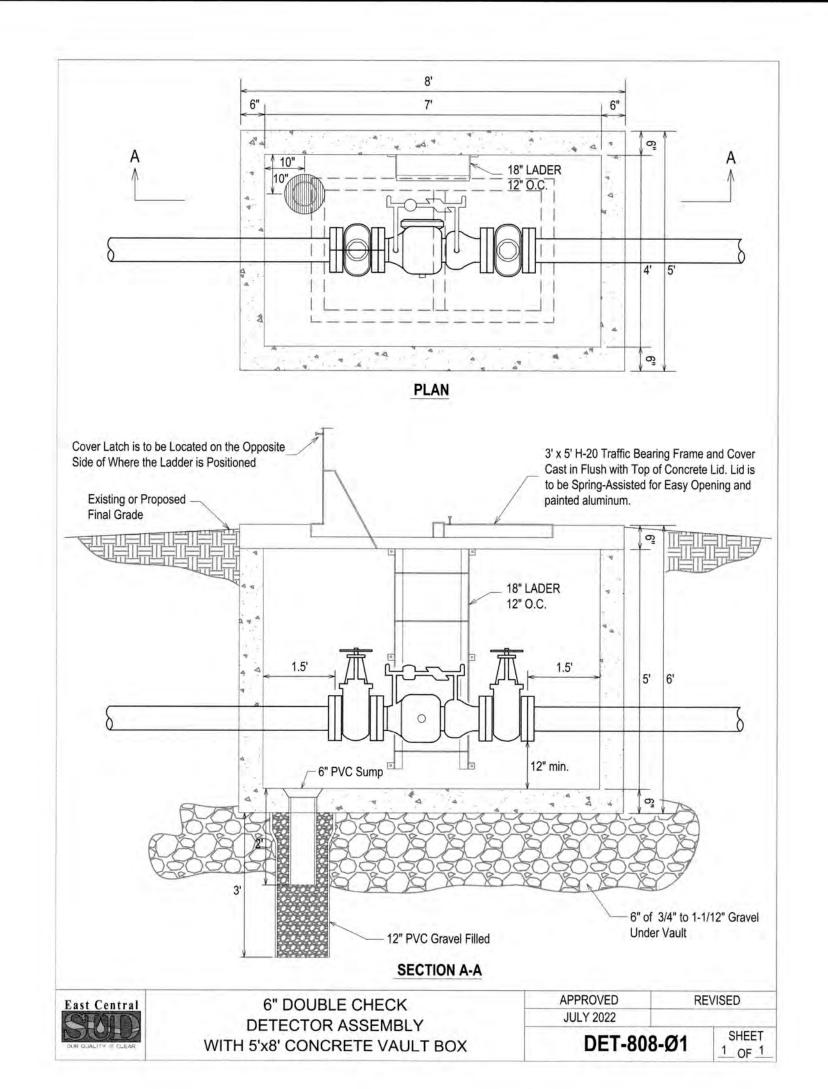


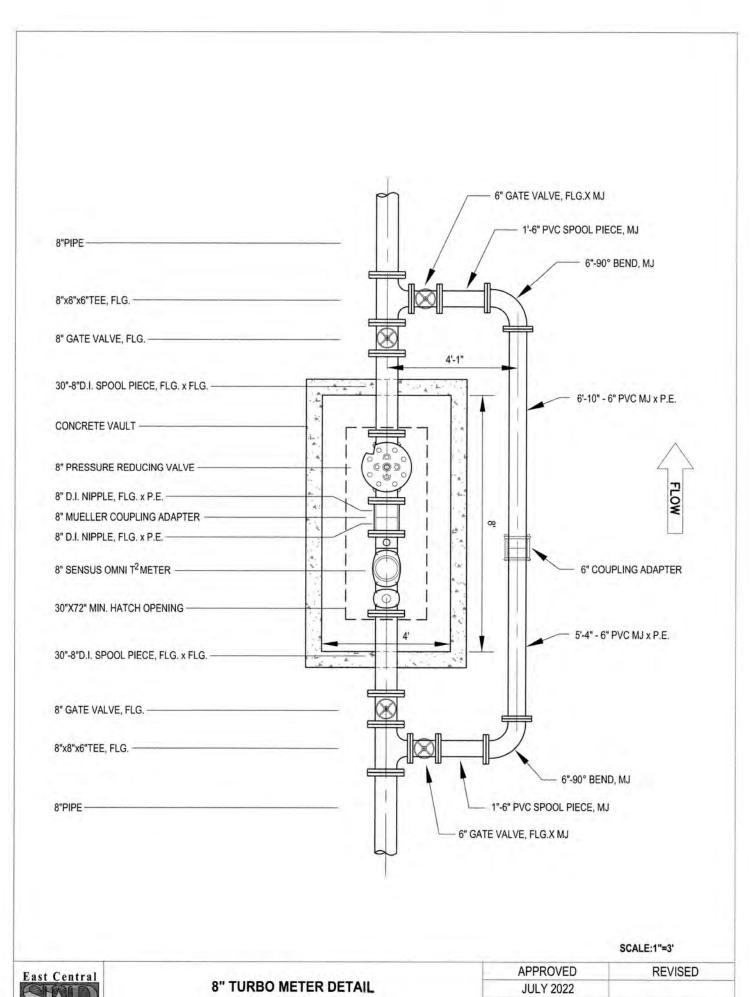




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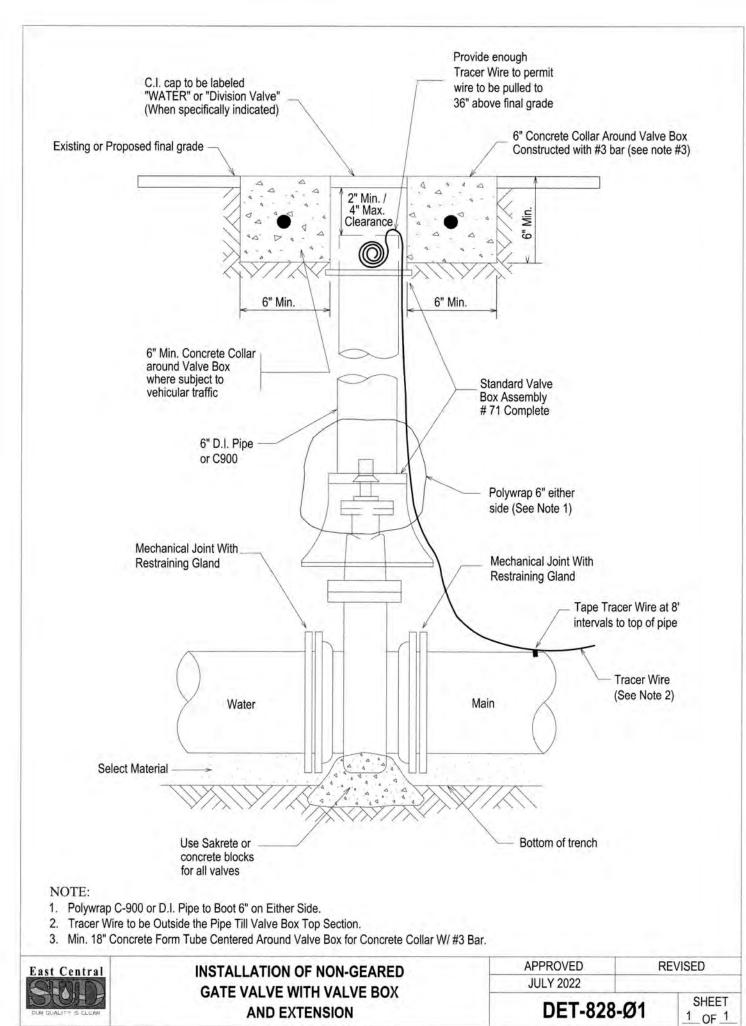


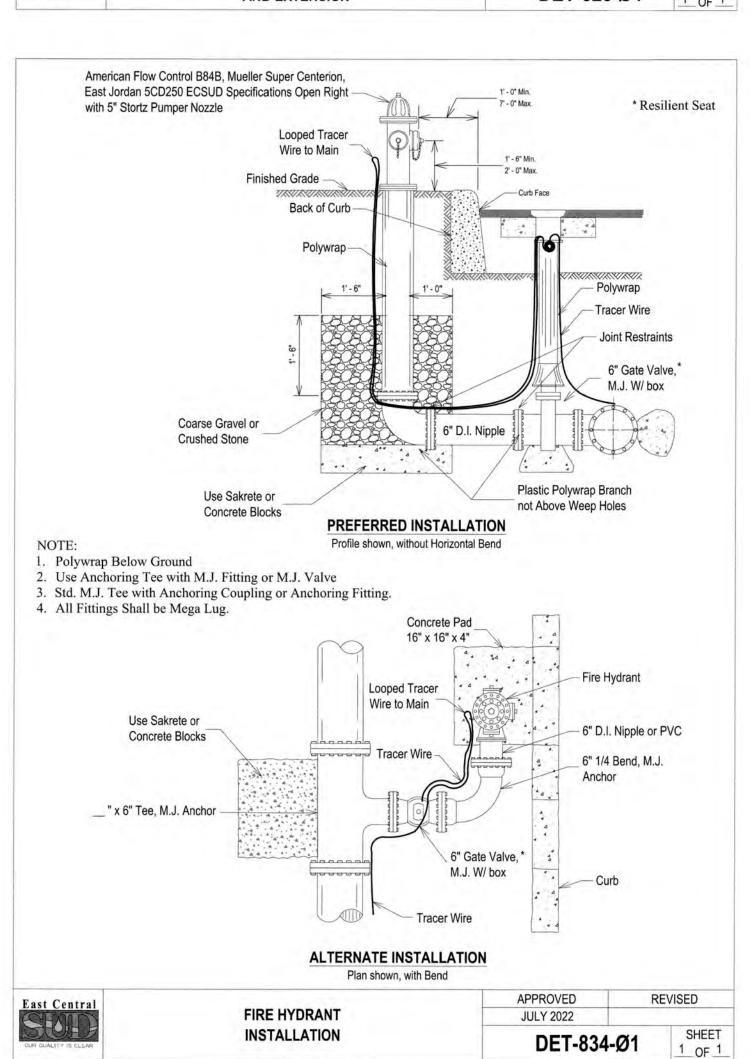


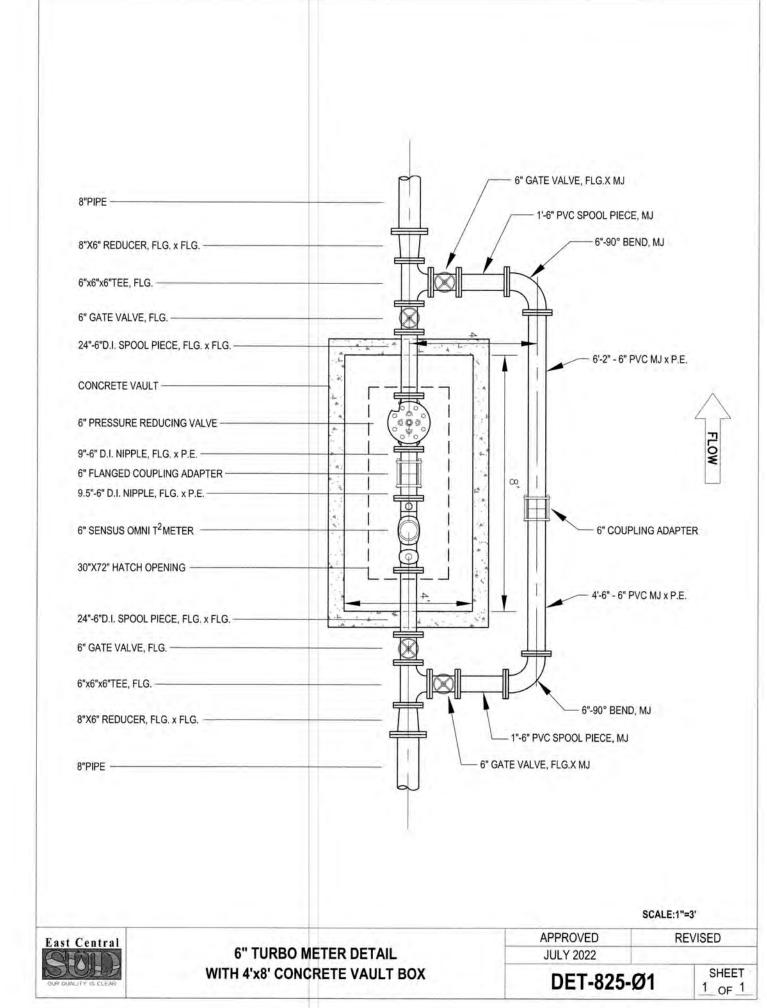
WITH 4'x8' CONCRETE VAULT BOX

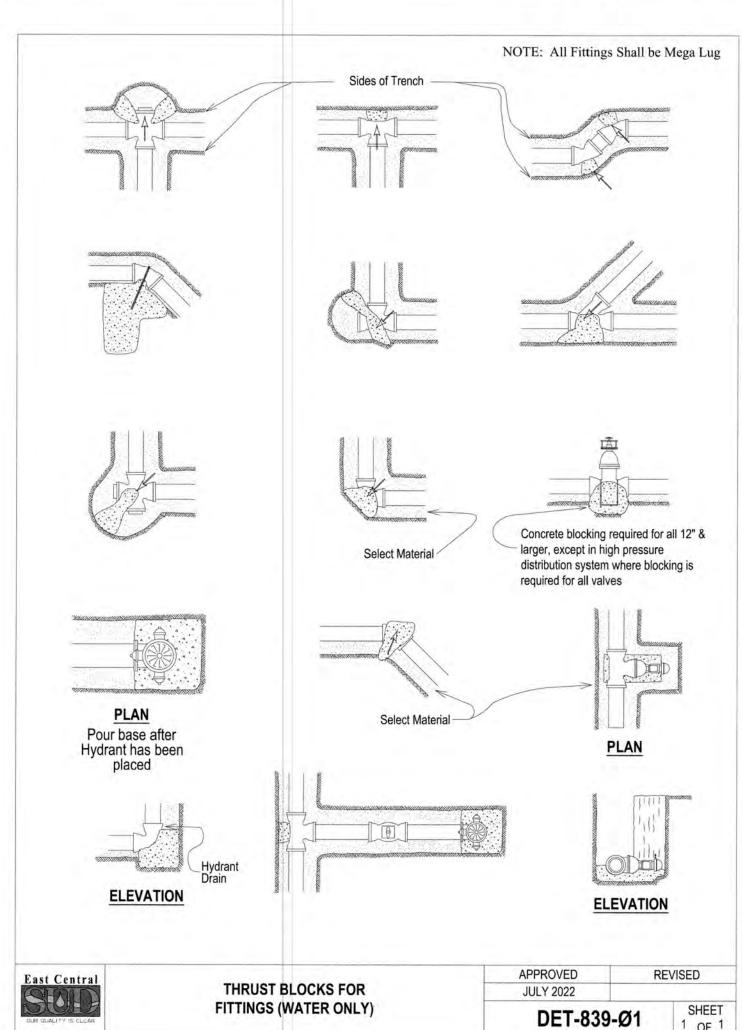
SHEET 1\_OF\_1

DET-825-Ø2









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DISTRIBUTION

SUBDIVISION UNIT BEXAR COUNTY, TX

GATES

PLAT NO. 22-11800017

JOB NO.:205-43-05

DATE: NOVEMBER 2021

DRAWN:OM CHECKED:CL

SHEET NUMBER:

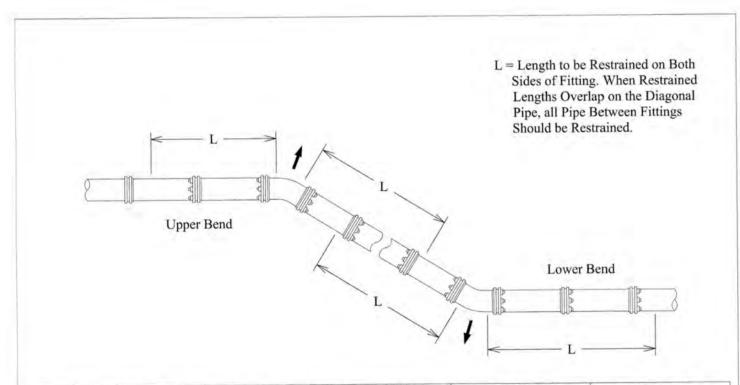
		RESTR	AINED LENGTH FOR T	EES
PIPE SIZE (Inch)	BRANCH SIZE (Inch)	LENGTH OF RUN (Feet)	RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=200 psi	RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=150 ps
6	4	0	42	31
6	4	5	7	1
6	4	10	1	1
6	6	0	59	44
6	6	5	35	20
6	6	10	11	1
8	4	0	42	31
8	4	5	1	1
8	6	0	59	44
8	6	5	28	13
8	6	10	1	1
8	8	0	77	58
8	8	5	53	34
8	8	10	30	11
8	8	15	6	1

#### RESTRAINED LENGTH DESIGN

Restrained Length Calculations are for P.V.C. Pipe Bedded in Compacted Granular Material Extending to the Top of the Pipe. The Native Soil Material is Assumed to be Inorganic Clay of High Plasticity. Depth of Bury is Assumed to be 4 Feet.

- 1. These Calculations are Provided for Reference. The Restrained Length Shall be Designed Based Upon the Conditions
- Encountered During Installation. 2. All joints within the calculated length must be restrained.
- 3. If your distance between fittings is less than or equal to the calculated restraint length, restrain all joints between those fittings.

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East Central	RESTRAINED LENGTHS	MARCH 2020	
OUR QUALITY IS CLEAR	FOR TEES	DET-839-04	SHEE 1 OF



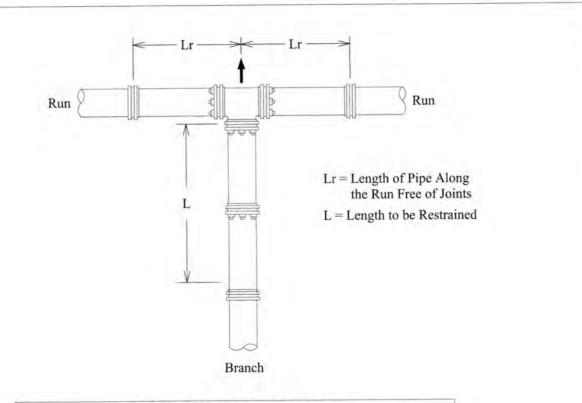
PIPE SIZE (Inch)	BEND ANGLE (deg.)	LOW SIDE DEPTH	UPPER BEND RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=200 psi	LOWER BEND RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=200 psi	UPPER BEND RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=150 psi	LOWER BEND RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=150 psi
6	45	5	24	8	18	6
6	22.5	5	12	4	9	3
6	11.25	5	6	2	4	1
6	45	10	24	5	18	4
6	22.5	10	12	2	9	2
6	11.25	10	6	1	4	1
8	45	5	32	11	24	8
8	22.5	5	15	5	11	4
8	11.25	5	8	3	6	2
8	45	10	32	7	24	5
8	22.5	10	15	3	11	2
8	11.25	10	8	2	6	1
12	45	5	45	16	34	12
12	22.5	5	22	7	16	6
12	11.25	5	11	4	8	3
12	45	10	45	10	34	7
12	22.5	10	22	5	16	3
12	11.25	10	11	2	8	2

#### RESTRAINED LENGTH DESIGN

Restrained Length Calculations are for P.V.C. Pipe Bedded in Compacted Granular Material Extending to the Top of the Pipe. The Native Soil Material is Assumed to be Inorganic Clay of High Plasticity. Depth of Bury is Assumed to be 4 Feet.

- 1. These Calculations are Provided for Reference. The Restrained Length Shall be Designed Based Upon the Conditions
- Encountered During Installation.
- All joints within the calculated length must be restrained. 3 If your distance between fittings is less than or equal to the calculated restraint length, restrain all joints between those fittings.

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Central  ALITY IS CLEAR	RESTRAINED LENGTHS VERTICAL OFFSETS	JULY 2022		
		DET-839	-06	SHE 1 OF



		RESTR	AINED LENGTH FOR T	EES
PIPE SIZE (Inch)	BRANCH SIZE (Inch)	LENGTH OF RUN (Feet)	RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=200 psi	RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=150 psi
12	4	0	42	31
12	4	5	1	1
12	6	0	59	44
12	6	5	13	1
12	6	10	1	1
12	8	0	77	58
12	8	5	42	23
12	8	10	7	1
12	8	15	1	1
12	12	0	109	82
12	12	5	86	59
12	12	10	63	35
12	12	15	39	12

#### RESTRAINED LENGTH DESIGN

Restrained Length Calculations are for P.V.C. Pipe Bedded in Compacted Granular Material Extending to the Top of the Pipe. The Native Soil Material is Assumed to be Inorganic Clay of High Plasticity. Depth of Bury is Assumed to be 4 Feet.

1. These Calculations are Provided for Reference. The Restrained Length Shall be Designed Based Upon the Conditions Encountered During Installation.

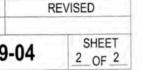
All joints within the calculated length must be restrained.

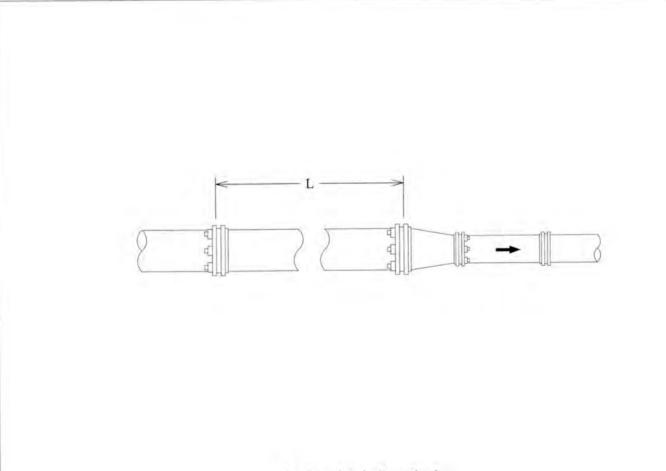
3. If your distance between fittings is less than or equal to the calculated restraint length, restrain all joints between those fittings.

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l	OUR QUALITY IS CLEAR

RESTRAINED LENGTHS
FOR TEES

MARCH 2020 **DET-839-04** 





#### L = Length to be Restrained

PIPE SIZE (Inch)	SMALL SIZE (Inch)	RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=200 psi	RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=150 psi
6	4	30	23
8	4	55	42
8	6	32	24
12	4	95	71
12	6	80	60
12	8	58	43

#### RESTRAINED LENGTH DESIGN

Restrained Length Calculations are for P.V.C. Pipe Bedded in Compacted Granular Material Extending to the Top of the Pipe. The Native Soil Material is Assumed to be Inorganic Clay of High Plasticity. Depth of Bury is Assumed to be 4 Feet.

- 1. These Calculations are Provided for Reference. The Restrained Length Shall be Designed Based Upon the Conditions
- Encountered During Installation. All joints within the calculated length must be restrained.
- 3. If your distance between fittings is less than or equal to the calculated restraint length, restrain all joints between those fittings.

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RESTRAINED LENGTHS FOR REDUCERS

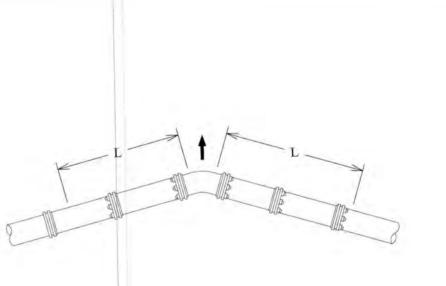
JULY 2022 **DET-839-07** 

**APPROVED** 

REVISED

RESTRAINED LENGTHS FOR HORIZONTAL BENDS

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L = Length to be Restrained

Restrained Length Calculations are for P.V.C. Pipe Bedded in Compacted Granular Material Extending to the Top of the Pipe. The

1. These Calculations are Provided for Reference. The Restrained Length Shall be Designed Based Upon the Conditions

3. If your distance between fittings is less than or equal to the calculated restraint length, restrain all joints between those fittings.

JULY 2022

**DET-839-05** 

Native Soil Material is Assumed to be Inorganic Clay of High Plasticity. Depth of Bury is Assumed to be 4 Feet.

RESTRAINED LENGTHS FOR

DEAD ENDS / INLINE VALVES

Encountered During Installation.

All joints within the calculated length must be restrained.

RESTRAINED LENGTH RESTRAINED LENGTH IN FEET, WHEN IN FEET, WHEN TEST PRESSURE=200 psi TEST PRESSURE=150 psi

L = Length to be Restrained on Both Sides of Fitting

PIPE SIZE (Inch)	BEND ANGLE (deg)	RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=200 psi	RESTRAINED LENGTH IN FEET, WHEN TEST PRESSURE=150 psi
6	90	23	17
6	45	9	7
6	22.5	5	3
6	11.25	2	2
8	90	30	22
8	45	12	9
8	22.5	6	4
8	11.25	3	2
12	90	43	32
12	45	18	13
12	22.5	8	6
12	11,25	4	3

#### RESTRAINED LENGTH DESIGN

Restrained Length Calculations are for P.V.C. Pipe Bedded in Compacted Granular Material Extending to the Top of the Pipe. The Native Soil Material is Assumed to be Inorganic Clay of High Plasticity. Depth of Bury is Assumed to be 4 Feet.

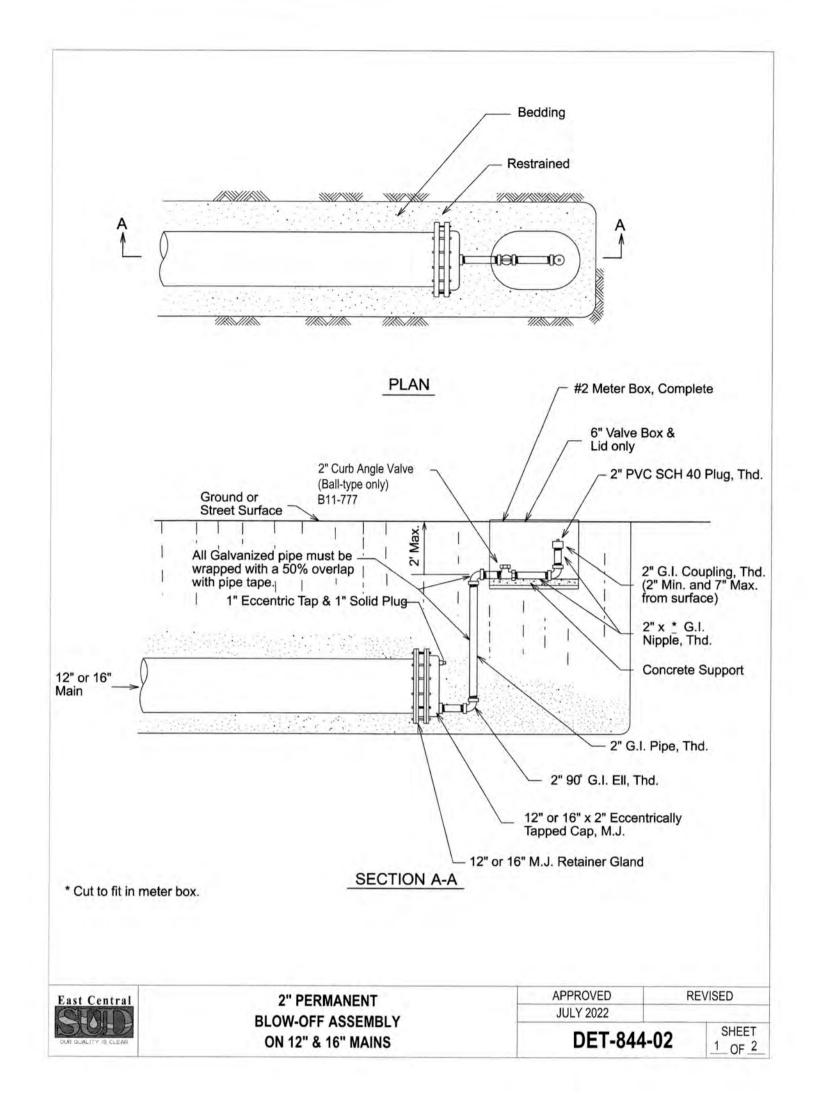
- 1. These Calculations are Provided for Reference, The Restrained Length Shall be Designed Based Upon the Conditions
- Encountered During Installation. All joints within the calculated length must be restrained.
- 3. If your distance between fittings is less than or equal to the calculated restraint length, restrain all joints between those fittings.

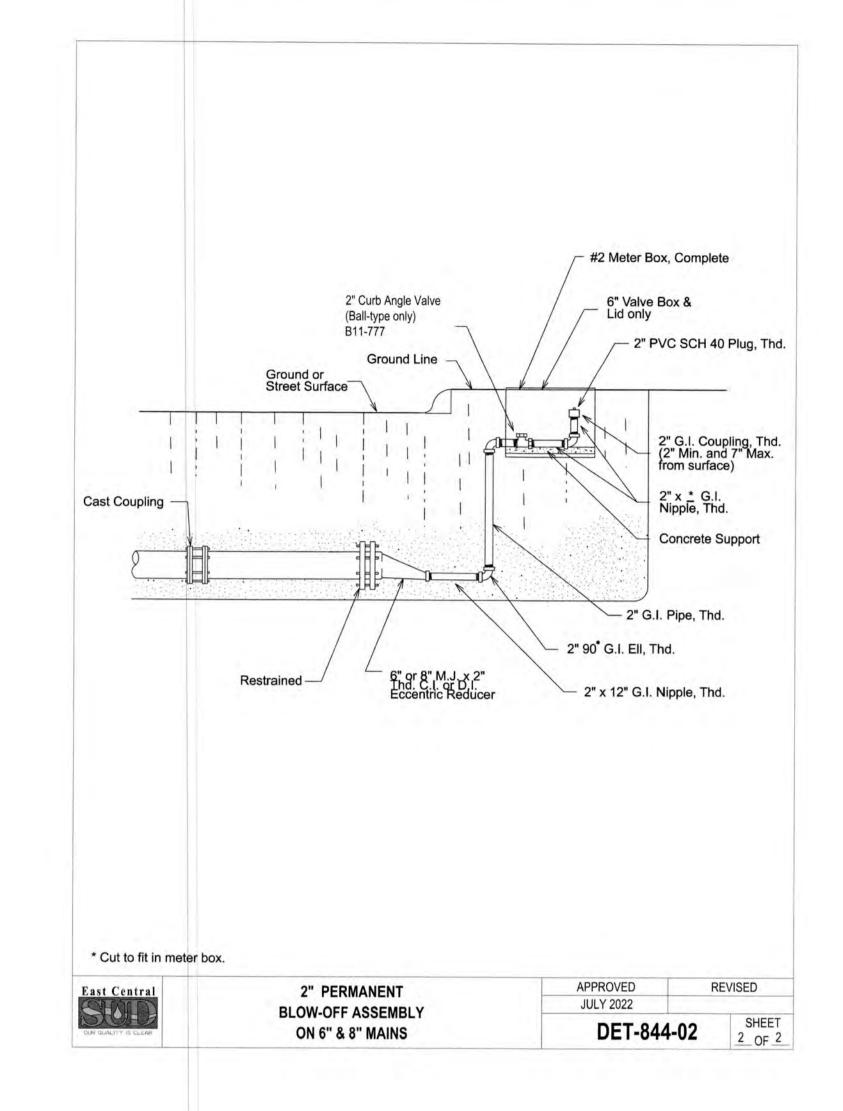


REVISED JULY 2022 **DET-839-08** 

PLAT NO. 22-11800017 JOB NO.:205-43-05 DATE: NOVEMBER 2021 DRAWN:OM CHECKED:CL

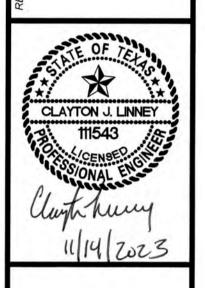
SHEET NUMBER:





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DISTRIBUTION PLAN DETAILS (SHEE

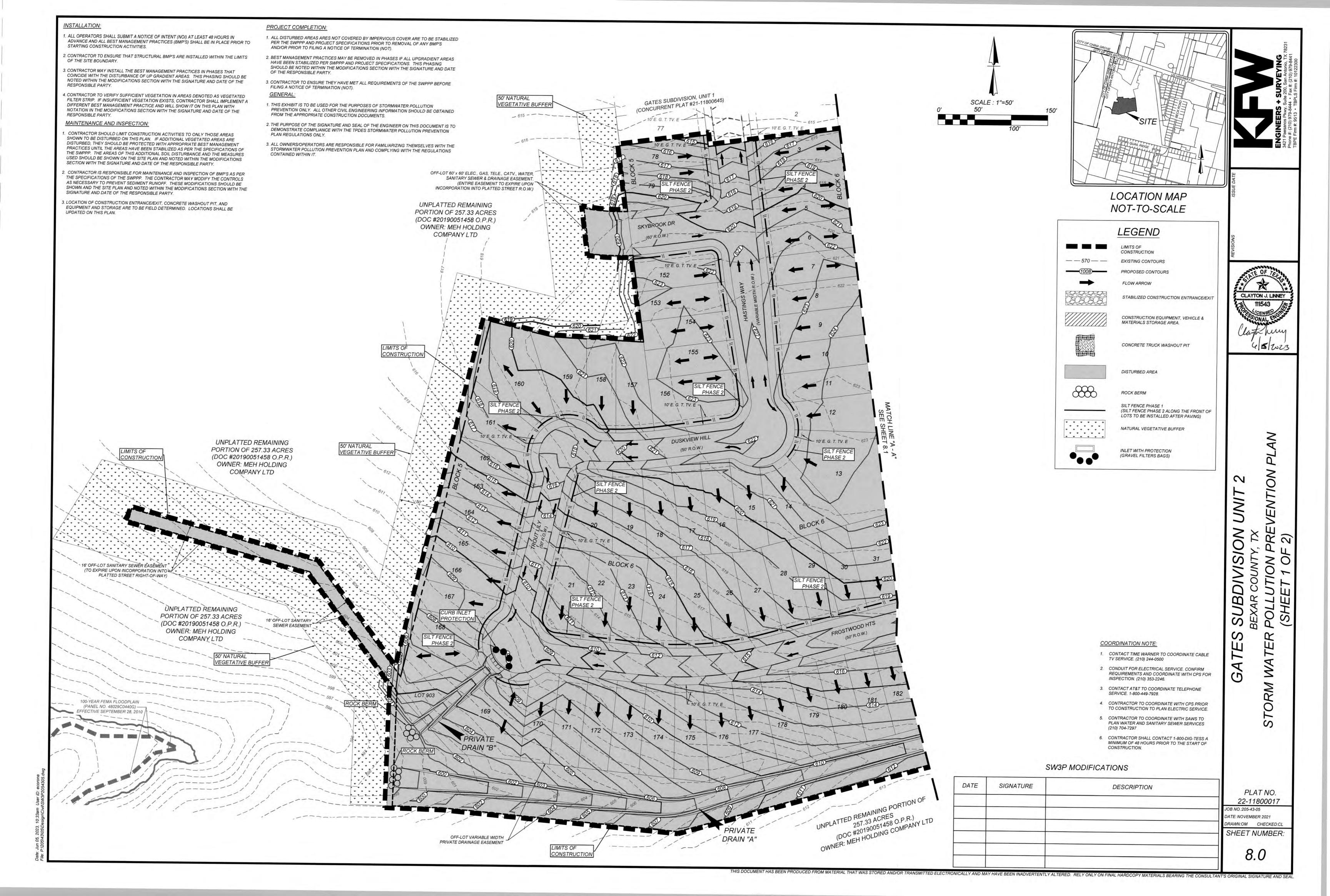
GATES SUBDIVISION UNIT 2 BEXAR COUNTY, TX

PLAT NO. 22-11800017 JOB NO.:205-43-05 DATE: NOVEMBER 2021

DATE: NOVEMBER 2021

DRAWN:OM CHECKED:CL

SHEET NUMBER:



#### INSTALLATION:

- 1. ALL OPERATORS SHALL SUBMIT A NOTICE OF INTENT (NOI) AT LEAST 48 HOURS IN ADVANCE AND ALL BEST MANAGEMENT PRACTICES (BMP'S) SHALL BE IN PLACE PRIOR TO STARTING CONSTRUCTION ACTIVITIES.
- 2. CONTRACTOR TO ENSURE THAT STRUCTURAL BMP'S ARE INSTALLED WITHIN THE LIMITS OF THE SITE BOUNDARY.
- 3. CONTRACTOR MAY INSTALL THE BEST MANAGEMENT PRACTICES IN PHASES THAT COINCIDE WITH THE DISTURBANCE OF UP GRADIENT AREAS. THIS PHASING SHOULD BE NOTED WITHIN THE MODIFICATIONS SECTION WITH THE SIGNATURE AND DATE OF THE
- 4. CONTRACTOR TO VERIFY SUFFICIENT VEGETATION IN AREAS DENOTED AS VEGETATED FILTER STRIP. IF INSUFFICIENT VEGETATION EXISTS, CONTRACTOR SHALL IMPLEMENT A DIFFERENT BEST MANAGEMENT PRACTICE AND WILL SHOW IT ON THIS PLAN WITH NOTATION IN THE MODIFICATIONS SECTION WITH THE SIGNATURE AND DATE OF THE RESPONSIBLE PARTY.

#### PROJECT COMPLETION:

- 1. ALL DISTURBED AREAS ARES NOT COVERED BY IMPERVIOUS COVER ARE TO BE STABILIZED PER THE SWPPP AND PROJECT SPECIFICATIONS PRIOR TO REMOVAL OF ANY BMP'S AND/OR PRIOR TO FILING A NOTICE OF TERMINATION (NOT).
- 2. BEST MANAGEMENT PRACTICES MAY BE REMOVED IN PHASES IF ALL UPGRADIENT AREAS HAVE BEEN STABILIZED PER SWPPP AND PROJECT SPECIFICATIONS. THIS PHASING SHOULD BE NOTED WITHIN THE MODIFICATIONS SECTION WITH THE SIGNATURE AND DATE
- 3. CONTRACTOR TO ENSURE THEY HAVE MET ALL REQUIREMENTS OF THE SWPPP BEFORE FILING A NOTICE OF TERMINATION (NOT).

PHASE 2

183

CONSTRUCTION

- 1. THIS EXHIBIT IS TO BE USED FOR THE PURPOSES OF STORMWATER POLLUTION PREVENTION ONLY. ALL OTHER CIVIL ENGINEERING INFORMATION SHOULD BE OBTAINED FROM THE APPROPRIATE CONSTRUCTION DOCUMENTS.
- 2. THE PURPOSE OF THE SIGNATURE AND SEAL OF THE ENGINEER ON THIS DOCUMENT IS TO DEMONSTRATE COMPLIANCE WITH THE TPDES STORMWATER POLLUTION PREVENTION
- 3. ALL OWNERS/OPERATORS ARE RESPONSIBLE FOR FAMILIARIZING THEMSELVES WITH THE STORMWATER POLLUTION PREVENTION PLAN AND COMPLYING WITH THE REGULATIONS

CONSTRUCTION

#### MAINTENANCE AND INSPECTION:

GATES SUBDIVISION, UNIT 1 (CONCURRENT PLAT #21-11800645)

> FROSTWOOD HTS VARIABLE WIDTH R.O.W.)

> > BLOCK 5

PRIVATE DRAINAGE EASEMENT

192

UNPLATTED KEMAINING POK.

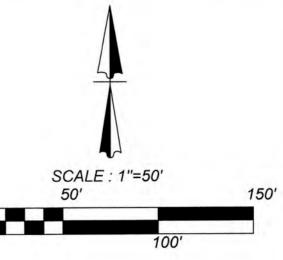
257.33 ACRES

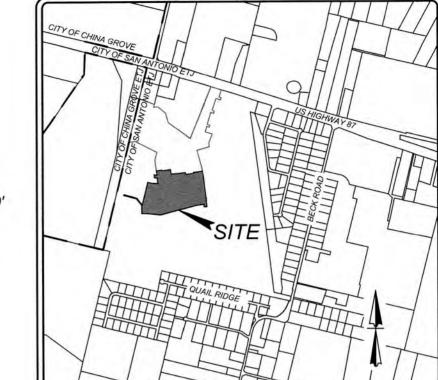
257.33 ACRES

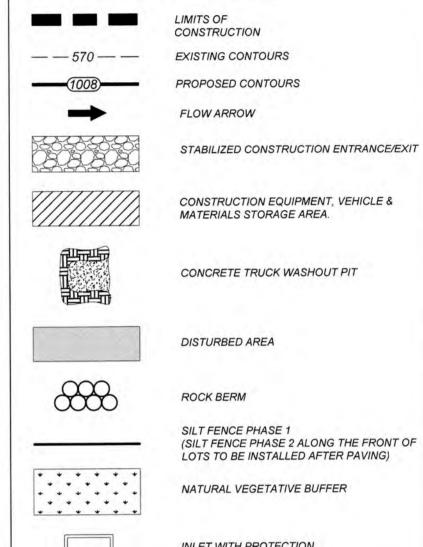
(DOC #20190051458 O.P.R.)

- 1. CONTRACTOR SHOULD LIMIT CONSTRUCTION ACTIVITIES TO ONLY THOSE AREAS SHOWN TO BE DISTURBED ON THIS PLAN. IF ADDITIONAL VEGETATED AREAS ARE DISTURBED, THEY SHOULD BE PROTECTED WITH APPROPRIATE BEST MANAGEMENT PRACTICES UNTIL THE AREAS HAVE BEEN STABILIZED AS PER THE SPECIFICATIONS OF THE SWPPP. THE AREAS OF THIS ADDITIONAL SOIL DISTURBANCE AND THE MEASURES USED SHOULD BE SHOWN ON THE SITE PLAN AND NOTED WITHIN THE MODIFICATIONS SECTION WITH THE SIGNATURE AND DATE OF THE RESPONSIBLE PARTY.
- 2. CONTRACTOR IS RESPONSIBLE FOR MAINTENANCE AND INSPECTION OF BMP'S AS PER THE SPECIFICATIONS OF THE SWPPP. THE CONTRACTOR MAY MODIFY THE CONTROLS AS NECESSARY TO PREVENT SEDIMENT RUNOFF. THESE MODIFICATIONS SHOULD BE SHOWN AND THE SITE PLAN AND NOTED WITHIN THE MODIFICATIONS SECTION WITH THE SIGNATURE AND DATE OF THE RESPONSIBLE PARTY.
- 3. LOCATION OF CONSTRUCTION ENTRANCE/EXIT, CONCRETE WASHOUT PIT, AND EQUIPMENT AND STORAGE ARE TO BE FIELD DETERMINED. LOCATIONS SHALL BE UPDATED ON THIS PLAN.

CONSTRUCTION ENTRANCE/EXIT







CLAYTON J. LINNEY

- CONTACT TIME WARNER TO COORDINATE CABLE
- 2. CONDUIT FOR ELECTRICAL SERVICE. CONFIRM REQUIREMENTS AND COORDINATE WITH CPS FOR INSPECTION. (210) 353-2246.
- 3. CONTACT AT&T TO COORDINATE TELEPHONE
- 4. CONTRACTOR TO COORDINATE WITH CPS PRIOR
- PLAN WATER AND SANITARY SEWER SERVICES (210) 704-7297
- CONTRACTOR SHALL CONTACT 1-800-DIG-TESS A MINIMUM OF 48 HOURS PRIOR TO THE START OF CONSTRUCTION.

	37731 1		
DATE	SIGNATURE	DESCRIPTION	PLAT NO. 22-11800017
			JOB NO.:205-43-05  DATE: NOVEMBER 2021  DRAWN:OM CHECKED:CL
			SHEET NUMBER:
			8.1

GATES SUBDIVISION, UNIT 1 (CONCURRENT PLAT #21-11800645) EQUIPMENT VEHICLE & MATERIALS LOCATION MAP CONSTRUCTION STORAGE AREA NOT-TO-SCALE LEGEND PHASE 2 UNPLATTED REMAINING PORTION OF 257.33 ACRES INLET WITH PROTECTION (DOC #20190051458 O.P.R.) (GRAVEL FILTERS BAGS) OWNER: MEH HOLDING COMPANY LTD

BLOCK 5

CONSTRUCTION

OFF-LOT 60' x 60' ELEC., GAS, TELE., CATV., WATER,

INCORPORATION INTO PLATTED STREET R.O.W.)

SANITARY SEWER, & DRAIN MAINTENANCE

(ENTIRE EASEMENT TO EXPIRE UPON

- ACCESS EASEMENT.

**COORDINATION NOTE:** 

TV SERVICE. (210) 244-0500

SERVICE. 1-800-449-7928.

TO CONSTRUCTION TO PLAN ELECTRIC SERVICE. 5. CONTRACTOR TO COORDINATE WITH SAWS TO

SW3P MODIFICATIONS

LINING FREE OF HOLES, TEARS, OR OTHER DEFECTS

ISOMETRIC PLAN VIEW

SILT FENCE

SECTION A-A

SECTION B-B

SECTION A-A

TYPE "ABOVE GRADE"

DETAIL ABOVE ILLUSTRATES MINIMUM DIMENSIONS. PIT CAN BE INCREASED IN SIZE DEPENDING ON EXPECTED FREQUENCY OF USE.

WASHOUT PIT SHALL BE LOCATED IN AN AREA EASILY ACCESSIBLE TO

FROM STORM WATER RUNOFF AND AT LEAST 50 FEET FROM SENSITIVE

CONCRETE TRUCK WASHOUT PIT

THE FILTER BAG MATERIAL SHALL BE MADE OF POLYPROPYLENE, POLYETHYLENE OR POLYAMIDE WOVEN FABRIC, MIN UNIT WEIGHT OF 4

OUNCES/SY, MULLEN BURST STRENGTH EXCEEDING 300 PSI AND ULTRAVIOLET STABILITY EXCEEDING 70%.

THE FILTER BAG SHALL BE FILLED WITH CLEAN, MEDIUM TO COARSE GRAVEL (0.31

FEATURES, STORM DRAINS, OPEN DITCHES, STREETS, OR STREAMS.

WASHOUT PIT SHALL NOT BE LOCATED IN AREAS SUBJECT TO INUNDATION

**GENERAL NOTES:** 

(1) Silt fence material should be polypropylene, polyethylene or polyamide woven or nonwoven fabric. The fabric width should be 36 inches, with a minimum unit weight of 4.5 oz/yd, mullen burst strength exceeding 190 lb/in2, ultraviolet stability exceeding 70%, and minimum apparent opening size of U.S. Sieve No.

(2) Fence posts should be made of hot rolled steel, at least 4 feet long with Tee or Ybar cross section, surface painted or galvanized, minimum nominal weight 1.25 lb/fl2, and Brindell hardness exceeding 140. (3) Woven wire backing to support the fabric should be galvanized 2" x 4" welded wire, 12 gauge minimum.

(1) Steel posts, which support the silt fence, should be installed on a slight angle toward the anticipated runoff source. Post must be embedded a minimum of 1- foot deep and spaced not more than 8 feet on center. Where water concentrates, the maximum spacing (2) Lay out fencing down-slope of disturbed area, following the contour as closely as

possible. The fence should be sited so that the maximum drainage area is 1/4 acre/i 00 feet of fence. (3) The toe of the silt fence should be trenched in with a spade or mechanical trencher, so that the down- slope face of the trench is flat and perpendicular to the line of flow. Where fence cannot be trenched in (e.g., pavement or rock outcrop), weight fabric flap with 3 inches of pea gravel on uphill side to prevent flow from seeping under fence. (4) The trench must be a minimum of 6 inches deep and 6 inches wide to allow for the silt fence fabric to be laid in the ground and backfilled with compacted material. (5) Silt fence should be securely fastened to each steel support post or to woven wire,

which is in turn attached to the steel fence post. There should be a 3-foot overlap, securely fastened where ends of fabric meet. (6) Silt fence should be removed when the site is completely stabilized so as not to block or impede stone flow or drainage.

Inspection and Maintenance Guidelines:
(1) Inspect all fencing weekly, and after any rainfall. (2) Remove sediment when buildup reaches 6 inches. (3) Replace any torn fabric or install a second line of fencing parallel to the torn section. (4) Replace or repair any sections crushed or collapsed in the course of construction activity. If a section of fence is obstructing vehicular access, consider relocating it to a spot

where it will provide equal protection, but will not obstruct vehicles. A triangular filter dike may be preferable to a silt fence at common vehicle access points. (5) When construction is complete, the sediment should be disposed of in a manner that will not cause additional siltation and the prior location of the silt fence should be revegetated. The fence itself should be disposed of in an approved landfill.

THE MATERIAL, INSTALLATION, INSPECTION, AND MAINTENANCE OF FIBER ROLLS WILL BE PER THE MANUFACTURE'S SPECIFICATIONS AND SHALL ALSO COMPLY WITH THE TEXAS COMMISSION OF ENVIRONMENTAL QUALITY CURRENT "TECHNICAL GUIDANCE ON BEST MANAGEMENT PRACTICES" AS NOTED BELOW.

(1) Core material: Core material should be biodegradable or recyclable. Material may be compost, mulch, aspen wood fibers, chipped site vegetation, agricultural rice or wheat straw, coconut fiber, 100% recyclable fibers, or similar materials.

(2) Containment Mesh: Containment mesh should be 100% biodegradable, photodegradable or recyclable such as burlap, twine, UV photodegradable plastic, polyester, or similar material. When the fiber role will remain in place as part of a vegetative system use biodegradable or photodegradable mesh. For temporary installation recyclable mesh is recommended.

(1) Locate fiber rolls on level contours spaced as follows: Slope inclination of 4:1 (H:V) or flatter: Fiber rolls should be placed at a

maximum interval of 20 ft. Slope inclination between 4:1 and 2:1 (H:V): Fiber Rolls should be placed at a maximum interval of 15 ft. (a closer spacing is more effective). Slope inclination 2:1 (H:V) or greater: Fiber Rolls should be placed at a maximum interval of 10 ft. (a closer spacing is more effective). (2) Turn the ends of the fiber roll up slope to prevent runoff from going around the

(3) Stake fiber rolls into a 2 to 4 in. deep trench with a width equal to the diameter of the fiber roll. (4) Drive stakes at the end of each fiber roll and spaced 4 ft maximum on center. (5) Use wood stakes with a nominal classification of 0.75 by 0.75 in. and minimum

(6) If more than one fiber roll is placed in a row, the rolls should be overlapped, not

Inspection and Maintenance Guidelines: (1) Inspect prior to forecast rain, daily during extended rain events, after rain events,

(2) Repair or replace split, torn, unraveling, or slumping fiber rolls. (3) If the fiber roll is used as a sediment capture device, or as an erosion control device to maintain sheet flows, sediment that accumulates behind the role must be periodically removed in order to maintain its effectiveness, Sediment should be removed when the accumulation reaches one-half the designated sediment storage depth, usually one-half the distance between the top of the fiber roll and the adjacent ground surface. Sediment removed during maintenance may be incorporated into earthwork on the site or disposed of at an appropriate location.

CONSTRUCTION

**EQUIPMENT** & VEHICLE STORAGE MAINTENANCE

CONSTRUCTION

AND WASTE

MATERIAL

STORAGE

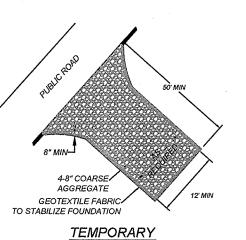
AREA

FABRIC CROSS-SECTION OF A CONSTRUCTION ENTRANCE/EXIT

<u>Materials:</u>

(1) The aggregate should consist of 4 to 8 inch washed stone over a stable foundation as specified in the plan. (2) The aggregate should be placed with a minimum thickness of 8 inches. (3) The geotextile fabric should be designed specifically for use as a soil filtration media with an approximate weight of 6 oz/yd2, a mullen burst rating of 140 lb/in2, and an equivalent opening size greater than a number

(4) If a washing facility is required, a level area with a minimum of 4 inch diameter washed stone or commercial rack should be included in the plans. Divert wastewater to a sediment trap or basin.



CONSTRUCTION ENTRANCE/EXIT

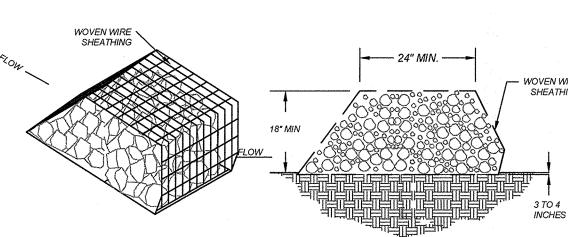
Installation: (North Carolina, 1993) (1) Avoid curves on public roads and steep slopes. Remove vegetation and other objectionable material from the foundation area. Grade crown foundation for positive drainage. (2) The minimum width of the entrance/exit should be 12 feet or the full width of exit roadway, whichever is greater. (3) The construction entrance should be at least 50 feet long. (4) If the slope toward the road exceeds 2%, construct a ridge, 6 to 8 inches high with 3:1 (H:V) side slopes, across the foundation approximately 15 feet from the entrance to divert runoff away from the public road. (5) Place geotextile fabric and grade foundation to improve stability, especially where wet conditions are anticipated. (6) Place stone to dimensions and grade shown on plans. Leave surface smooth and slope for drainage.

(7) Divert all surface runoff and drainage from the stone pad to a sediment trap or basin.

(8) Install pipe under pad as needed to maintain proper public road drainage.

The entrance should be maintained in a condition, which will prevent tracking or flowing of sediment onto public rights-of-way. This may require periodic top dressing with additional stone as conditions demand and repair andlor cleanout of anv measures used to trap sediment. (2) All sediment spilled, dropped, washed or tracked onto public rights-of-way should be removed immediately by contractor. (3) When necessary, wheels should be cleaned to remove sediment prior to entrance onto public right-of-way. (4) When washing is required, it should be done on an area stabilized with crushed stone that drains into an approved sediment (5) All sediment should be prevented from entering any storm drain, ditch or water course by using approved methods.

STABILIZED CONSTRUCTION ENTRANCE / EXIT



(1) The berm structure should be, secured with a woven wire sheathing having maximum opening of 1

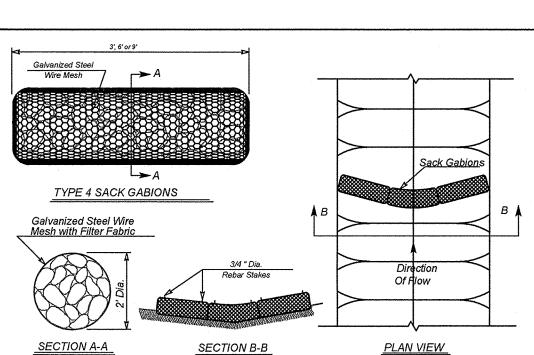
or large volumes of flow are expected, where 5- to 8-inch diameter rocks may be used. (1) Lay out the woven wire sheathing perpendicular to the flow line. The sheathing should be 20 gauge

(4) Wrap the wire sheathing around the rock and secure with tie wire so that the ends of the sheathing overlap at least 2 inches, airl the berm retains its shape when walked upon. (5) Berm should be built along the contour at zero percent grade or as near as possible. (6) The ends of the berm should be tied into existing upslope grade and the berm should be buried in a

Inspection and Maintenance Guidelines: (1) Inspection should be made weekly and after each rainfall by the responsible party. For installations in streambeds, additional daily inspections should be made. (2) Remove sediment and other debris when buildup reaches 6 inches and dispose of the accumulated silt in an approved manner that will not cause any additional siltation.

(4) The berm should be reshaped as needed during inspection. (5) The berm should be replaced when the structure ceases to function as intended due to silt accumulation among the rocks, washout, construction traffic damage, etc. (6) The rock berm should be left in place until all upstream areas are stabilized and accumulated silt

ROCK BERM



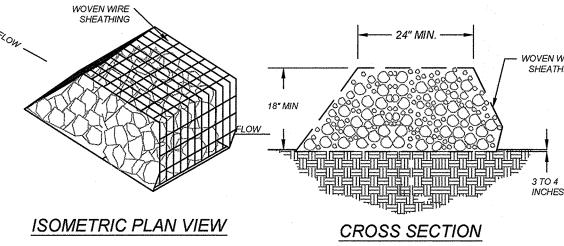
THE TOP OF THE SACK GABIONS SHOULD BE LEVEL AND ORIENTED PERPENDICULAR TO THE DIRECTION OF FLOW.

FILTER FABRIC MATERIAL SHALL BE FASTENED TO WOVEN WIRE FILTER FABRIC MATERIAL SHOULD MEET THE FOLLOWING

SPECIFICATIONS: RESISTANT TO ULTRAVIOLET LIGHT, FABRIC SHOULD BE NON-WOVEN GEOTEXTILE WITH MINIMUM WEIGHT OF 3.5 UNCES PER SQUARE YARD, MINIMUM MULLEN BURST STRENGTH OF 200 POUNDS PER SQUARE INCH AND A FLOW THRU RATE OF 120 GALLONS PER MINUTE PER SQUARE FOOT OF FRONTAL AREA. STONE SIZE: ±4"-8" OPEN GRADED CRUSHED LIMESTONE.

INSPECT WEEKLY OR AFTER EACH RAINFALL EVENT AND REPAIR OR

WHEN SILT REACHES A DEPTH OF 6 INCHES OR MORE ABOVE NATURAL GROUND, SILT SHALL BE REMOVED AND DISPOSED IN AN APPROVED MANNER THAT WILL NOT CONTRIBUTE TO RESILTATION. OFF-SITE IN ACCORDANCE WITH APPLICABLE REGULATIONS.

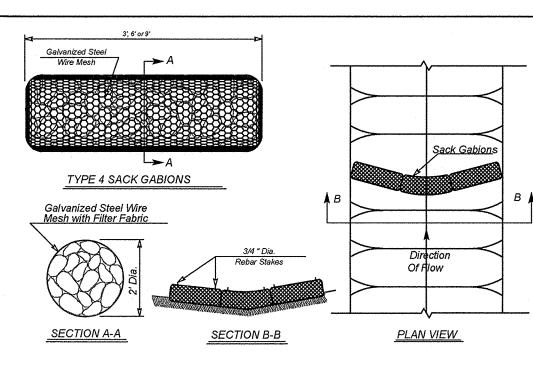


inch and a minimum wire diameter of 20 gauge galvanized and should be secured with shoat rings. (2) Clean, open graded 3- to 5-inch diameter rock should be used, except in areas where high velocities

woven wire mesh with 1 inch openings. (2) Berm should have a top width of 2 feet minimum with side slopes being 2:1 (H:V) or flatter. (3) Place the rock along the sheathing as shown in the diagram Figure 1-28), to a height not less than

trench approximately 3 to 4 inches deep to prevent failure of the control.

(3) Repair any loose wire sheathing.



TYPE 4 SACK GABIONS

GRAVEL FILTER BAG DETAIL

**GENERAL NOTES:** 

TO 0.75 INCH DIAMETER).

GRAVEL FILTER BAG -WITH FILTER FABRIC <u>PLAN VIEW</u> DETAIL-A -GRAVEL FILTER BAG -WIRE MESH COVERED WITH FILTER FABRIC WITH FILTER FABRIC SECTION A-A CURB INLET PROTECTION (ALTERNATE)

CURB INLET PROTECTION GRAVEL FILTER BAGS

► FILTERED RUNOFF

SECTION A-A

ALL STORM DRAINAGE SYSTEMS INLETS SHOULD FILTER RUNOFF BEFORE

THE WATER IS DISCHARGED INTO STREAMS OR ONTO ADJACENT PROPERTIES, UNLESS TREATMENT IS PROVIDED ELSEWHERE.

IF NO ADDITIONAL DOWNSTREAM TREATMENT EXISTS, THE MAXIMUM DRAINAGE AREA TRIBUTARY TO AN AREA DRAIN INSTALLED WITH A GRAVEL

ALL CURB INLET GRAVEL FILTERS SHOULD BE INSPECTED AND REPAIRED AFTER EACH RUNOFF EVENT. SEDIMENT SHOULD BE REMOVED WHEN MATERIAL IS WITHIN THREE INCHES OF THE TOP OF THE CONCRETE BLOCKS.

PERIODICALLY, THE GRAVEL SHOULD BE RAKED TO INCREASE INFILTRATION AND FILTERING OF RUNOFF WATERS.

GENERAL NOTES:

FILTER SHOULD BE ONE ACRE.

Vertical spacing

varies between

10' and 20'

measured along the face of the slope

TYPICAL FIBER ROLL INSTALLATION

ENTRENCHMENT DETAIL

Install fiber roll

FIBER ROLL

<u>PLAN VIEW</u> SOD PLACED IN CHECKER BOARD PATTERN SECTION A-A CHANNEL TO BE STABILIZED WITH SOD PLACED IN A CHECKER BOARD PATTERN TON THE CHANNEL BOTTOM AND ON THE SIDES UP TO 1/3 THE DEPTH OF

/EXIT

TYPICAL CONSTRUCTION STAGING AREA

CHANNEL LINING

PLAT NO. 22-11800017 OB NO.:205-43-05

PATE: NOVEMBER 2021 PRAWN:OM CHECKED:CL SHEET NUMBER:

OFFICE

FLOW ARROWS

THIS DOCUMENT HAS BEEN PRODUCED FROM MATERIAL THAT WAS STORED AND/OR TRANSMITTED ELECTRONICALLY AND MAY HAVE BEEN INADVERTENTLY ALTERED. RELY ONLY ON FINAL HARDCOPY MATERIALS BEARING THE CONSULTANT'S ORIGINAL SIGNATURE AND SEAL.

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