# GENERAL NOTES

WITH CITY OF CONVERSE AMENDMENTS.

THE DESIGN LOADS ARE AS FOLLOWS:

ROOF LIVE LOAD WIND LOAD BASIC WIND SPEED WIND EXPOSURE RISK CATEGORY	
SNOW LOAD	
SEISMIC LOAD	
SEISMIC LOAD SEISMIC DESIGN CATEGOR RISK CATEGORY SITE CLASS Ss S1 SMS SM1 SDS SD1	Y
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F <sub>v</sub>	
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THE STRUCTURE IS DESIGNED FOR A BASIC WIND SPEED OF Vasd = 83 MPH, Vult = 107 MPH EXPOSURE B AND RISK CATEGORY II. REFER TO SECTION 1609 OF THE INTERNATIONAL BUILDING CODE FOR THE APPLICABLE COEFFICIENTS TO DETERMINE THE ACTUAL WIND PRESSURES ON THE VARIOUS STRUCTURAL COMPONENTS.

THE FOUNDATION FOR THE STRUCTURE HAS BEEN DESIGNED BASED ON THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER, INTEGRATED TESTING AND ENGINEERING COMPANY OF SAN ANTONIO, AS PRESENTED IN THEIR REPORT NUMBER S241748 AND DATED OCTOBER 16, 2024. THE FOLLOWING ALLOWABLE BEARING PRESSURES WERE USED TO PROPORTION THE FOUNDATION BEARING ON UNTREATED SOIL AT A DEPTH OF TWENTY FIVE FEET (25' - 0") BELOW EXISTING GRADE UNLESS NOTED OTHERWISE.

## END BEARING SKIN FRICTION

SHALL BE REPORTED IMMEDIATELY TO THE STRUCTURAL ENGINEER.

PRINCIPAL OPENINGS ARE SHOWN ON THE DRAWINGS. REFER TO ARCHITECTURAL, MECHANICAL ELECTRICAL AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS, SMALL OPENINGS, ETC.

PROVIDE CRACK CONTROL JOINTS AT SLAB ON GRADE AREAS AS SHOWN ON STRUCTURAL DRAWINGS. REFER TO SHEET S2.1 FOR TYPICAL JOINT DETAILS.

CONCRETE IN THE FOLLOWING AREAS SHALL HAVE NATURAL SAND FINE AGGREGATES, NORMAL WEIGHT COARSE AGGREGATES CONFORMING TO ASTM C33, TYPE I PORTLAND CEMENT, AND SHALL HAVE THE FOLLOWING DESIGNATED COMPRESSIVE STRENGTH (fc) IN 28 DAYS:

> DRILLED FOOTINGS 3000 PSI GRADE BEAMS, TIE BEAMS, & PLINTHS 3000 PSI SLAB ON GRADE (WATER TO CEMENT RATIO < 0.50) 4000 PSI

THE SLAB ON GRADE SHALL BE THICKNESS NOTED ON PLAN AND REINFORCED IN ACCORDANCE WITH THE SLAB NOTES ON THE FOUNDATION PLAN. REFER TO TYPICAL SLAB ON GRADE CONTROL AND CONSTRUCTION JOINT DETAILS FOR REINFORCEMENT PLACEMENT, ETC. SLAB SHALL BE CAST ON A VAPOR RETARDER ON A PREPARED SUBGRADE. THE SUBGRADE SHALL BE PREPARED AS SPECIFIED IN THE GEOTECHNICAL REPORT, RESULTING IN A MAXIMUM POTENTIAL VERTICAL RISE OF ONE INCH.

CORNER BARS SHALL BE PROVIDED AT ALL GRADE BEAM CORNERS AND INTERSECTIONS. BARS SHALL BE PLACED TOP AND BOTTOM AND SHALL BE 4 FOOT LONG BENT 90 DEGREES AT THE CENTER AND SIZED TO MATCH THE GRADE BEAM REINFORCING.

CONCRETE REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60. NUMBER THREE BARS MAY CONFIRM TO ASTM A615, GRADE 40.

NOTED OTHERWISE.

REINFORCING BARS MAY NOT BE WELDED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE STANDARDS AS PRESCRIBED IN ACI 301-10. MIXING, TRANSPORTING, PLACING AND CURING OF CONCRETE SHALL BE IN ACCORDANCE WITH ACI 301-10.

CONCRETE COVER PROTECTION FOR REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF ACI 318-19.

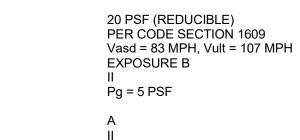
HORIZONTAL JOINTS WILL NOT BE PERMITTED IN CONCRETE CONSTRUCTION, EXCEPT AS SHOWN ON THE STRUCTURAL DRAWINGS. ALL CONSTRUCTION JOINTS SHALL BE MADE WITH A VERTICAL BULKHEAD AND KEY WAY AT LOCATIONS APPROVED BY THE STRUCTURAL ENGINEER.

ANCHOR BOLTS SHALL CONFORM TO ASTM A307, UNLESS NOTED OTHERWISE.

THE ARCHITECTURAL GLAZING SYSTEMS SHALL BE DESIGNED BY A REGISTERED ENGINEER LICENSED IN THE STATE OF TEXAS. THE DESIGN CALCULATIONS SHALL BE PREPARED BY THE ENGINEER AND SUBMITTED WITH GLAZING SHOP DRAWINGS FOR APPROVAL.

THE SUPERSTRUCTURE SHALL BE A PRE-ENGINEERED METAL BUILDING DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF TEXAS. THE SUPERSTRUCTURE SHALL BE DESIGNED PER BUILDING CODE PRESCRIBED GRAVITY AND LATERAL LOADS. THE BASES OF MAIN FRAMES SHALL BE DESIGNED AS "PIN BASES" SUCH THAT NO SIGNIFICANT BENDING MOMENT IS TRANSFERRED TO THE FOUNDATION. METAL BUILDING SHOP DRAWINGS SHALL INCLUDE ALL FOUNDATION REACTIONS AND ANCHOR BOLT SETTING PLAN AND SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL PRIOR TO COMMENCING FOUNDATION CONSTRUCTION.

THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE 2024 EDITION OF THE INTERNATIONAL BUILDING CODE



0.051 0.024 0.081 0.058 0.054 0.039 1.6 2.4 12

> 12,000 PSF 875 PSF

ANY FOUNDATION CONDITIONS WHICH DIFFER FROM THOSE DESCRIBED IN THE GEOTECHNICAL REPORT

REINFORCEMENT DESIGNATED AS "CONTINUOUS" SHALL LAP 36 BAR DIAMETERS AT SPLICES, UNLESS



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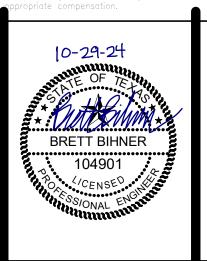
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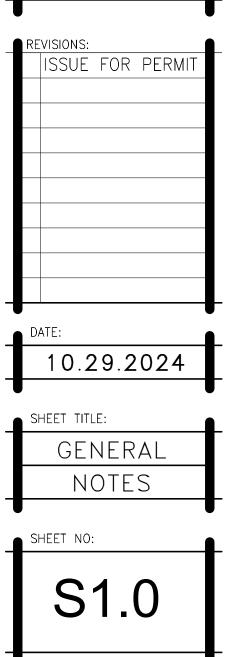
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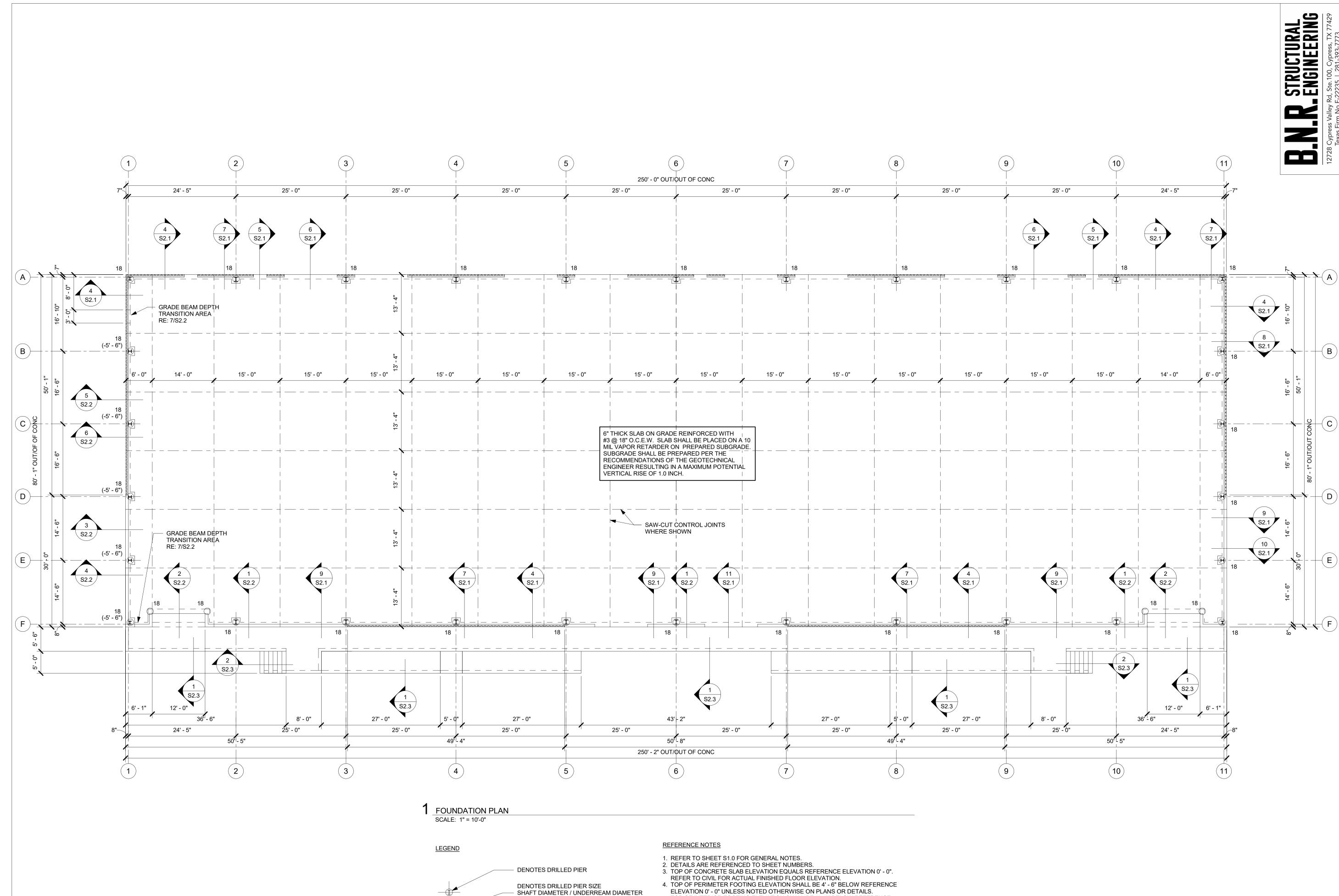


Sub Contractors shall verify and be dimensions and conditions shown on these drawings.

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(EXAMPLE SIZE SHOWN)

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- ELEVATION 0' 0" UNLESS NOTED OTHERWISE ON PLANS OR DETAILS. 5. TOP OF INTERIOR FOOTING ELEVATION SHALL BE 0' 6" BELOW REFERENCE
- ELEVATION 0' 0" UNLESS NOTED OTHERWISE ON PLANS OR DETAILS.
- 6. GENERAL CONTRACTOR SHALL SUBMIT FINAL PRE-ENGINEERED METAL BUILDING BUILDING DRAWINGS TO THE STRUCTURAL ENGINEER FOR FINAL COORDINATION PRIOR TO COMMENCEMENT OF FOUNDATION CONSTRUCTION.

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BUSINESS

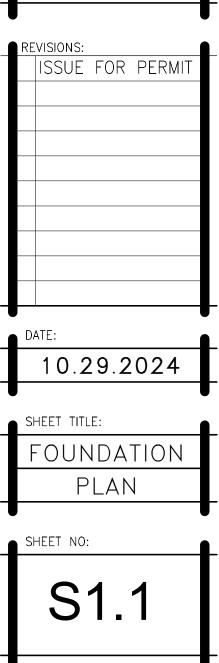
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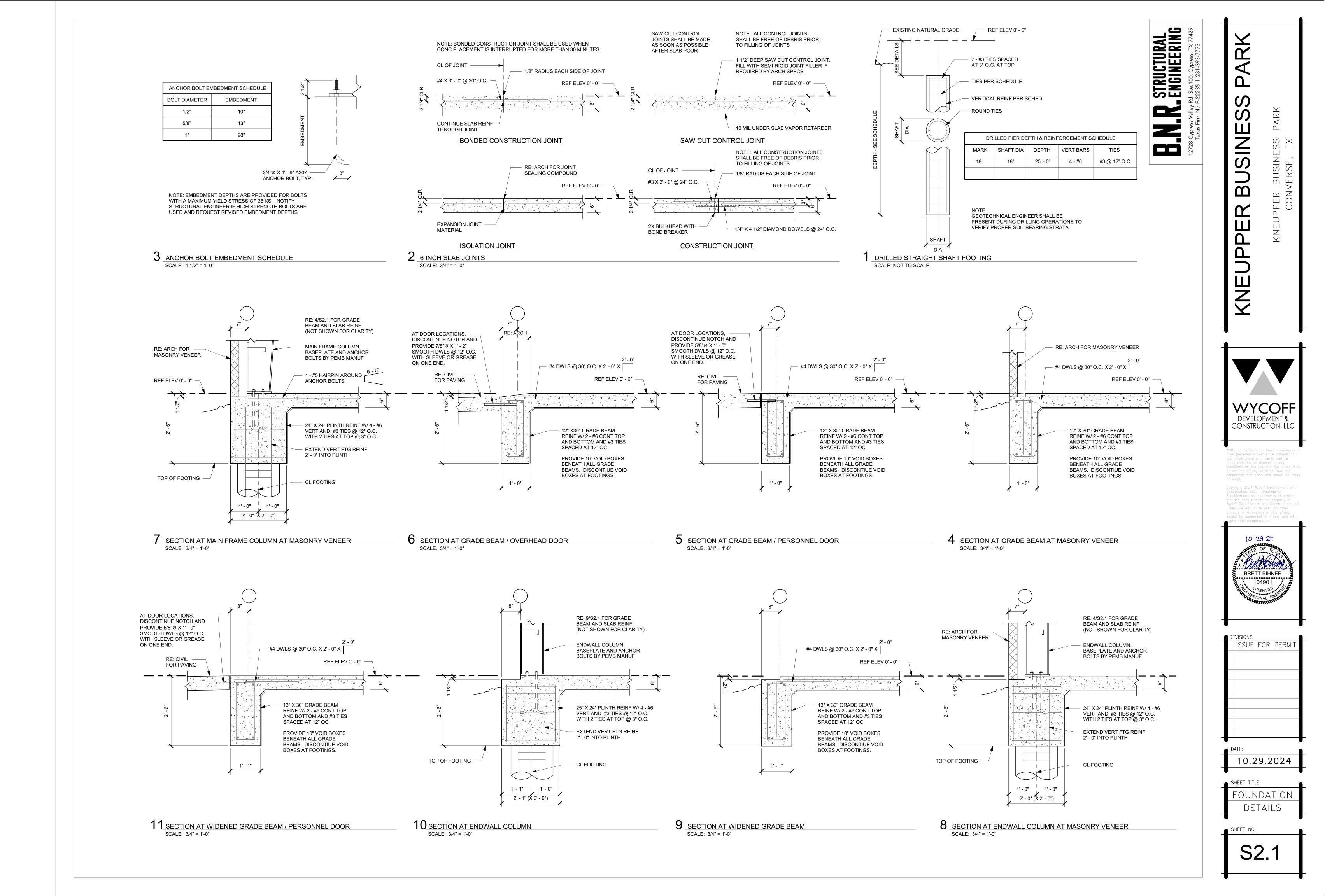
# WYCOFF DEVELOPMENT & CONSTRUCTION, LLC

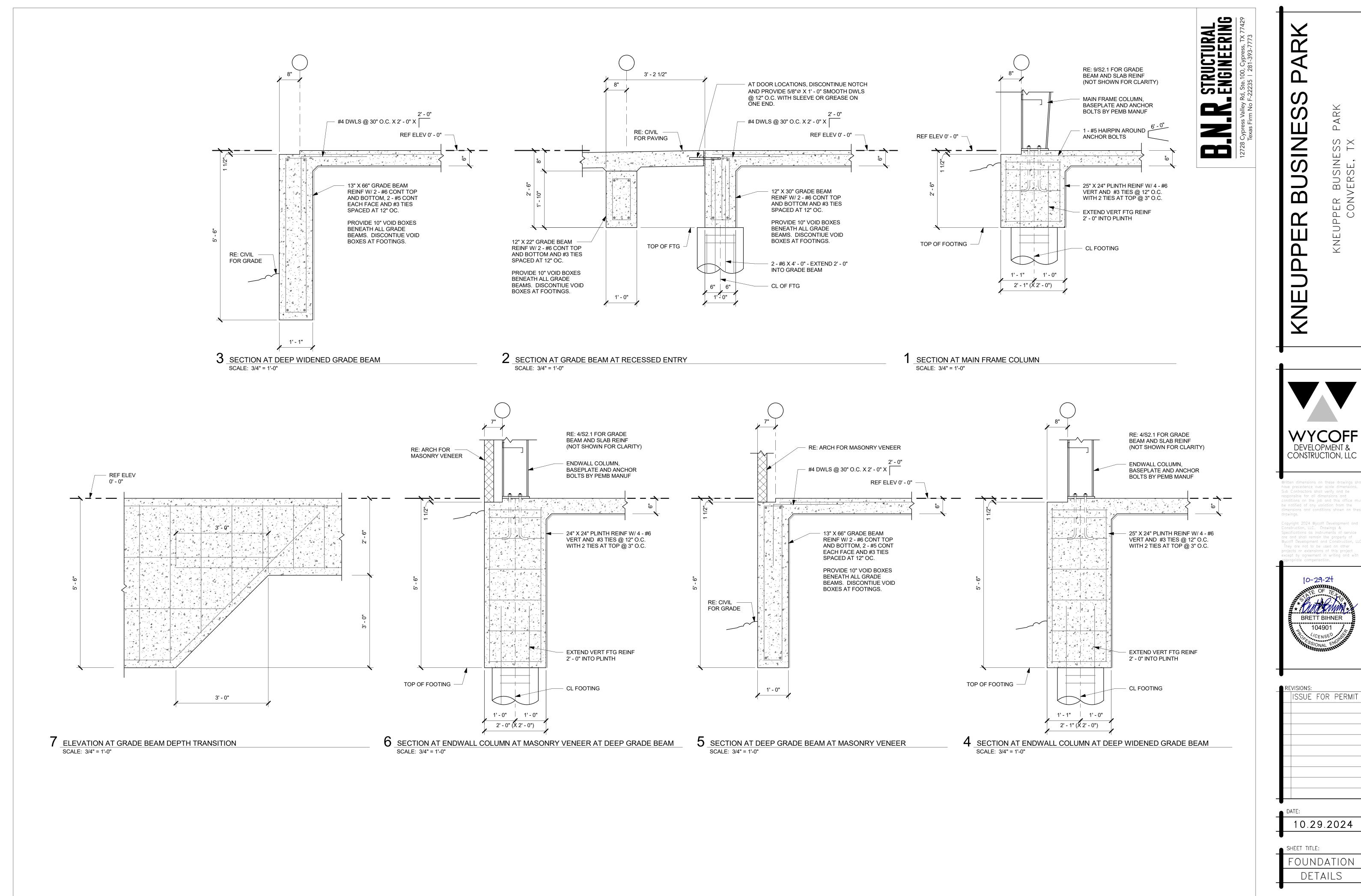
be notified of any variation from the drawings.

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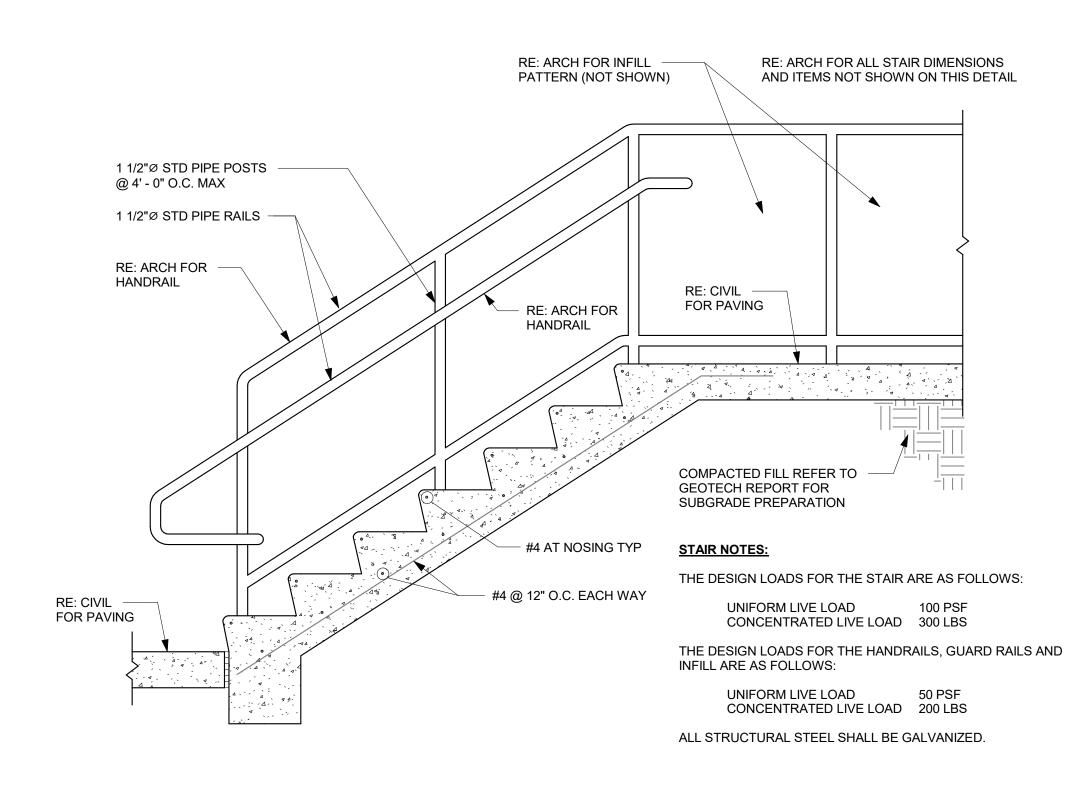






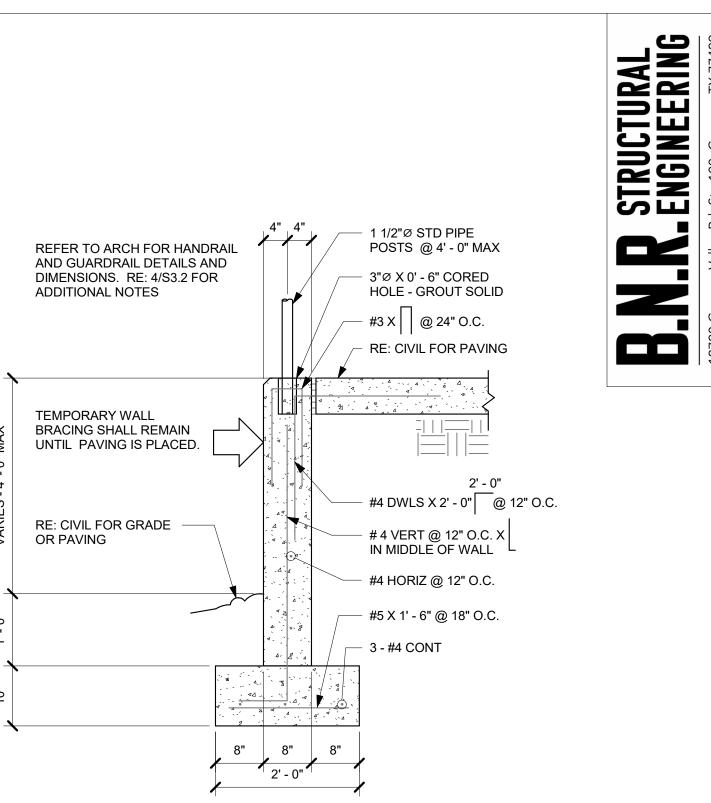
SHEET NO:

S2.2



# 2 SECTION AT CONCRETE STAIRS SCALE: 3/4" = 1'-0"

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**WYCOFF** DEVELOPMENT & CONSTRUCTION, LLC

have precedence over scale dimension Sub Contractors shall verify and be responsible for all dimensions and

be notified of any variation from the dimensions and conditions shown on these drawings.

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10-29-24

BRETT BIHNER

ISSUE FOR PERMIT

10.29.2024

FOUNDATION

DETAILS

S2.3

REVISIONS:

DATE:

SHEET TITLE:

SHEET NO:

Park

R BUSINESS Averse, tx

KNEUPPER Conv

SECTION AT RETAINING WALL SCALE: 3/4" = 1'-0"