# LEGACY AT GREEN ENCLAVE, UNIT 1

SUBMITTED BY: MOY TARIN RAMIREZ ENGINEERS, LLC. 12770 CIMARRON PATH, SUITE 100 SAN ANTONIO, TEXAS 78249 TEL: (210) 698-5051

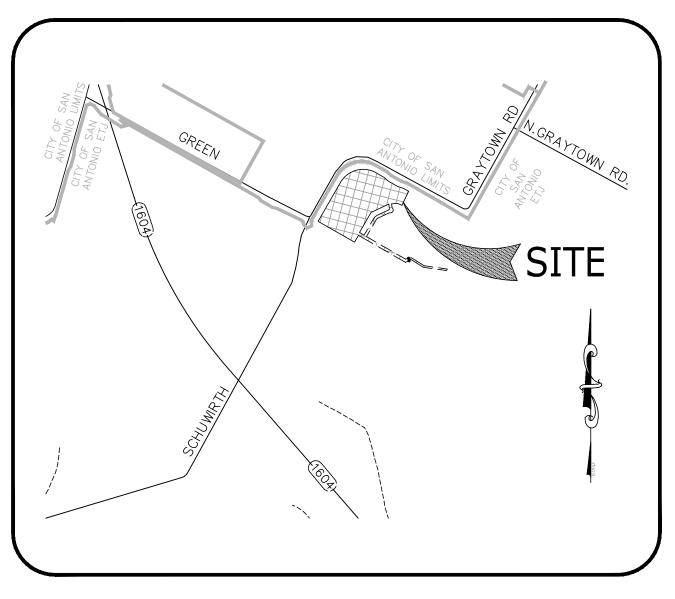
FAX: (210) 698-5085

OWNER/DEVELOPER

FOUR BROTHERS CAPITAL, LLC 85 N.E. LOOP 410, SUITE 203 SAN ANTONIO, TX 78216



# **CONSTRUCTION PLANS FOR**



VICINITY MAP N.T.S.

# SUBMITTAL DATE:

# LEGAL DESCRIPTION:

BEING A TOTAL OF 26.726 ACRE TRACT OF LAND PARTIALLY SITUATED IN THE ANDREW JF PHELAN SURVEY NO. 45, ABSTRACT NO. 580, COUNTY BLOCK 5107, AND PARTIALLY IN THE PI CO SURVEY NO. 4, ABSTRACT NO. 909, COUNTY BLOCK 5107, BOTH OF BEXAR COUNTY, TEXAS, BEING A PORTION OF A 125.588 ACRE TRACT AS CONVEYED TO HELEN RAKOWITZ BY WARRANTY DEED WITH VENDOR'S LIEN AS RECORDED IN VOLUME 1741, PAGE 299, OF THE OFFICIAL PUBLIC RECORDS OF BEXAR COUNTY, TEXAS.



## PLAT NO. 23-11800388

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SUBMITTAL SET

SW3P DETAILS

TEXAS CO.0

C6.2

### TRENCH EXCAVATION SAFETY PROTECTION Contractor and/or Contractor's independently retained employee or structural

design/geotechnical/safety/equipment consultant, if any, shall review these plans and available geotechnical information and the anticipated installation site(s) within the project work area in order to implement Contractor's trench excavation safety protection systems, programs and/or procedures for the project described in the contract documents. The Contractor's implementation of these systems, programs and/or procedures shall provide for adequate trench excavation safety protection that comply with as a minimum, OSHA standards for trench excavations. Specifically, Contractor and/or Contractor's independently retained employee or safety consultant shall implement a trench safety program in accordance with OSHA standards governing the presence and activities of individuals working in and around trench excavation.

### UTILITY GENERAL NOTES

- LOCATIONS AND DEPTHS OF EXISTING UTILITIES AND DRAINAGE STRUCTURES SHOWN ON THE PLANS ARE APPROXIMATE ONLY. THE CONTRACTOR SHALL VERIFY THE EXACT LOCATION AND DEPTHS OF ALL UNDERGROUND UTILITIES AT LEAST 48 HOURS PRIOR TO CONSTRUCTION WHETHER SHOWN ON THE PLANS OR NOT. CONTRACTOR SHALL PROTECT ALL EXISTING UTILITIES.
- 2. ALL EXCAVATION IS UNCLASSIFIED. THERE IS NO ADDITIONAL PAYMENT FOR ROCK EXCAVATION. 3. ALL SPOIL AND UNUSABLE MATERIAL FROM THIS PROJECT SHALL BE
- REMOVED FROM THE SITE BY THE CONTRACTOR AT NO ADDITIONAL EXPENSE.
- 4. CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING ALL PERMITS, TESTS, APPROVALS AND ACCEPTANCES REQUIRED TO COMPLETE CONSTRUCTION OF THE PROJECT.
- 5. CONSTRUCTION STAKING TO BE PROVIDED BY CONSULTANT IS AS FOLLOWS:
- A. STREET CENTERLINE STAKING FOR CLEARING.
- B. STREET STAKING (ONE SIDE) FOR STREET EXCAVATION AND WATER MAIN INSTALLATION.
- SEWER STAKING AT 100-FT INTERVALS. STAKING FOR WATER SERVICES.
- STAKING FOR DRAINAGE CHANNELS.
- FINAL STREET STAKING. G. METER BOX STAKING.
- H. CPS STAKING. I. SETTING OF LOT CORNERS.

### CPS NOTES:

1. CPS TO SUPPLY ALL ELECTRIC CONDUITS FOR TRENCH AS FOLLOW: PRIMARY – 2 1/2" HDPE SCHEDULE 40 SECONDARY - 3" PVC SCHEDULE 40

SERVICE STUBS - 2 1/2" PVC SCHEDULE 40

- 6" P.V.C. SCHEDULE 80 WILL BE REQUIRED FOR C.P.S. UTILITIES CROSSINGS WHEN DRAIN OR STREET CONSTRUCTION PRECEDES UTILITY INSTALLATION.
- 3. 4" P.V.C. SCHEDULE 40 WILL BE REQUIRED FOR UNDERGROUND TELEPHONE AND CABLE T.V. IF ABOVE APPLIES.
- 4. P.V.C. CONDUIT WITH 90° SWEEPS TO 6" ABOVE GRADE WITH CAP.

### NOTE :

TELEPHONE AND CABLE LINES TO GO IN JOINT TRENCH WITH CITY PUBLIC SERVICE.

# LEGACY AT GREEN ENCLAVE, UNIT 1 UTILITY IMPROVEMENTS — E8"W -----

### LEGEND

EXISTING WATER MAIN	E8"W
PROPOSED WATER MAIN	8"W
PROPOSED FIRE HYDRANT	
EXISTING FIRE HYDRANT	
PROPOSED GATE VALVE	
EXISTING GATE VALVE	
PROPOSED SANITARY SEWER MAIN	
EXISTING SANITARY SEWER MAIN	E8"SS
EXISTING OVERHEAD ELECTRIC	OHE
EXISTING UNDERGROUND ELECTRIC	UE
EXISTING UNDERGROUND TELEPHONE	UGT
EXISTING STREET LIGHT	<b>\</b>
OFFICIAL PUBLIC RECORDS OF BEXAR COUNTY, TEXAS	D.P.R.B.C.T.
OFFICIAL PUBLIC RECORDS OF MEDINA COUNTY, TEXAS	O.P.R.M.C.T.
PROPOSED STREET LIGHT UG, 100W AND SINGLE ARM	
PROPOSED STREET LIGHT UG, 250W AND SINGLE ARM	*
EXISTING POWER POLE	PP
EXISTING SECONDARY ENCLOSURE	O
PROPOSED SECONDARY ENCLOSURE	Ø
PROPOSED POWER POLE	PP
PROPOSED TRANSFORMER	
PROPOSED WATER SERVICE	
PROPOSED SERVICE LATERAL WITH ONE-WAY CLEANOU	
EXISTING TRANSFORMER	
EXISTING IRRIGATION CONTROL VALVE	8
EVICTING WATER NETER	

FRONT LOT LINE

SIDE LOT LIN

FRONT LOT LINE

SIDE LOT LINI

SECONDARY ENCLOSURE

FRONT LOADED

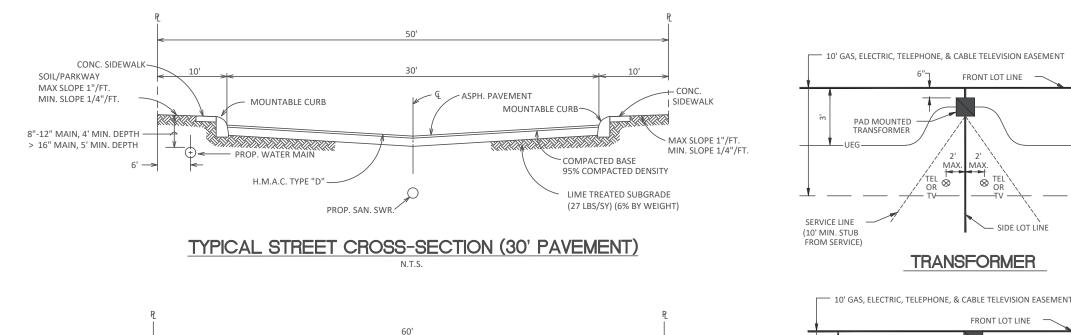
PAD MOUNTED SECONDARY ENCLOSURE

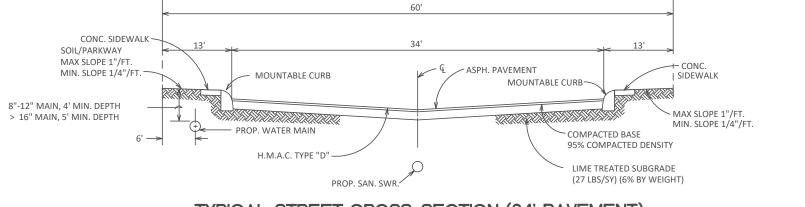
SERVICE LINE

(10' MIN. STUB FROM SERVICE)

SEC. ENCLOSUR



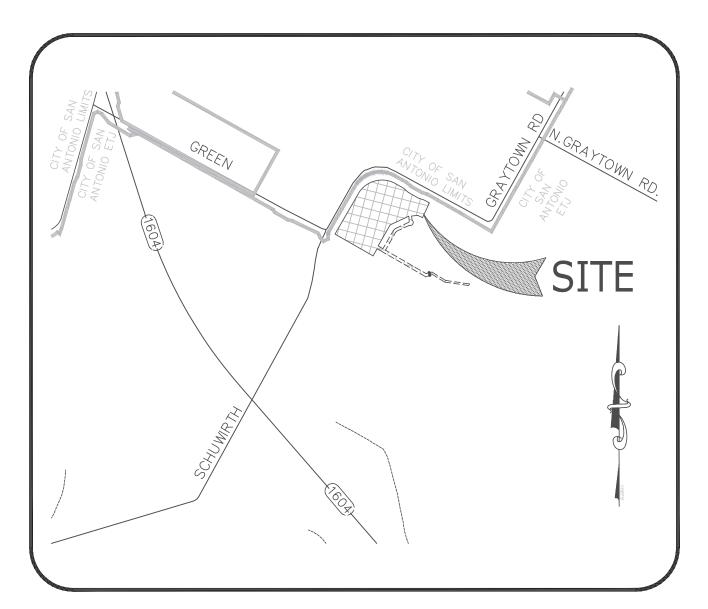




TYPICAL STREET CROSS-SECTION (34' PAVEMENT)

# BEXAR COUNTY

# **CONSTRUCTION PLANS FOR**



### SUBMITTAL DATE: SEPTEMBER 2023



## **LEGAL DESCRIPTION:**

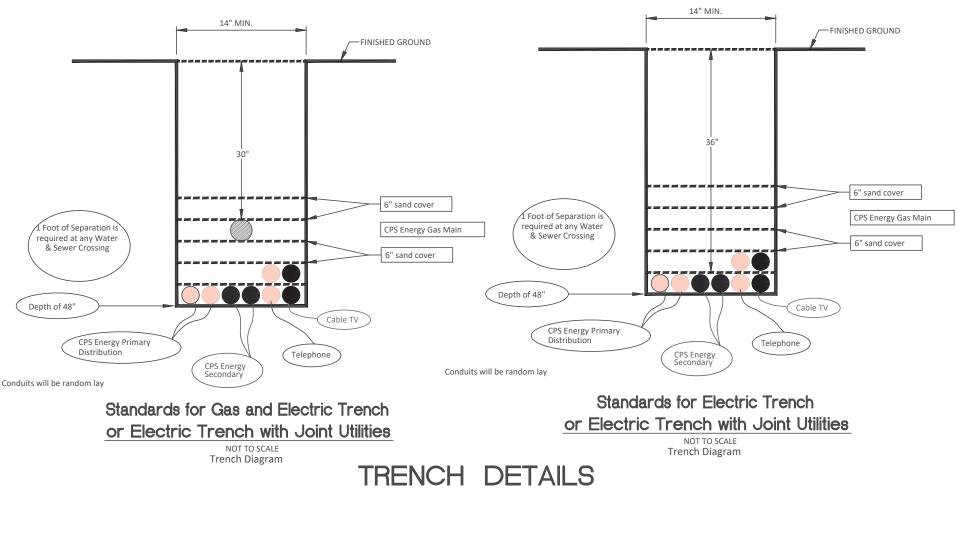
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NOTE TO CONTRACTOR

BY THE ACT OF SUBMITTING A BID FOR THIS PROPOSED CONTRACT, THE BIDDER WARRANTS THAT THE BIDDER, AND ALL SUBCONTRACTORS AND MATERIAL SUPPLIERS HE INTENDS TO USE HAVE CAREFULLY AND THOROUGHLY REVIEWED THE DRAWINGS, SPECIFICATIONS AND ALL OTHER CONTRACT DOCUMENTS AND HAVE FOUND THEM COMPLETE AND FREE FROM ANY AMBIGUITIES AND SUFFICIENT FOR THE PURPOSE INTENDED. THE BIDDER FURTHER WARRANTS THAT TO THE BEST OF HIS OR HIS SUBCONTRACTORS' AND MATERIAL SUPPLIERS' KNOWLEDGE, ALL MATERIALS AND PRODUCTS SPECIFIED OR INDICATED HEREIN ARE ACCEPTABLE FOR ALL APPLICABLE CODES AND AUTHORITIES.

THE LOCATION OF ALL EXISTING UTILITIES SHOWN ON THESE PLANS HAS BEEN BASED UPON RECORD INFORMATION ONLY AND MAY NOT MATCH LOCATIONS AND/OR DEPTHS AS CONSTRUCTED. THE CONTRACTOR SHALL CONTACT EACH INDIVIDUAL UTILITY, FOR ASSISTANCE IN DETERMINING EXISTING UTILITY LOCATIONS AND DEPTHS PRIOR TO BEGINNING ANY CONSTRUCTION. CONTRACTOR SHALL FIELD VERIFY LOCATIONS OF ALL UTILITY CROSSINGS PRIOR TO BEGINNING ANY CONSTRUCTION.





### PLAT NO. 23-11800388

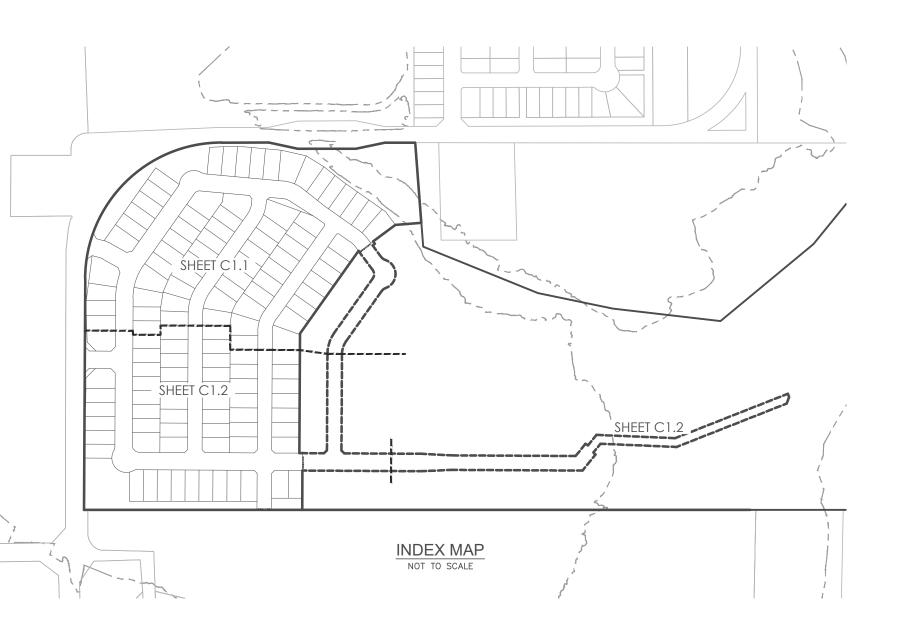
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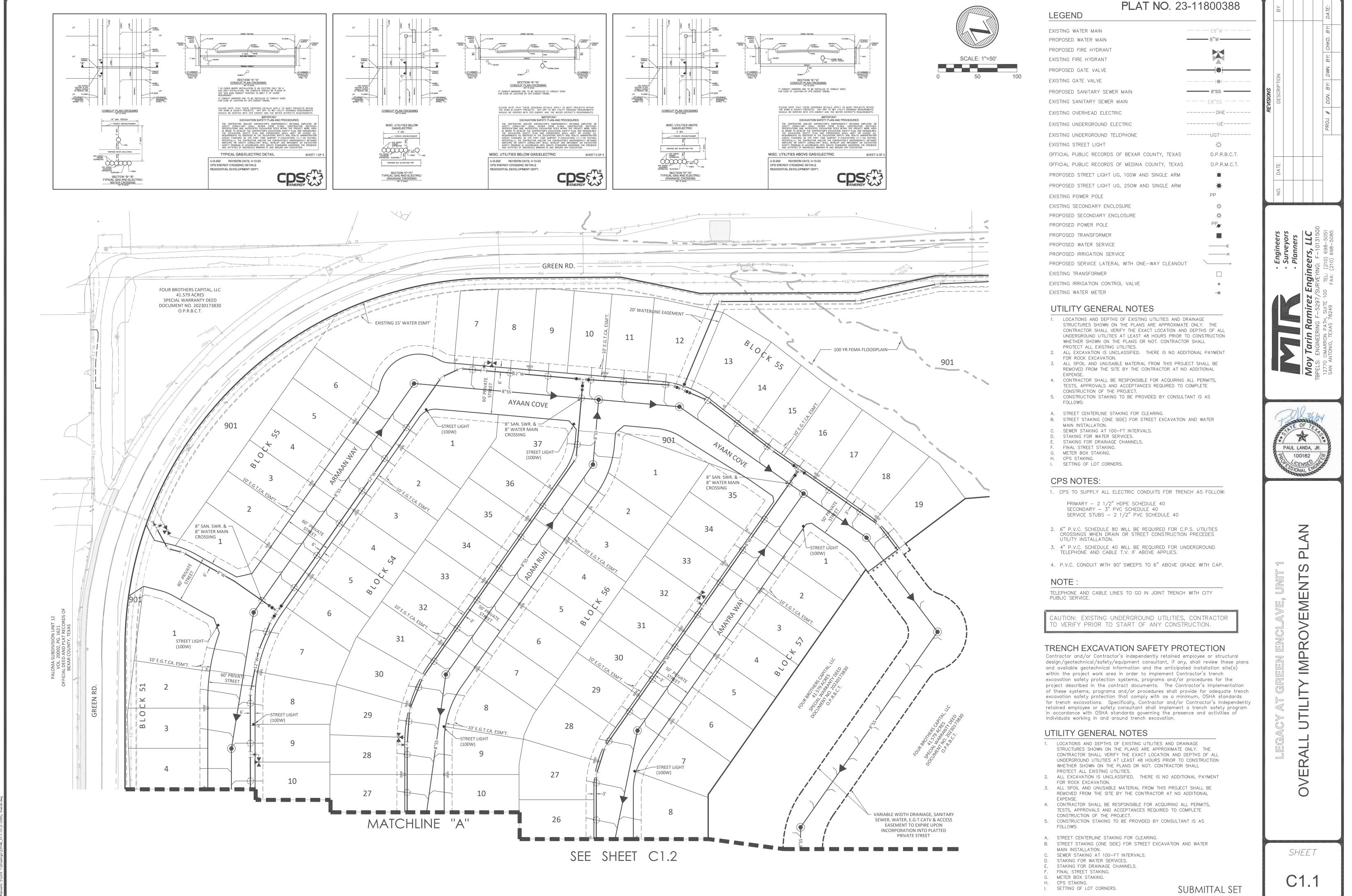
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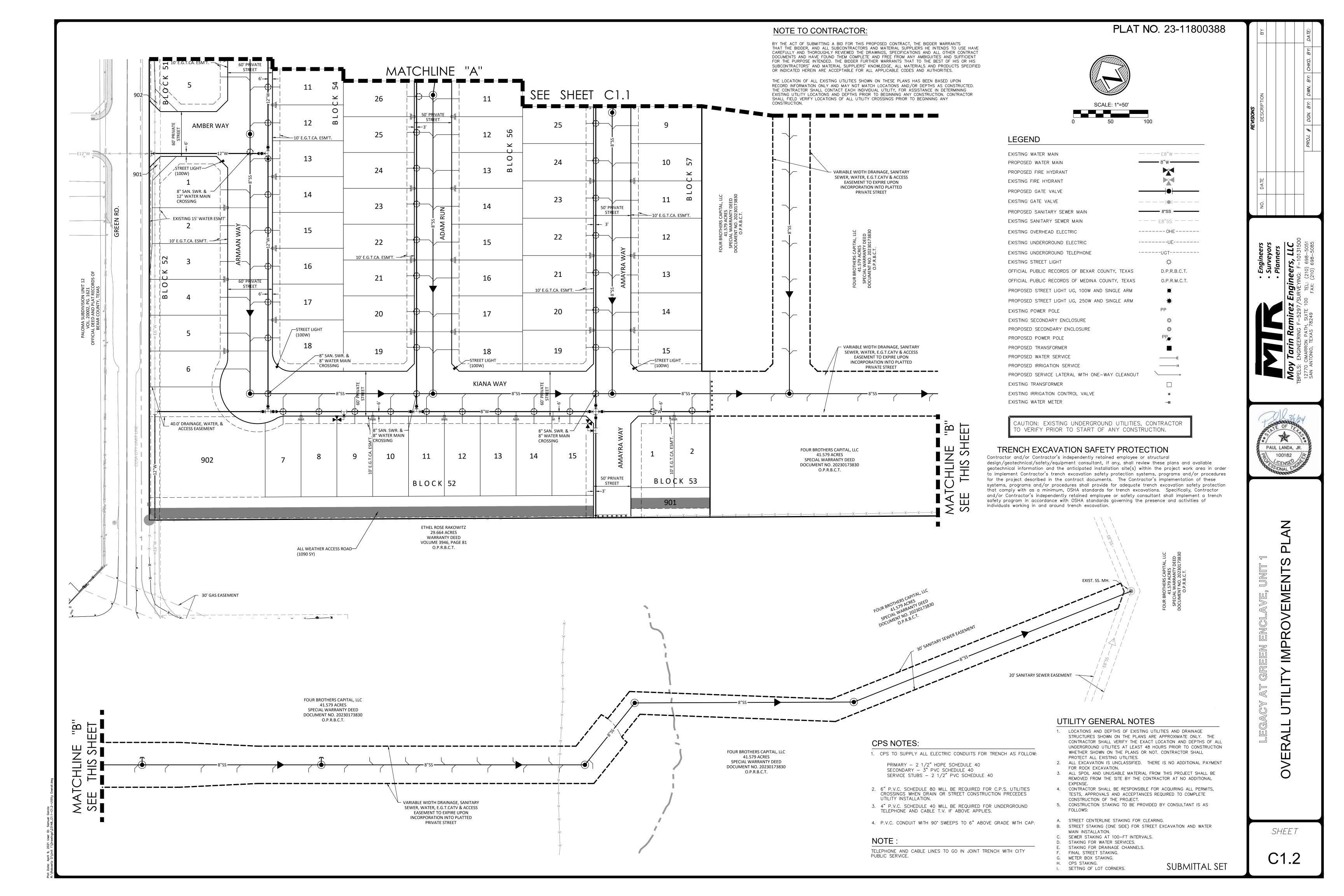
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C1.2	UTILITY OVERALL



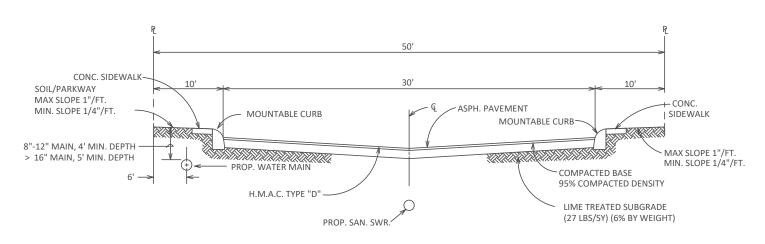
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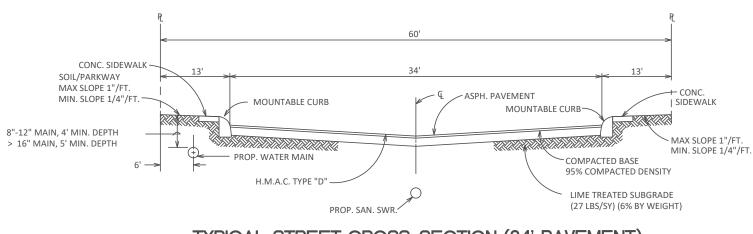




# LEGACY AT GREEN ENCLAVE, UNIT 1 SANITARY SEWER IMPROVEMENTS







TYPICAL STREET CROSS-SECTION (34' PAVEMENT

### UTION!

THE CONTRACTOR SHALL BE REQUIRED TO LOCATE ALL PUBLIC OR PRIVATE UTILITIES INCLUDING BUT NOT LIMITED TO: WATER, SEWER, TELEPHONE AND FIBER OPTIC LINES, SITE LIGHTING ELECTRIC, SECONDARY ELECTRIC, PRIMARY ELECTRICAL DUCT BANKS, LANDSCAPE IRRIGATION FACILITIES, AND GAS LINES. ANY UTILITY CONFLICTS THAT ARISE SHOULD BE COMMUNICATED TO THE ENGINEER IMMEDIATELY AND PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL CONTACT 1-800-DIG-TESS A MINIMUM OF 48 HOURS PRIOR TO THE START OF CONSTRUCTION. ANY DAMAGE TO EXISTING UTILITIES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND THE REPAIR SHALL BE AT CONTRACTOR'S SOLE EXPENSE WHETHER THE UTILITY IS SHOWN ON THESE PLANS OR NOT.

TRENCH EXCAVATION SAFETY PROTECTION Contractor and/or Contractor's independently retained employee or structural design/geotechnical/safety/equipment consultant, if any, shall review these plans and available geotechnical information and the anticipated installation site(s) within the project work area in order to implement Contractor's trench excavation safety protection systems, programs and/or procedures for the project described in the contract documents. The Contractor's implementation of these systems, programs and/or procedures shall provide for adequate trench excavation safety protection that comply with as a minimum, OSHA standards for trench excavations. Specifically, Contractor and/or Contractor's independently retained employee or safety consultant shall implement a trench safety program in accordance with OSHA standards governing the presence and activities of individuals working in and around trench excavation.

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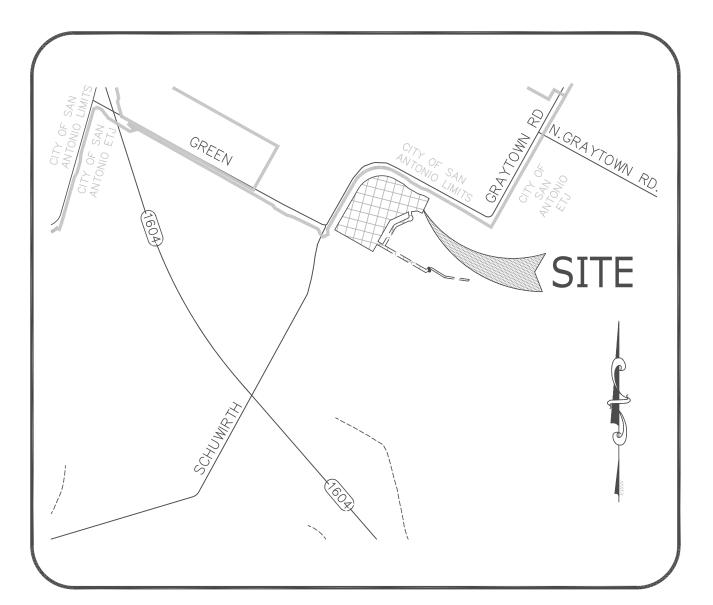
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### **GENERAL SEWER NOTES**

- 1. ALL SEWER CONSTRUCTION IS WITHIN THE JURISDICTION OF THE SAN ANTONIO RIVER AUTHORITY (SARA).
- 2. A PRECONSTRUCTION CONFERENCE WILL BE HELD WITH THE CONTRACTOR, CONSULTANT AND SARA STAFF PRIOR TO START OF CONSTRUCTION.
- 3. THE CONTRACTOR SHALL OBTAIN A COPY OF THE TECHNICAL SPECIFICATIONS FOR UTILITIES CONSTRUCTION, DATED APRIL, 2012, AS PUBLISHED BY THE SAN ANTONIO RIVER AUTHORITY. THESE SPECIFICATIONS AND THE GENERAL NOTES SHOWN ON THE PLANS WILL GOVERN ALL SEWER CONSTRUCTION IN THIS PROJECT.
- 4. DENSITY TESTING OF SECONDARY BACKFILL MATERIAL IN SEWER TRENCHES WILL BE REQUIRED. SEE SECTION 400.4 OF THE SPECIFICATIONS FOR MORE INFORMATION.
- 5. SEEPAGE RETAINERS WILL BE REQUIRED AT CERTAIN LOCATIONS. SEE SECTION 400.4(C) OF THE SPECIFICATIONS FOR MORE INFORMATION.
- 6. COORDINATE ALL WORK WITH THE SARA INSPECTOR.
- 7. 6" CLEANOUTS ARE NOT ALLOWED IN DRIVEWAY



# CONSTRUCTION PLANS FOR



VICINITY MAP

# SUBMITTAL DATE:

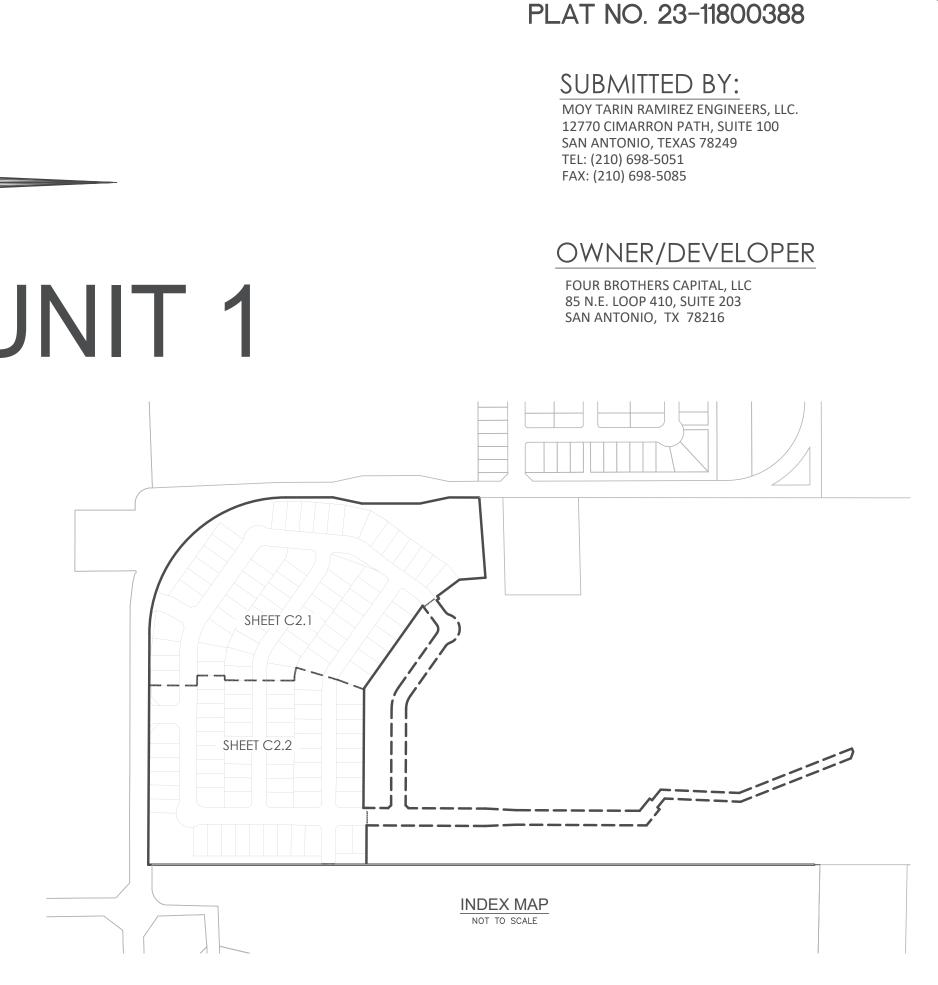
# **REVISION DATE:**

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C2.12	LINE "F" PLAN AND PROFILE
C2.13	SARA SANITARY SEWER DETAILS

SUBMITTAL SET TEXAS C2.0

GENERAL NOTES:

- . SAN ANTONIO RIVER AUTHORITY (RIVER AUTHORITY) STANDARD SPECIFICATIONS AND STANDARD DETAILS ARE PROVIDED FOR DESIGN AND CONSTRUCTION OF SEWER COLLECTION SYSTEMS MANAGED AND CONTRACTED BY THE RIVER AUTHORITY.
- 2. AT ANY TIME, THESE STANDARD SPECIFICATIONS AND DETAILS MAY BE ALTERED OR SUPERSEDED BY THE GENERAL CONDITIONS, SUPPLEMENTAL CONDITIONS, PLANS OR PROJECT SPECIFICATIONS WITHIN THE CONTRACT DOCUMENT PER DIRECTION FROM THE RIVER AUTHORITY.
- 3. ALL MATERIALS AND CONSTRUCTION PROCEDURES WITHIN THE SCOPE OF THIS CONTRACT SHALL BE APPROVED BY RIVER AUTHORITY AND COMPLY WITH THE CONTRACT DOCUMENTS AND THE FOLLOWING AS APPLICABLE:
- 3.1. CURRENT TEXAS COMMISSION ON ENVIRONMENTAL QUALITY (TCEQ) "DESIGN CRITERIA FOR DOMESTIC WASTEWATER SYSTEM", TEXAS ADMINISTRATIVE CODE (TAC) TITLE 30, PART 1, CHAPTER 217.
- 3.2. CURRENT TEXAS DEPARTMENT OF TRANSPORTATION (TXDOT). "STANDARD SPECIFICATIONS FOR CONSTRUCTION OF HIGHWAYS, STREETS AND DRAINAGE".
- 3.3. CURRENT RIVER AUTHORITY "STANDARD SPECIFICATIONS FOR 10. WHERE WATER LINES AND NEW SEWER LINES ARE INSTALLED SANITARY SEWER CONSTRUCTION".
- 3.4. CURRENT CITY OF SAN ANTONIO "STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION". CURRENT CITY OF SAN ANTONIO "UTILITY EXCAVATION CRITERIA MANUAL".
- 4. THE CONTRACTOR IS TO NOTIFY AND MAKE ARRANGEMENTS WITH THE RIVER AUTHORITY INSPECTIONS DIVISION AT (210) 302-4200 FORTY EIGHT (48) HOURS PRIOR TO ANY EXCAVATION. CONTRACTOR SHALL ALSO PROVIDE PROCEDURES THAT WILL BE USED TO NOTIFY AFFECTED RESIDENTS AND/OR PROPERTY OWNERS 48 HOURS PRIOR TO ANY EXCAVATION OR CONSTRUCTION. A PRE-CONSTRUCTION CONFERENCE SHALL BE HELD BEFORE ANY EXCAVATION OR START OF PROJECT.
- 5. WORK SHALL NOT BE PERFORMED ON SATURDAYS, SUNDAYS, FEDERAL HOLIDAYS, RIVER AUTHORITY HOLIDAYS, BEFORE 7:30 AM. OR AFTER 4:30 PM, UNLESS PRIOR APPROVAL IS GRANTED BY THE RIVER AUTHORITY ENGINEER. REQUEST TO PERFORM WORK DURING THESE TINES MUST BE EMAILED 48 HOURS IN ADVANCE TO UTILITIESDEVELOPMENT@SARIVERAUTHORITY.ORG.
- 6. NO EXTRA PAYMENT SHALL BE ALLOWED FOR WORK CALLED FOR IN THE PLANS BUT NOT INCLUDED IN THE BID SCHEDULE. THIS INCIDENTAL WORK WILL BE REQUIRED AND SHALL BE INCLUDED UNDER THE PAY ITEM WHICH IT RELATES TO.
- 7. WORK COMPLETED BY CONTRACTOR WHICH HAS NOT RECEIVED A WORK ORDER OR THE CONSENT OF RIVER AUTHORITY WILL BE SUBJECT TO REMOVAL AND REPLACEMENT AT THE EXPENSE OF THE CONTRACTOR.
- 8. LOCATIONS AND DEPTHS OF EXISTING UTILITIES AND SERVICE LATERALS SHOWN ON THE PLANS ARE UNDERSTOOD TO BE APPROXIMATE. ACTUAL LOCATIONS AND DEPTHS MUST BE FIELD VERIFIED BY THE CONTRACTOR AT LEAST 48 HOURS PRIOR TO CONSTRUCTION REGARDLESS OF ILLUSTRATION ON THE PLANS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO LOCATE UTILITY SERVICE LINES AS REQUIRED FOR CONSTRUCTION AND

TO PROTECT THEM DURING CONSTRUCTION AT NO COST TO RIVER AUTHORITY CONTRACTOR WILL BE RESPONSIBLE FOR ANY DAMAGES TO EXISTING UTILITIES AND REPAIRS WILL BE AT CONTRACTOR'S EXPENSE. THE FOLLOWING CONTACT INFORMATION IS SUPPLIED FOR

VERIFICATION PURPOSES:	
EAST CENTRAL SPECIAL UTILITY DISTRICT CITY OF SAN ANTONIO DRAINAGE CITY PUBLIC SERVICE (CPS) CITY OF CONVERSE (PUBLIC WORKS) TIME WARNER VALERO ENERGY CO. RIVER AUTHORITY INSPECTIONS TEXAS 811 SAN ANTONIO WATER SYSTEM (SAWS)	210-649-2383 210-207-5048 210-973-3500 210-659-9513 210-352-4872 210-349-7555 210-302-4200 800-344-8377 210-233-3500
	210 200 0000

- 9. CERTAIN PORTIONS OF THE PROJECT MAY PARALLEL AND/OR CROSS EXISTING UTILITIES, AND CONTRACTOR IS REQUIRED T PROTECT THESE UTILITIES. ADDITIONAL SUPPORTIVE SHORING MAY BE REQUIRED. IT IS THE CONTRACTCR'S RESPONSIBILITY TO PROTECT HIS WORKERS, EXISTING UTILITIES, AND FINISHED WORK THROUGHOUT THE PROJECT. CONTRACTOR WILL BE RESPONSIBLE FOR ANY DAMAGES AND REPAIRS WILL BE AT CONTRACTORS EXPENSE.
- WITH A SEPARATION DISTANCE LESS THAN 9 FEET (I.E. WATER LINES CROSSING WASTEWATER LINES, WATER LINES PARALLELING WASTEWATER LINES OR WATER LINES NEXT TO MANHOLES). THE INSTALLATION MUST MEET THE REQUIREMENTS OF 30 TAC 217 AND 30 TAC 290.
- 11. DUE TO FEDERAL REGULATIONS TITLE 49, PART 192.161, CPS MUST MAINTAIN ACCESS TO GAS VALVES AT ALL TIMES. THE CONTRACTOR MUST PROTECT AND WORK AROUND VALVES THAT ARE IN THE PROJECT AREAS.
- 12. AN APPROPRIATELY SAFE OVERHEAD CLEARANCE MUST BE MAINTAINED BETWEEN ALL OVERHEAD EQUIPMENT AND PERSONNEL. THE CONTRACTOR SHALL NOTIFY CPS AT LEAST 48 HOURS PRIOR TO ANY CONSTRUCTION IN THE VICINITY OF CPS OVERHEAD LINES. CONTRACTOR SHALL MAINTAIN CPS RECOMMENDED CLEARANCE REQUIREMENTS.
- 13. ALL WORK IN THE TEXAS DEPARTMENT OF TRANSPORTATION (TXDOT) RIGHT-OF-WAY SHALL PROCEED DURING WORKING HOURS AGREED UPON BY RIVER AUTHORITY AND TXDOT INSPECTORS.
- 14. BEFORE THE START OF ANY CONSTRUCTION, THE PROJECT SITE MUST BE VIDEO RECORDED BY THE CONTRACTOR WITH ONE COPY SUBMITTED TO RIVER AUTHORITY. THE PRE-CON SITE VIDEO WILL PROVIDE ACCURATE DOCUMENTATION OF EXISTING CONDITIONS.
- 15. THE CONTRACTOR SHALL BE RESPONSIBLE FOR RESTORING EXISTING FENCES, CURBS, STREETS, DRIVEWAYS, SIDEWALKS, LANDSCAPING AND STRUCTURES TO ORIGINAL OR BETTER CONDITION AS A RESULT OF DAMAGE DONE DURING THE PROJECT CONSTRUCTION.
- 16. ANY AND ALL FENCING, INCLUDING ELECTRIC FENCE, WHETHER OR NOT IDENTIFIED ON THE PLANS. MUST BE MAINTAINED AT ALL TIMES. ANY AND ALL DAMAGES DIRECTLY ATTRIBUTED TO THE CONTRACTOR MUST BE REPLACED TO EQUAL OR

- 17. CONTRACTOR MUST AVOID DAMAGE TO ADJACENT LAND

AUTHORITY

- PLUGGED OVERNIGHT.

- FLOODPLAIN.
- PERSONNEL.

### <u>SEWER NOTES:</u>

- 25. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT NO SANITARY SEWER OVERFLOW (SSO) OCCURS AS A RESULT OF THE WORK ALL PERSONNEL RESPONSIBLE FOR SSO PREVENTION AND CONTROL SHALL BE TRAINED ON THE PROPER RESPONSE. SHOULD AN SSO OCCUR, THE CONTRACTOR SHALL:
- 25.1. IDENTIFY THE SOURCE OF THE SSO AND ATTEMPT TO ELIMINATE ANY ADDITIONAL SPILLAGE.
- 25.2. NOTIFY RIVER AUTHORITY CONSTRUCTION INSPECTIONS DIVISION AT 210-302-4216 OR 210-219-0130
- 25.3. ATTEMPT TO ELIMINATE THE SOURCE OF THE SSO. 25.4. CONTAIN SEWAGE FROM THE SSO TO PREVENT
- CONTAMINATION OF WATERWAYS. 25.5. CLEAN UP THE SPILL SITE AND REMOVE CONTAMINATED
- MATERIALS. 25.6. DISINFECT THE AREA OF THE SPILL WITH A MIXTURE OF HTH
- CHLORINE AND WATER. 25.7. CLEAN THE AFFECTED SEWER LINE AND REMOVE ANY DEBRIS.
- 25.8. IDENTIFY AND TRAIN PERSONNEL RESPONSIBLE FOR SPILLAGE PREVENTION AND CONTROL 25.9. NO SEPARATE MEASUREMENT OR PAYMENT SHALL BE MADE FOR THIS WORK, ALL WORK SHALL BE DONE ACCORDING TO
- GUIDELINES SET BY THE TCEQ AND RIVER AUTHORITY. 26. TIE-INS OR SHUTDOWNS OF EXISTING FORCE MAINS OF ANY SIZE MUST BE COORDINATED WITH THE RIVER AUTHORITY INSPECTOR AT LEAST ONE WEEK IN ADVANCE OF THE SHUTDOWN. CONTRACTOR SHALL PROVIDE A SEQUENCE OF WORK AS RELATED TO TIE-INS AT NO ADDITIONAL COST TO RIVER AUTHORITY OR THE PROJECT.
- 27. ELEVATIONS OF THE TOP OF MANHOLES AND INVERTS ARE FOR REFERENCE ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO MAKE ALLOWANCES AND ADJUSTMENTS FOR THE TOPS OF MANHOLES AND INVERTS TO MATCH THE FINISHED GRADE OF THE PROJECT IMPROVEMENTS (NSPI).
- 28. THE CONTRACTOR SHALL PROVIDE BYPASS PUMPING OF SEWAGE AROUND EACH SEGMENT OF PIPE TO BE REPLACED. CONTRACTOR SHALL HAVE STANDBY PUMPS AVAILABLE TO BYPASS FLOW IN CASE PRIMARY PUMP FAILS. THE CONTRACTOR SHALL PROVIDE A SEQUENCE OF BYPASS PUMPING FOR REVIEW AND APPROVAL BY RIVER AUTHORITY. THE CONTRACTOR SHALL ALSO PROVIDE A DETAILED SKETCH SHOWING THE LOCATION OF BYPASS PUMPING; SPECIFICATIONS FOR THE PUMPING EQUIPMENT: AND TYPE, SIZE CAPACITY AND NUMBER OF PUMPS REQUIRED TO HANDLE THE PEAK WET WEATHER FLOW.
- 29. CONTRACTOR WILL MAINTAIN SERVICE TO ALL EXISTING SANITARY SEWERS AT ALL TIMES DURING CONSTRUCTION. CONTRACTOR WILL CLEAN ALL DEBRIS, GRAVEL, DIRT, ETC, OUT OF MANHOLES AND FIX ANY STOPPAGES CAUSED BY DEBRIS DURING CONSTRUCTION AT CONTRACTOR'S EXPENSE. ANY DAMAGE TO EXISTING MANHOLES OR SEWER MAIN WILL BE CORRECTED AT CONTRACTOR'S EXPENSE. CONTRACTOR SHALL TAKE ALL NECESSARY MEASURES TO PREVENT DAMAGE TO EXISTING OR NEW RINGS, COVERS, OR CONES FROM EQUIPMENT AND MATERIALS USED OR TAKEN THROUGH THE WORK AREA. IF AN EXISTING OR NEW MANHOLE COVER, RING, OR CONE IS DAMAGED BY THE CONTRACTOR, IT SHALL BE REPLACED AS DIRECTED BY THE RIVER AUTHORITY INSPECTOR. MANHOLES WILL NEED TO BE RESEALED WITH RIVER AUTHORITY APPROVED SEALANT. IF SEAL COATING IS COMPROMISED, CONTRACTOR WILL HAVE MANHOLE RECOATED. CONTRACTOR SHALL RESEAL ALL LEAKS AT CONTRACTOR EXPENSE.
- 30. CONTRACTOR TO ENSURE ALL PLUGS USED TO PLUG SEWER LINES WHILE TESTING THE PROJECT (SUCH AS AIR PLUGS, SCREW TYPE PLUGS, ETC.) ARE LABELED, MARKED OR TAGGED. THE CONTRACTOR SHALL RECORD HOW MANY PLUGS ARE BEING USED, AS WELL AS THE LOCATION AND IDENTIFICATION OF EACH PLUG. CONTRACTOR WILL REPORT TO PROJECT INSPECTOR OF ANY LOST OR UNRESTRAINED PLUGS.

- COLLECTION SYSTEM STOPPAGES OVER-FLOWS OR BACKUPS INTO HOMES CAUSED BY LOST OR RUNAWAY SEWER PLUGS.
- 32. CONTRACTOR WILL BE HELD RESPONSIBLE FOR ANY DAMAGE TO WASTEWATER TREATMENT EQUIPMENT CAUSED BY LOST OR RUNAWAY SEWER PLUGS. CONTRACTOR WILL BE HELD LIABLE FOR REPAIRS.
- 33. RIVER AUTHORITY IS NOT RESPONSIBLE FOR ANY ABNORMALITIES ON STUB OUT, INVERT, GRADE OR SLOPE FOR ANY EXISTING MANHOLE TIE-IN OR SERVICE LATERAL

MANHOLE NOTES:

- 34. THERE SHALL BE 400 FEET MAXIMUM SPACE BETWEEN MANHOLES TO PROVIDE ACCESS FOR CLEANING. A 16-FOOT WIDE GATE WITH A LOCK SHALL ALSO BE SUPPLIED AND INSTALLED BY THE CONTRACTOR FOR ACCESSING THE SANITARY SEWER LINE IN EASEMENTS.
- 35. ALL MANHOLES SHALL BE CONSTRUCTED SO THAT THE TOP OF THE RING IS AT LEAST FOUR (4) NCHES ABOVE THE FINISHED GRADE OF THE SURROUNDING GROUND IN UNPAVED AREAS. IN PAVED AREAS, THE MANHOLE RING SHALL BE FLUSH WITH THE PAVEMENT.
- 36. EVERY THIRD MANHOLE COVER WILL HAVE A 1"HOLE FOR A VENT. VENTING SHALL COMPLY WITH TAC 217.55.
- 37. EACH MANHOLE SHALL HAVE TWO LOCKS INSTALLED TO PREVENT REMOVAL.
- 38. ALL MANHOLES SHALL HAVE A 30"OPENING, WATERTIGHT RINGS AND COVERS, WITH THE RIVER AUTHORITY LOGO AND I/I BARRIFR.
- 39. NEW MANHOLE PROTECTIVE COATING LINER MUST BE APPLIED TO ALL MANHOLES. APPLICATION PROCEDURES SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATION AND PER THE FOLLOWING SPECIFICATIONS:
- 39.1. CONTRACTOR WILL BE RESPONSIBLE FOR MANHOLE SAFETY AND CONFINED SPACE ENTRY SET BY OCCUPATIONAL SAFETY AND HEALTH STANDARDS, 29 CFR 1910.146 APP. E. 39.2. THE CONTRACTOR SHALL NOTIFY THE RIVER AUTHORITY
- UTILITIES INSPECTIONS DEPARTMENT A MINIMUM OF 48 HOURS IN ADVANCE OF THE START OF ANY FIELD SURFACE PREPARATION WORK FOR MANHOLES. 39.3. ALL NEW MANHOLES AND THE EXISTING MANHOLE THAT THE
- PROPOSED SEWER LINE WILL TE-IN TO SHALL HAVE THE INTERIOR WALL PREPPED AS PER MANUFACTURER'S RECOMMENDATIONS AND COATED WITH A RIVER AUTHORITY APPROVED PRODUCT
- 39.4. FOR ALL MANHOLES, APPLY THE CEMENTITIOUS COATING FIRST, FOLLOWED BY THE EPOXY COATING. LAFARGE SEWPERCOAT 200 HR PRODUCT IS THE ONLY APPROVED PRODUCT WHICH COMBINES THE CEMENTITIOUS AND EPOXY COATINGS, UNLESS OTHERWISE LISTED IN THE APPROVED PRODUCT II
- 39.5. CEMENTITIOUS COATING WITH REQUIRED ONE-INCH-THICK APPLICATION: SEE RIVER AUTHORITY APPROVED PRODUCT
- 39.6. EPOXY COATING: WITH SPECIFIED THICKNESS APPLICATION: SEE RIVER AUTHORITY APPROVED PRODUCT SHEET.
- 39.7. SPRAY WALL POLYURETHANE SYSTEM REQUIRED THICKNESS 150 MILS
- 39.8. CONTRACTOR SHALL SUBMIT WARRANTY LETTER ON MANHOLE PROTECTIVE COATING FOR 10 YEARS AFTER FINAL ACCEPTANCE OF PROTECTIVE COATINGS.
- 40. ANY CONNECTIONS TO EXISTING MANHOLES WILL REQUIRE A CRADLE TO SUPPORT THE INCOMING PIPE. A RUBBER GASKET WILL ALSO BE REQUIRED (CENTERED AT MANHOLE WALL) WITH GROUTING AT INTERIOR AND EXTERIOR PENETRATIONS.

### BETTER CONDITIONS AT THE CONTRACTOR'S EXPENSE AND AS APPROVED BY THE RIVER AUTHORITY INSPECTOR. GAPS IN THE FENCING MUST 3E PROVIDED AT ALL LOCATIONS WHERE THE SEWER LINE EASEMENT CROSSES FENCING. FENCING REQUIRED TO MAINTAIN LIVESTOCK MUST BE MAINTAINED AT ALL TIMES.

OUTSIDE THE IDENTIFIED CONSTRUCTION LIMITS. ANY CLAIMS DIRECTLY ATTRIBUTED TO THE CONTRACTOR RESULTING FROM HIS STRAYING BEYOND THE CONSTRUCTION LIMITS MUST BE SETTLED BY THE CONTRACTOR TO THE SATISFACTION OF RIVER AUTHORITY AND THE APPROPRIATE LANDOWNER.

18. CONTRACTOR MUST MAINTAIN ACCESS FOR PRIVATE INDIVIDUALS AND BUSINESSES AT ALL TIMES. IF NORMAL ACCESS IS DAMAGED DURING CONSTRUCTION, THE CONTRACTOR MUST REPLACE THE ACCESS TO EQUAL OR BETTER CONDITION AT THE CONTRACTOR'S EXPENSE AND AS APPROVED BY RIVER

19. CONTRACTOR MUST COMPLY WITH TEXAS GOVERNMENT CODE SECTION 2166.303 UNIFORM TRENCH SAFETY CONDITIONS. 20. CONTRACTOR SHALL BACKFILL ALL OPEN TRENCHES AT THE END OF THE DAY. CONTRACTOR SHALL NOT INSTALL MORE PIPE THAN CAN BE COVERED. NO OPEN TRENCHES WILL BE PERMITTED OVERNIGHT. ALL ENDS OF OPEN PIPE SHALL BE

21. NO TREES SHALL BE REMOVED AS PART OF THIS PROJECT UNLESS OTHERWISE SPECIFIED IN THE PLANS.

22. FOR PORTIONS OF THE CONSTRUCTION THAT ARE WITHIN THE LIMITS OF THE 100-YEAR FLOODPLAIN, THE CONTRACTOR IS REQUIRED TO KEEP THE CHANNEL CLEAR OF POTENTIAL OBSTRUCTIONS TO FLOOD FLOWS. POTENTIAL OBSTRUCTIONS INCLUDE HEAVY CONSTRUCTION EQUIPMENT, TEMPORARY ROADS ACROSS CHANNEL, EXCAVATED MATERIAL, STOCKPILED DEBRIS. AND ALL OTHER ITEMS DEEMED UNACCEPTABLE BY RIVER AUTHORITY. UNDER THREATENING WEATHER CONDITIONS AND WHERE FLOODING IS LIKELY, OBSTRUCTIONS SHALL BE IMMEDIATELY REMOVED BY THE CONTRACTOR AT NO ADDITIONAL COST TO RIVER AUTHORITY. THE CONTRACTOR ASSUMES ALL RISK FOR UNFINISHED WORK. NO EQUIPMENT OR MATERIALS SHALL BE STOCKPILED IN THE 100-YEAR FLOODPLAIN.

23. NO WASTE MATERIAL SHALL BE PLACED IN EXISTING DRAINAGE LOWS THAT WILL BLOCK OR ALTER FLOW LIMITS, NATURAL DRAINAGE, OR PLACED WITHIN THE LIMITS OF EXISTING

24. IF A THREATENED OR ENDANGERED PLANT OR ANIMAL SPECIES AND/OR CULTURAL/ARCHAEOLOGICAL RESOURCES ARE ENCOUNTERED DURING CONSTRUCTION, THE CONTRACTOR SHALL STOP WORK IMMEDIATELY AND NOTIFY THE APPROPRIATE

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### 53. UPON REQUEST FROM THE SAN ANTONIO RIVER AUTHORITY, CONTRACTOR SHALL PROVIDE SAMPLE VERIFYING PROPER INSTALLATION OF FLOWABLE BACKFILL, INCLUDING, BUT NOT LIMITED TO CORE SAMPLES.

- SANITARY SEWER PIPING:
- 54. THE TYPE AND DESCRIPTION OF THE PIPE CONDUIT IS SHOWN ON THE PLANS. REFER TO RIVER AUTHORITY SPECIFICATIONS FOR MATERIALS, STIFFNESS, AND TYPE.
- 55. THE USE OF ASBESTOS CEMENT PIPE WILL BE PROHIBITED UNDER THIS CONTRACT.
- 56. ALL DUCTILE IRON PIPE USED IN THIS SYSTEM SHALL BE CORROSION PROTECTED ON BOTH THE INTERIOR AND EXTERIOR SURFACES. ALL CORROSION PROTECTION SHALL BE APPLIED AND INSTALLED IN SUCH A MANNER AS TO MAINTAIN A CONTINUOUSLY PROTECTED SURFACE AFTER FINAL PIPE INSTALLATION.
- 57. SEE SPECIFICATIONS FOR PVC SEWER PIPE WITH OVER 14 FEET OF COVER.
- 58. ALL SEWER PIPES SHALL HAVE COMPRESSION OR MECHANICAL JOINTS.
- 59. SAND MIGRATION PREVENTION COLLAR WHEN CHANGING THE INITIAL BACKFILL FROM SELECT INITIAL BACKFILL TO OPTIONAL SELECT INITIAL BACKFILL, A TWO (2) FOOT LONG CLASS B CONCRETE ENCASEMENT OR FIRMLY COMPACTED, CONSOLIDATED CLAY ENCASEMENT BETWEEN THE TWO SHALL BE PROVIDED FOR THE ENTIRE HEIGHT OF THE INITIAL BACKFILL, EVERY 180 FEET ALONG PIPE AND 20 FEET FROM WALL OF MANHOLE IN EACH DIRECTION.

SEWER LINE LOCATION:

- 60. SEWER LINES SHALL BE SIZED AND EXTENDED THROUGH THE LIMITS OF A DEVELOPMENT TO SERVE ADJACENT PROPERTY WITH MANHOLE AND STUB-OUT AT END OF SEWER LINE.
- 61. IN PHASED CONSTRUCTION OF THOROUGHFARES, THE SEWER LINE SHALL BE EXTENDED THE ENTIRE LENGTH OF PROPOSED THOROUGHEARE
- 62. ALL SANITARY SEWER LINES SHALL BE LOCATED A MINIMUM OF FIVE (5) FEET FROM ANY TREE.
- 63. SIZES AND GRADES FOR SANITARY SEWER SHALL BE AS REQUIRED BY THE RIVER AUTHORITY ENGINEER AND CONSIDERATION SHALL BE GIVEN AS TO POSSIBLE EXTENSIONS FOR FUTURE DEVELOPMENT, NO SANITARY SEWERS, OTHER THAN LATERALS AND FORCE MAINS, SHALL BE LESS THAN EIGHT (8) INCH IN DIAMETER.

SEWER SERVICE LATERALS

- 64. WHEN SEWER LATERALS ARE TO BE CONNECTED TO EXISTING SEWER MAINS AND NO STUB-OUT HAS BEEN EARLIER PROVIDED THE CONNECTION MUST BE CONDUCTED PER THE RIVER AUTHORITY STANDARD DETAILS AND APPROVED PRODUCT LIST.
- 65. REFER TO THE RIVER AUTHORITY APPROVED PRODUCTS LIST FOR ACCEPTABLE FITTINGS AND CONNECTIONS.
- 66. ALL RESIDENTIAL SERVICE LATERALS SHALL BE SDR 26 PVC WITH RATING OF 115 PSI OR 160 PSI, DETERMINED BY RIVER AUTHORITY SPECIFICATION. LINE SHALL BE EXTENDED TO THE PROPERTY LINE AT  $(6 \times 6)$  CAPPED AND SEALED. ATTACH SEWER BURIAL TAPE TO THE END OF ALL SEWER LATERALS AND BRING UP TO THE GROUND LEVEL FOR MARKER (GREEN). (SEE HOUSE LATERALS DETAILS).

# RIVER AUTHORITY ENGINEER:

- SERVICE LATERALS.
- TRAPS AND INTERCEPTORS (FOG TECQ) HAVE TRAPS (FOG -TECQ).

OIL SEPARATORS

SEWER SYSTEM.

SAND INTERCEPTORS

GALLONS CAPACITY.

AUTOMATIC CAR WASHES

20 GPM INTERCEPTOR MINIMUM.

NEUTRALIZING DEVICES

DISCHARGED INTO THE SANITARY SEWER SYSTEM.

DAMAGE TO EXISTING MANHOLE WILL BE REPAIRED AT CONTRACTOR'S EXPENSE. IF EXISTING SEWER MANHOLE SEAL COATING IS COMPROMISED, ALL OF THE MANHOLE WILL BE RESEALED

42. IF ANY EXISTING MANHOLES CONNECTED WITH THIS PROJECT ARE FOUND TO HAVE INFILTRATION, THE MANHOLES SHALL BE SEALED AND TESTED AT CONTRACTORS EXPENSE.

43. MANHOLES WITH STUB-OUTS 8-INCH OR LARGER MUST BE LOCATED AT THE END OF ALL SEWER LINES THAT MAY BE EXTENDED IN THE FUTURE. STUB-OUTS SHALL BE PLUGGED.

44. MANHOLE COVER INSERTS ARE SHOWN IN RIVER AUTHORITY APPROVED PRODUCT SHEET. INSERTS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAKING THE NECESSARY FIELD MEASUREMENTS FOR THE MANUFACTURER PRIOR TO PRODUCTION.

45. BEFORE BACK FILLING/COMPACTION/CONCRETE ENCASEMENT, ALL MANHOLE JOINT SECTION RISERS, CONE SECTIONS AND GRADE RING SHALL BE WRAPPED WITH INFI-SHIELD GATOR WRAP SEALING SYSTEMS. BUTYL ADHESIVE SEALANT WITH A MINIMUM THICKNESS OF 30 MILS. JOINT SEALANT MUST MEET ASTM C923. MASTIC MUST MEETS ASTM C 990 OR BE APPROVED BY THE RIVER AUTHORITY ENGINEER.

46. IF CONCRETE THROAT RINGS ARE TO BE INSTALLED THEY MUST BE USED IN CONJUNCTION WITH A UV STABILIZED POLYETHYLENE LINER AND I/I BARRIER. I/I BARRIER MUST MEET THE FOLLOWING ASTM STANDARDS: ASTM D-790/1505 DENSITY OF POLYETHYLENE MATERIALS, ASTM D-1238 MELT FLOW INDEX, ASTM 638 TENSILE STRENGTH@ YIELD (50 mm/mm). ASTM 790 FLEXURAL MODULUS, ASTM 648 HEAT DEFLECTION TEMPERATURE @IGEPAL, ASTM 1693 EsCR,100% IGEPAL /1C% IGEPAL.

47. A MINIMUM OF TWO AND A MAXIMUM OF FOUR THROAT RINGS WILL BE USED AT EACH MANHOLE FOR ADJUSTMENT.

48. DROP MANHOLES SHALL BE REQUIRED WHEN THE INFLOW ELEVATION IS MORE THAN 24 INCHES ABOVE THE OUTFLOW ELEVATION. DROP SHALL BE LOCATED OUTSIDE THE MANHOLE WITH THE FLOWLINE ELEVATION LOCATED BETWEEN THE CENTER LINE AND TOP OF SEWER LINE.

49. THERE SHALL BE CONCRETE ENCASEMENT 18 INCHES AROUND MANHOLE RING, AND 28 INCHES AROUND THE GATOR WRAP SEALING SYSTEM. CONCRETE ENCASEMENT SHALL BE CIRCULAR, FORMED LEVEL, AND HAVE A SMOOTH OR BROOM FINISH. SEE SPECIFICATIONS.

50. SEWER PIPE CONNECTIONS TO PRECAST MANHOLES SHALL BE APPROVED BY RIVER AUTHORITY. REFER TO APPROVED PRODUCT LIST. SEWER PIPE CONNECTIONS TO MONOLITHIC MANHOLES WILL BE AS SHOWN ON THE STANDARD DETAIL SHEET. ANY CHANGES IN THESE METHODS MUST BE APPROVED BY RIVER AUTHORITY FNGINFFR.

51. ALL PIPE TRENCHING, BEDDING AND BACKFILL SHALL BE DONE IN ACCORDANCE WITH APPROPRIATE ASTM/ANSI SPECIFICATIONS [REFERENCE 30 TAC 217.54; ASTM C-12 (ANSI A106.2) OR ASTM D-2321 (ANSI K65.171)]. ALL COMPACTION SHALL BE TO 98% DENSITY. THERE SHALL BE ONE RANDOM DENSITY TEST PER LIFT FOR EVERY 400 FEET. ALL TESTING SHALL BE IN COMPLIANCE WITH CURRENT TXDOT SPECIFICATIONS.

52. A SAND MIGRATION PREVENTION COLLAR SHALL BE INSTALLED WHEN TRANSITIONING FROM SELECT INITIAL BACKFILL TO OPTIONAL SELECT INITIAL BACKFILL. A 2-FOOT LONG CLASS B CONCRETE ENCASEMENT BETWEEN THE TWO SHALL BE PROVIDED FOR THE ENTIRE HEIGHT OF THE INITIAL BACKFILL EVERY 180 FEET ALONG PIPE AND 20 FEET FROM WALL OF MANHOLE IN EACH DIRECTION.

<u>WATERTIGHT TESTING (24\_HOURS):</u> 84. ALL INTERCEPTORS SHALL BE WATER TESTED OUT AT JOB SITE AFTER BEING INSTALLED (PLUG BOTH ENDS AND FILL TO TOP

- OF INTERCEPTOR), INTERCEPTOR SHALL SHOW NO LEAKAGE FROM SECTION SEAMS, PINHOLES, OR OTHER IMPERFECTIONS. ANY LEAKAGE IS CAUSE FOR REJECTION. WHEN LEAKAGE OCCURS, ADDITIONAL WATER TESTING SHALL BE MADE. AFTER CORRECTING MEASURE TEST, REPORTS SHALL SHOW TOTAL NUMBER OF INTERCEPTERS TESTED. WHEN LEAKAGE OCCURS, CORRECTIVE MEASURES TAKEN SHALL BE REPORTED BY THE RIVER AUTHORITY INSPECTORS. RIVER AUTHORITY INSPECTORS SHALL RECORD IN DAILY LOG WITH PROJECT NAME, DATE IT WAS TESTED AND COMPLETED.
- 84.1. MANHOLES WILL BE REQUIRED ON SIX (6) INCH AND LARGER LATERALS WHERE THEY CONNECT TO THE MAIN.
- 84.2. LATERALS WILL NOT BE ATTACHED TO SEWER MAINS THAT ARE DEEPER THAN TWELVE (12) FEET.
- 84.3. FITTINGS ARE NOT PERMITTED ON LATERALS BETWEEN THE WYE AND THE PROPERTY LINE. 84.4. DEEP CUT OR DROP CONNECTIONS SHALL NOT BE
- PERMITTED 84.5. A MINIMUM OF ONE (1) LATERAL PER BUILDING SHALL BE REQUIRED. ALSO, A MINIMUM OF ONE (1) LATERAL PER RESIDENTIAL LOT SHALL BE REQUIRED. DUPLEXES SHALL HAVE TWO (2) LATERALS THAT SHALL BE INDEPENDENTLY
- ATTACHED TO THE MAIN. 84.6. ALL SEWER LATERAL CROSSING WATER MAINS OR WATER SERVICE LINES SHALL CONFORM TO THE SAME REQUIREMENTS OF TAC CHAPTER 217.53, LATEST REVISION, SDR 26 150 PSI, OR DUCT IRON PIPE, CONCRETE ENCASMENT
- 85. WHERE REQUIRED CONCRETE ENCASEMENT SHALL BE PLACED FOR FULL WIDTH OF THE TRENCH TO A PLAIN SIX (6) INCHES ABOVE THE TOP OF THE PIPE WITH PAY UNITS AS SHOWN ON THE STANDARD DETAIL SHEET.
- 86. A MINIMUM OF FOUR (4) FEET OF COVER IS TO BE MAINTAINED OVER THE SANITARY SEWER MAIN AND LATERALS AT GRADE, OTHERWISE CONCRETE ENCASEMENT IS REQUIRED.
- 87. WHERE POROUS MATERIAL INCLUDING "SUBGRADE FILLER" IS USED FOR BACKFLL IN THE BEDDING AND INITIAL BACKFILL ZONES, REFER TO SPECIFICATIONS SEC. 33-05-05 FOR SPACING OF SEEPAGE RETAINERS RETAINERS SHALL CONSIST OF CLASS "D" CONCRETE ENCASEMENT OR FIRMLY COMPACTED. CONSOLIDATED CLAY ENCASEMENT, THE RETAINERS SHALL EXTEND FROM THE BOTTOM OF THE TRENCH TO THE TOP OF THE GRANULAR MATERIAL FOR THE ENTIRE TRENCH WIDTH. ENCASEMENT SHALL BE 24 INCHES LONG. NO EXTRA PAY ITEM.

BLASTING

TESTING

88. BLASTING SHALL NOT BE ACCEPTABLE.

89. TESTING SHALL NOT BE CONDUCTED UNTIL ALL OTHER UTILITIES WITHIN THE VICINITY OF THE **SANITARY** SEWER ARE FULLY INSTALLED.

- B. PERFORM AIR TEST C. PULL WIPER (AFTER STREET HAS BEEN ASPHALTED), AS
- APPLICABL D. VACUUM TEST ALL MANHOLES WITHIN THE PROJECT E. CCTV- ALL OF THE NEW LINES AND PAN/TILT ALL SERVICE

LINES BEFORE CCTV. AT END OF PROJECT, CONTRACTOR SHALL SUBMIT FIELD COPY PLAN AND PROFILES SHOWING AS-BUILT WORK, CCTV DVD, AND COMPACTION DENSITY REPORTS FOR MAIN SEWER LINES AND ALL SERVICE LATERALS. CONTRACTOR SHALL ALSO ISSUE WARRANTY LETTERS FOR MATERIAL AND WORKMANSHIP FOR

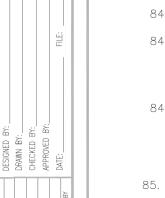
12 MONTHS AFTER FINAL ACCEPTANCE. 90. ALL SEWER LINES MUST BE TESTED IN ACCORDANCE WITH THE FOLLOWING:

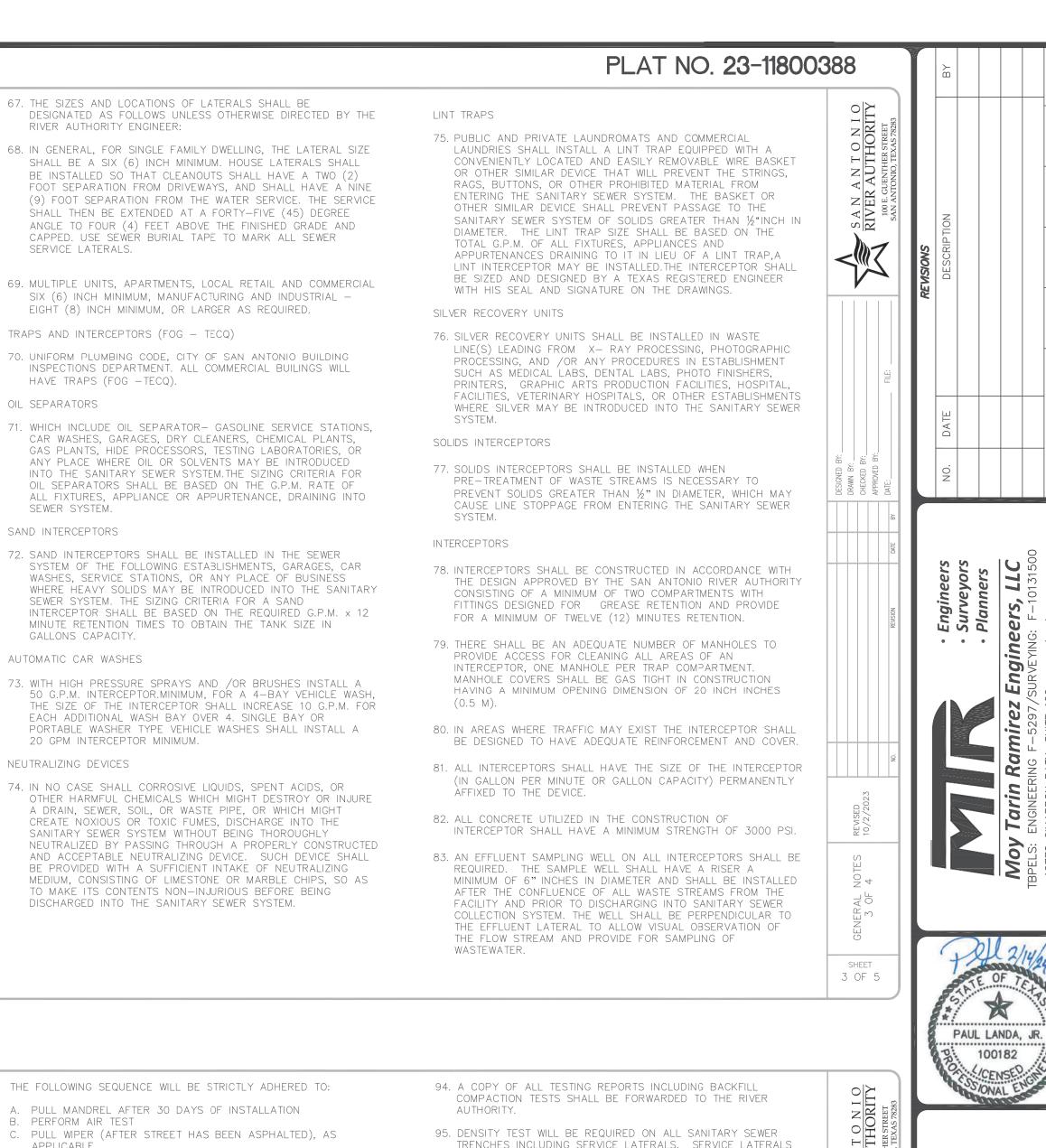
- LEAST 30 DAYS.
- OR EXFILTRATION AND OR LOW-PRESSURE AIR TEST.
- C. 217.58 OR RIVER AUTHORITY SPECIFICATIONS: ALL MANHOLES AND WET WELLS MUST BE TESTED SEPARATELY AND INDEPENDENTLY OF THE COLLECTION LINES.

D. IN THE EVENT THAT TESTING REQUIREMENTS CONFLICT, THE LATEST TCEQ DESIGN CRITERIA SHALL BE USED. 91. SEWER LINES SHALL BE TESTED FROM MANHOLE TO MANHOLE.

- FOLLOW MANUFACTURER'S RECOMMENDATION ON PROTECTIVE
- CAMERA BY THE CONTRACTOR AND OBSERVED BY THE INSPECTOR, WASTEWATER ENGINEERING PERSONNEL AND CONTRACTOR AS CAMERA IS RUN THROUGH THE LINES. JOINT, GRAVEL, DIRT, MUST BE CLEANED OUT, REPLACE TO STUB-OUT. ALL VIDEOS SHALL BE SUBMITTED IN

COATING.





LATERALS TO 6"X6" CLEAN OUT. CONTRACTOR SHALL FLOOD ALL

A. 217.57: DEFLECTION TEST FOR FLEXIBLE AND SEMI-RIGID PIPE CONDUCTED AFTER FINAL BACKFILL HAS BEEN IN PLACED AT

B. 217.57, OR RIVER AUTHORITY SPECIFICATIONS INFILTRATION AND

92. SANITARY SEWER CONNECTIONS MADE DIRECTLY TO EXISTING MANHOLES WHICH REQUIRE PENETRATION INTO THE MANHOLE WILL BE CORE DRILLED. ANY DAMAGE TO EXISTING MANHOLE WILL BE REPLACED AT CONTRACTOR'S EXPENSE AND WILL REQUIRE SUCCESSFUL TESTING OF THE EXISTING MANHOLE IN ACCORDANCE WITH THE RIVER AUTHORITY SPECIFICATIONS. THEY MUST HAVE A PROTECTIVE COATING SPECIFIED IN THE RIVER AUTHORITY APPROVED PRODUCTS LIST, COATING WILL BE MINIMUM OF 200 MILS THICKNESS DEPENDING ON EXISTING CONDITIONS, TO PREVENT INFRASTRUCTURE INFILTRATION,

93. AFTER CONSTRUCTION, TESTING WILL BE DONE BY PAN/TILT TV PAN/TILT ALL 6" SERVICE LATERALS TO 6"X6" STUB-OUT. VIDEOS MUST INCLUDE SUBDIVISION NAME, MANHOLE NUMBER, SERVICE LATERAL STATION NUMBER, FLOW DIRECTION, LOCATION ANY ABNORMALITIES, SUCH AS BROKEN PIPE OR MISALIGNED, AT CONTRACTOR'S EXPENSE. NEW SEWER SYSTEM WILL BE FLOODED WITH H20 BEFORE BEING TV. ALL SEWER LINES MUST BE PRESSURE CLEANED TO INCLUDE SERVICE LATERALS 6"INCH DVD FORMAT WITH WRITTEN REPORTS.

> SAN ANTONIO **RIVER AUTHORITY** Water Brings Us Together

95. DENSITY TEST WILL BE REQUIRED ON ALL SANITARY SEWER TRENCHES INCLUDING SERVICE LATERALS. SERVICE LATERALS TO BE CHOSEN RANDOMLY BY FIELD INSPECTOR. DENSITIES ON SERVICE LATERAL SHALL NOT EXCEED 25% OF TOTAL NUMBER

EXCAVATION

OF SERVICE.

- 96. CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR STRUCTURAL DESIGN/ GEOTECHNICAL/ SAFETY/ EQUIPMENT CONSULTANT, IF ANY, SHALL REVIEW THESE PLANS AND AVAILABLE GEOTECHNICAL INFORMATION AND THE ANTICIPATED INSTALLATION SITE(S) WITHIN THE PROJECT WORK AREA IN ORDER TO IMPLEMENT CONTRACTORS TRENCH EXCAVATION SAFETY PROTECTION SYSTEMS, PROGRAMS AND/OR PROCEDURES. THE CONTRACTOR'S IMPLEMENTATION OF THE SYSTEMS, PROGRAMS AND/OR PROCEDURES SHALL PROVIDE FOR ADEQUATE TRENCH EXCAVATION SAFETY PROTECTION THAT COMPLIES WITH AS A MINIMUM OSHA STANDARDS FOR TRENCH EXCAVATIONS. SPECIALLY, CONTRACTOR AND/OR CONTRACTORS INDEPENDENTLY RETAINED EMPLOYEE OR SAFETY CONSULTANT SHALL IMPLEMENT A TRENCH SAFETY PROGRAM IN
- ACCORDANCE WITH OSHA STANDARDS GOVERNING THE PRESENCE AND ACTIVITIES OF INDIVIDUALS WORKING IN AND AROUND TRENCH EXCAVATION. 97. CONTRACTOR IS RESPONSIBLE FOR REMOVAL OF ALL WASTE
- MATERIALS UPON PROJECT COMPLETION. THE CONTRACTOR SHALL NOT STOCKPILE ANY WASTE MATERIAL IN THE 100 YEAR FLOODPLAIN.



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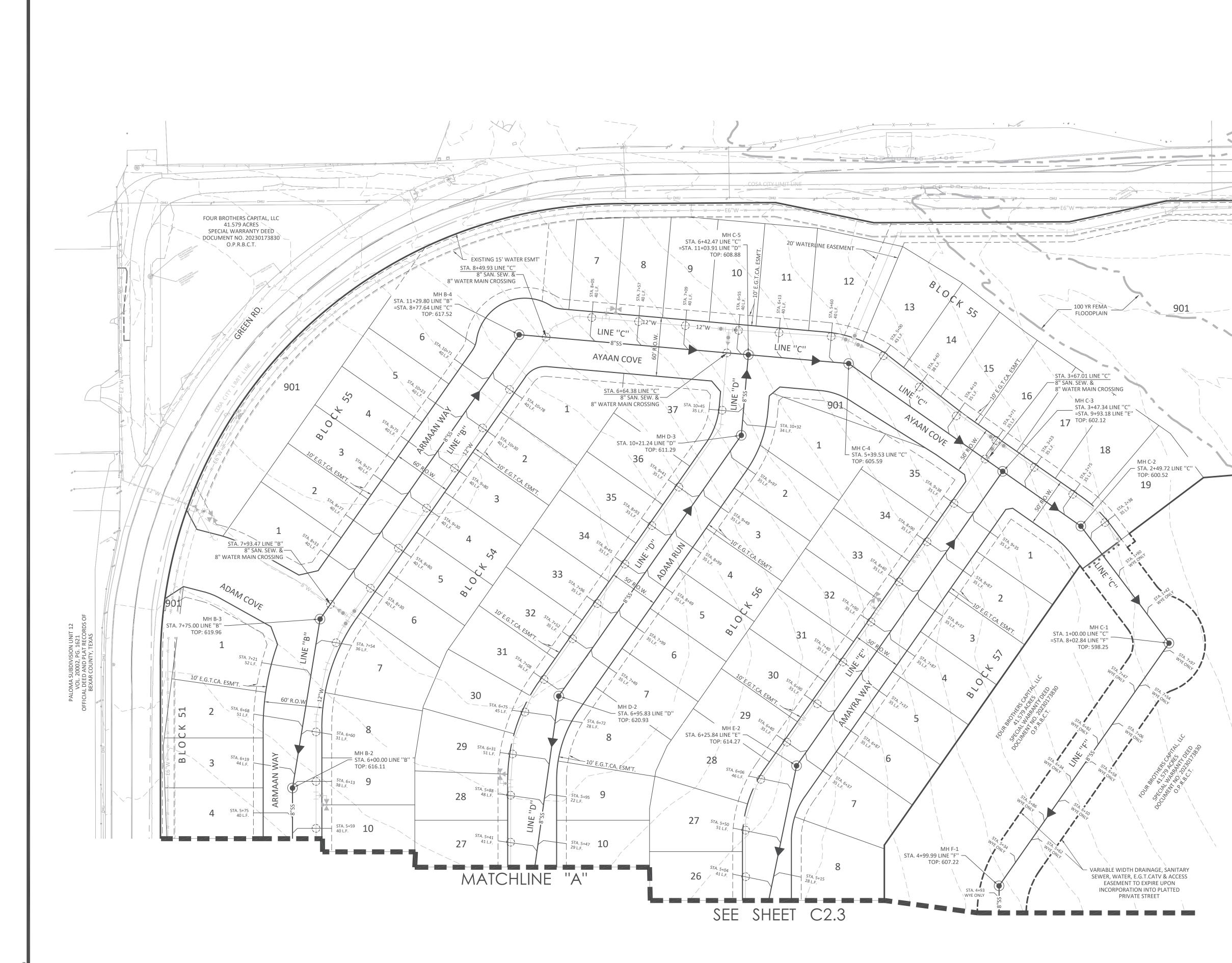
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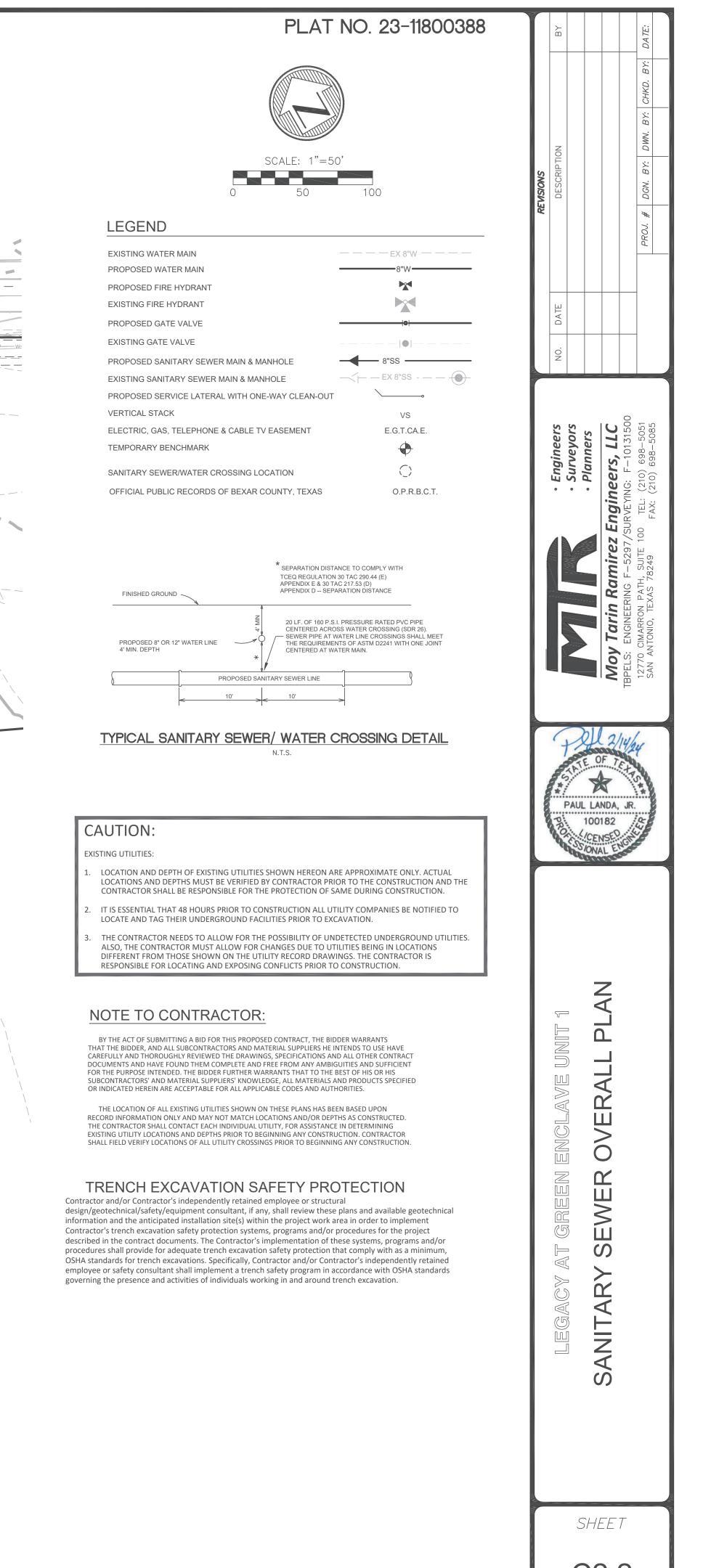
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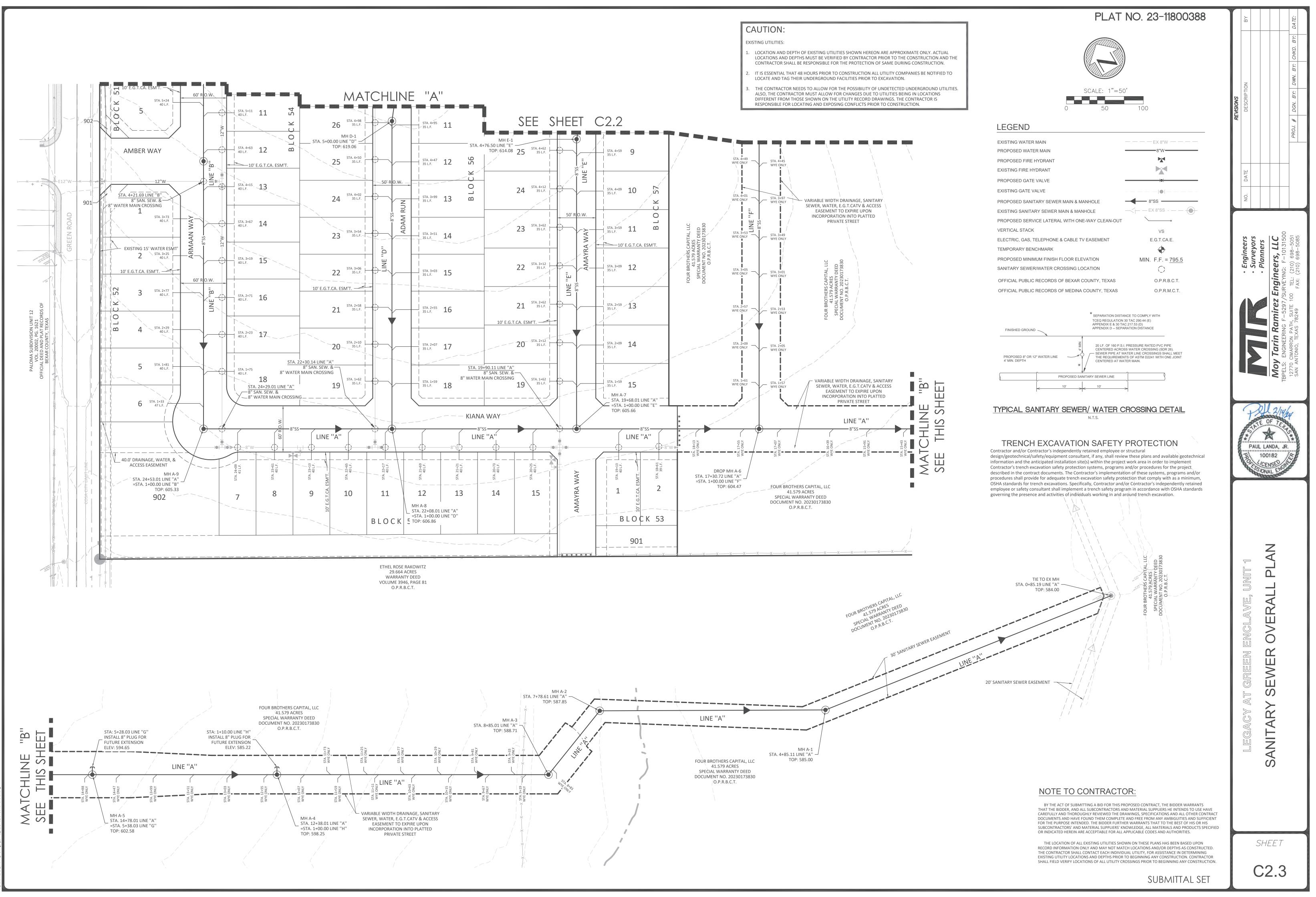


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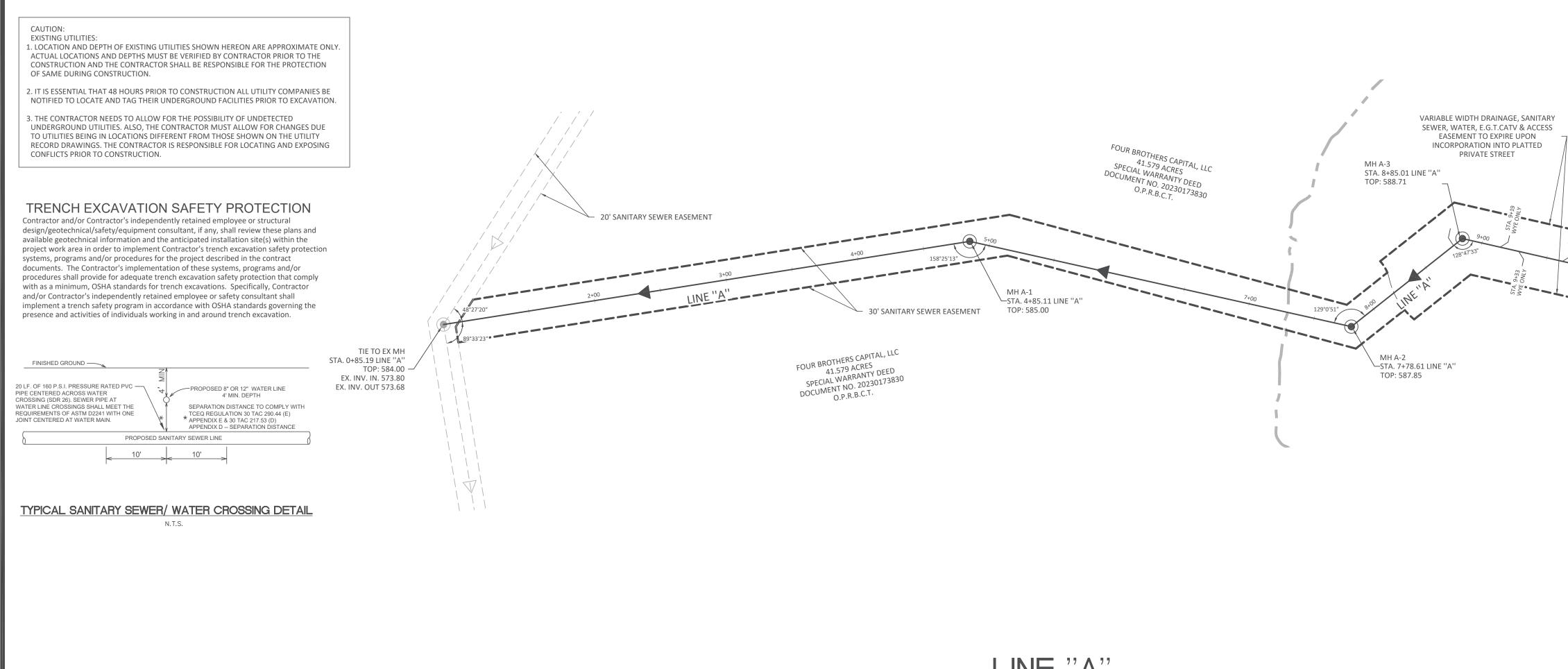


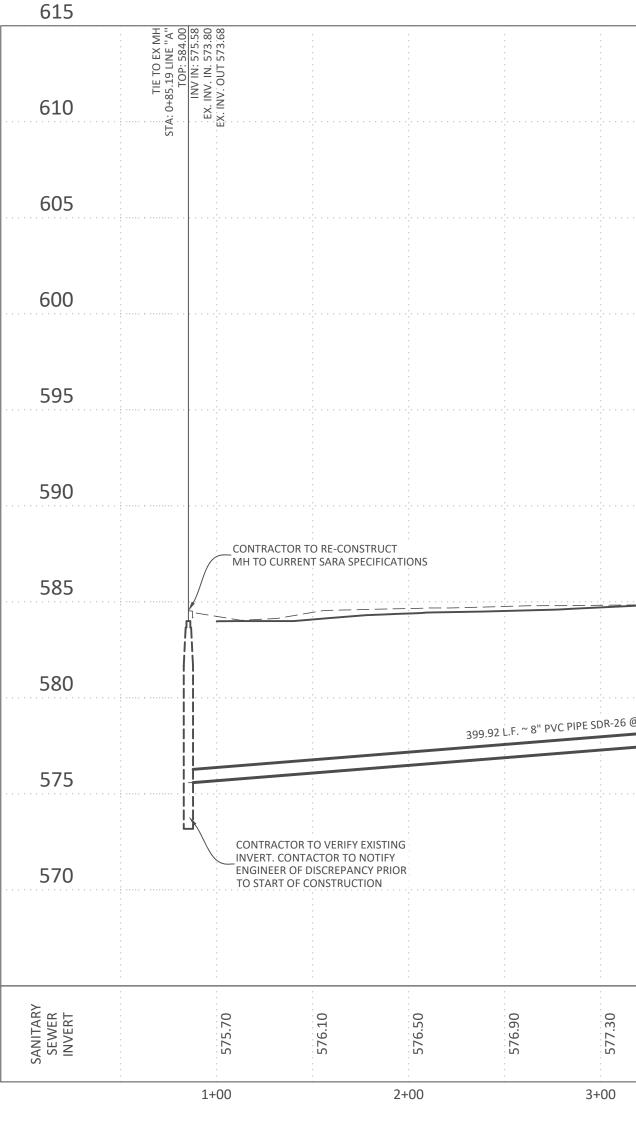
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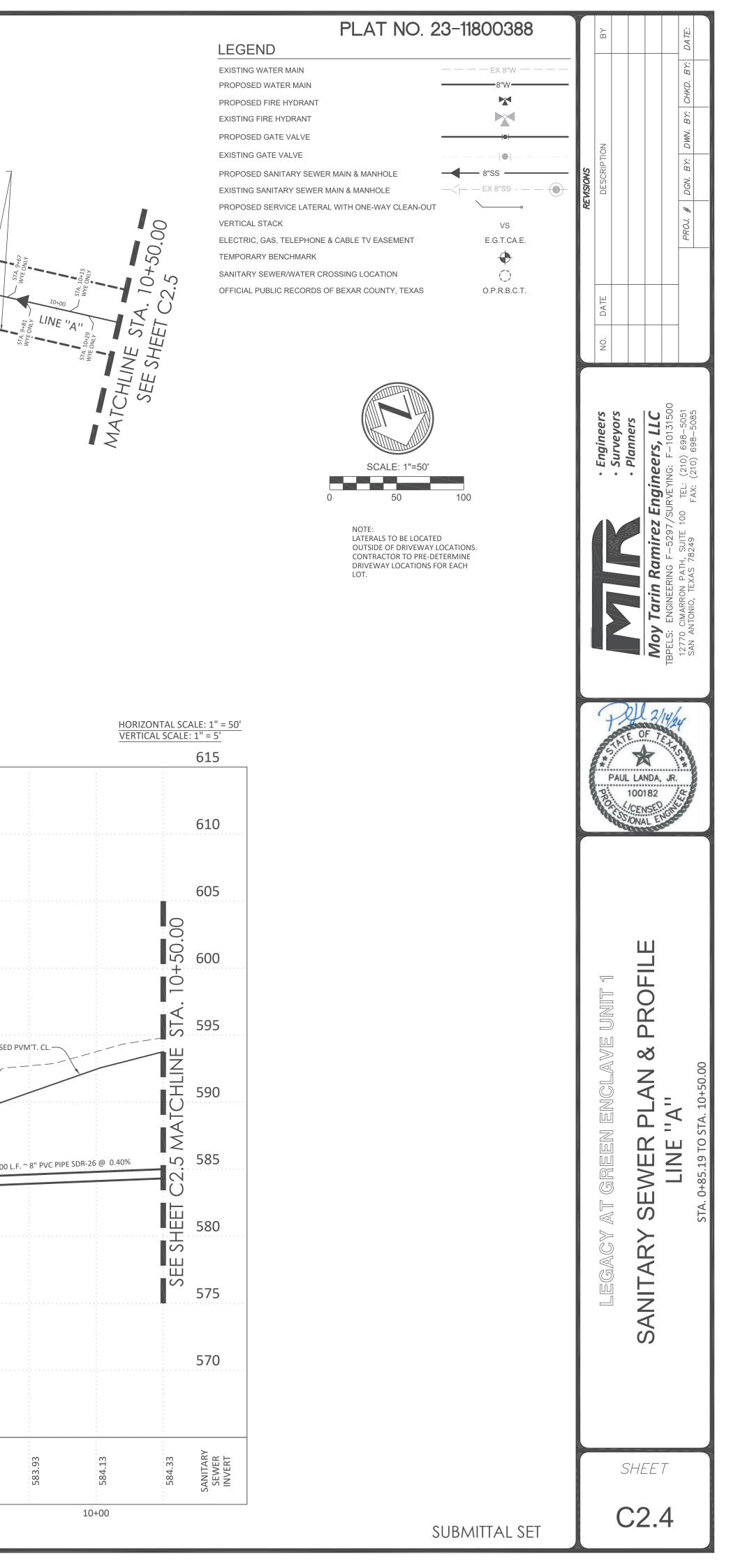




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			MH A-1 STA: 4+85.11 LINE "A" TOP: 585.00 INV IN: 578.87	MANHOLE MUST BE AT LEAST 4" ABOVE GROUND SURFACE. TYP ALL MANHOLES OUTSIDE ROW.					MH A-2 STA: 7+78.61 LINE "A" TOP: 587.85 INV IN: 581.48	MANHOLE MUST BE AT LEAST 4" ABOVE GROUND SURFACE. TYP ALL MANHOLES OUTSIDE ROW.	MH A-3 STA: 8+85.01 LINE "A" TOP: 588.71	MANHOLE MUST BE AT LEAST 4" ABOVE GROUND SURFACE. TYP ALL MANHOLES OUTSIDE ROW.
												PROPOSED
26 @ 0.80%				EXISTING	GROUND CL.	L.F. ~ 8" PVC PIPE SD	DR-26 @ 0.86%			106.40 L.F.~ 8" P PIPE SDR-26 @ 1.	VC 96%	353.00 L.
	02.77.2	578.09	578.49	579.00	579.43	579.86	580.29	580.71	581.14	581.90 581.90	582.88	283.73 283.73 00+6



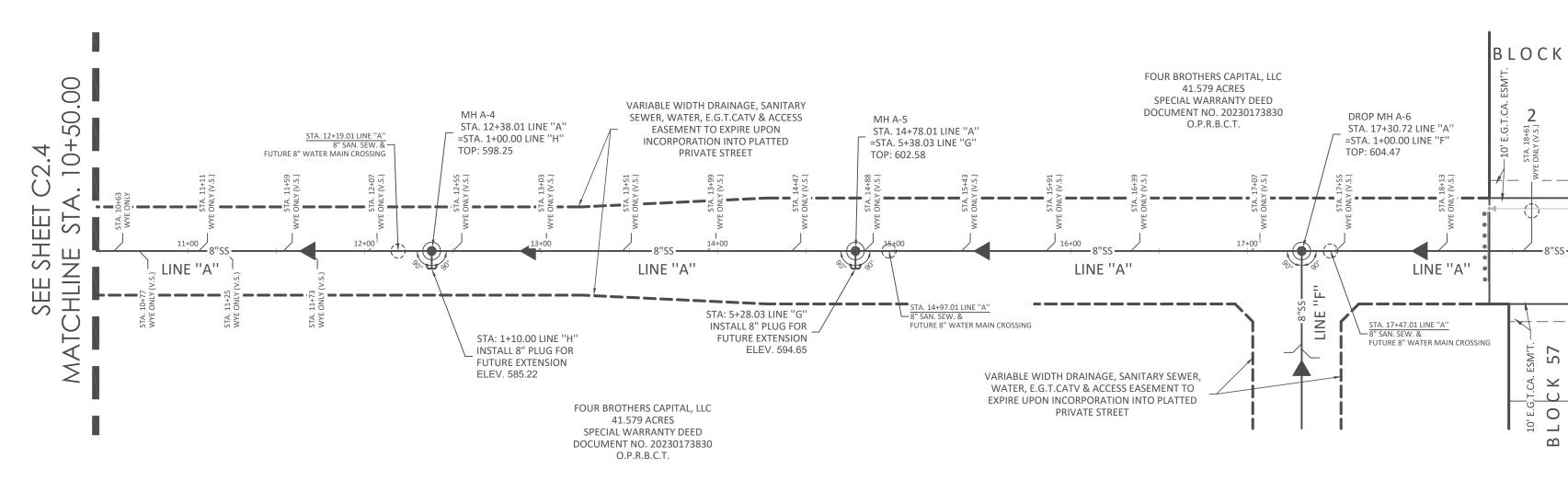
CAUTION: EXISTING UTILITIES:

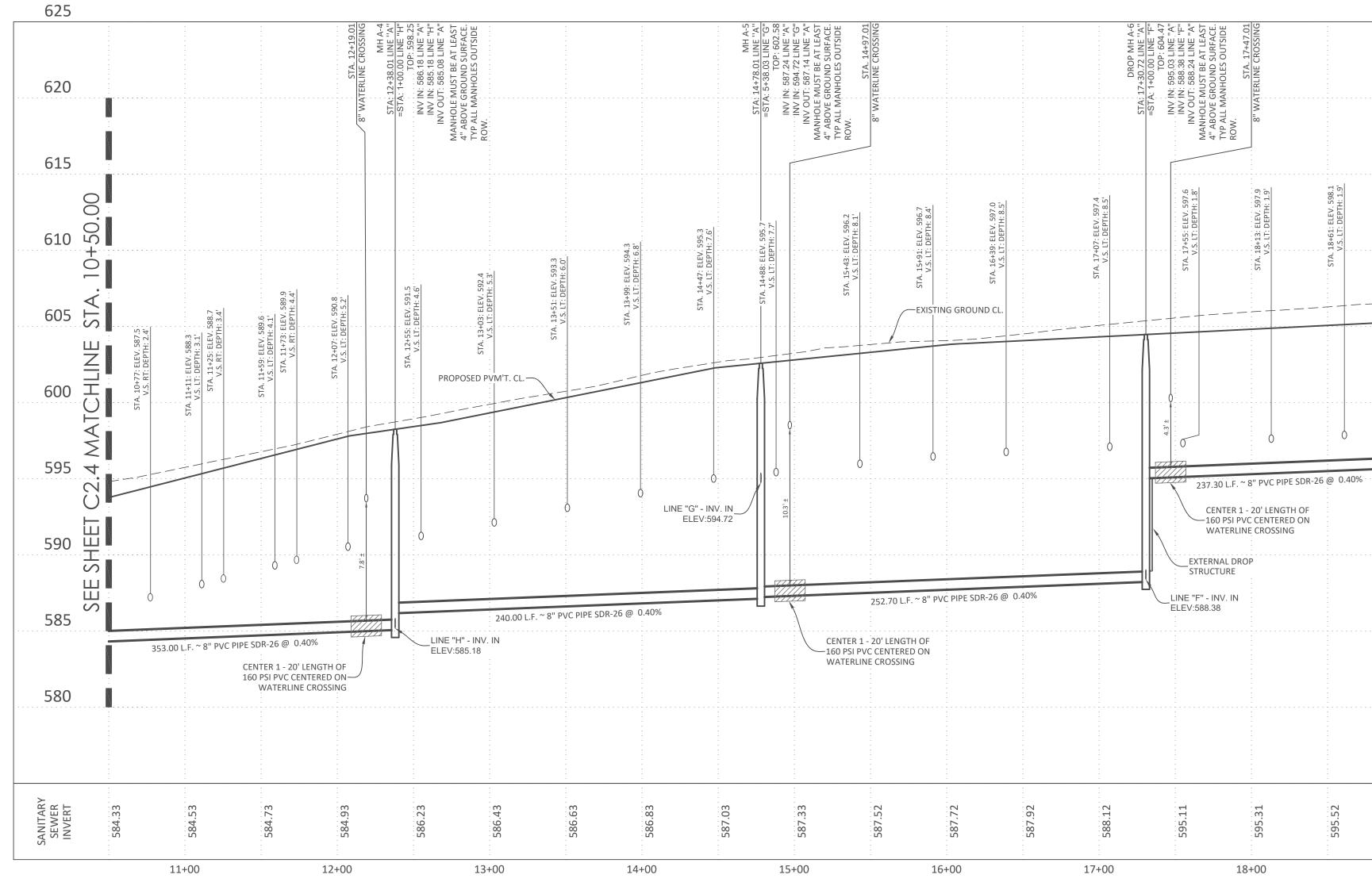
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PROPOS	ED SANITARY SEWER LINE
<u> </u>	

TYPICAL SANITARY SEWER/ WATER CROSSING DETAIL N.T.S.

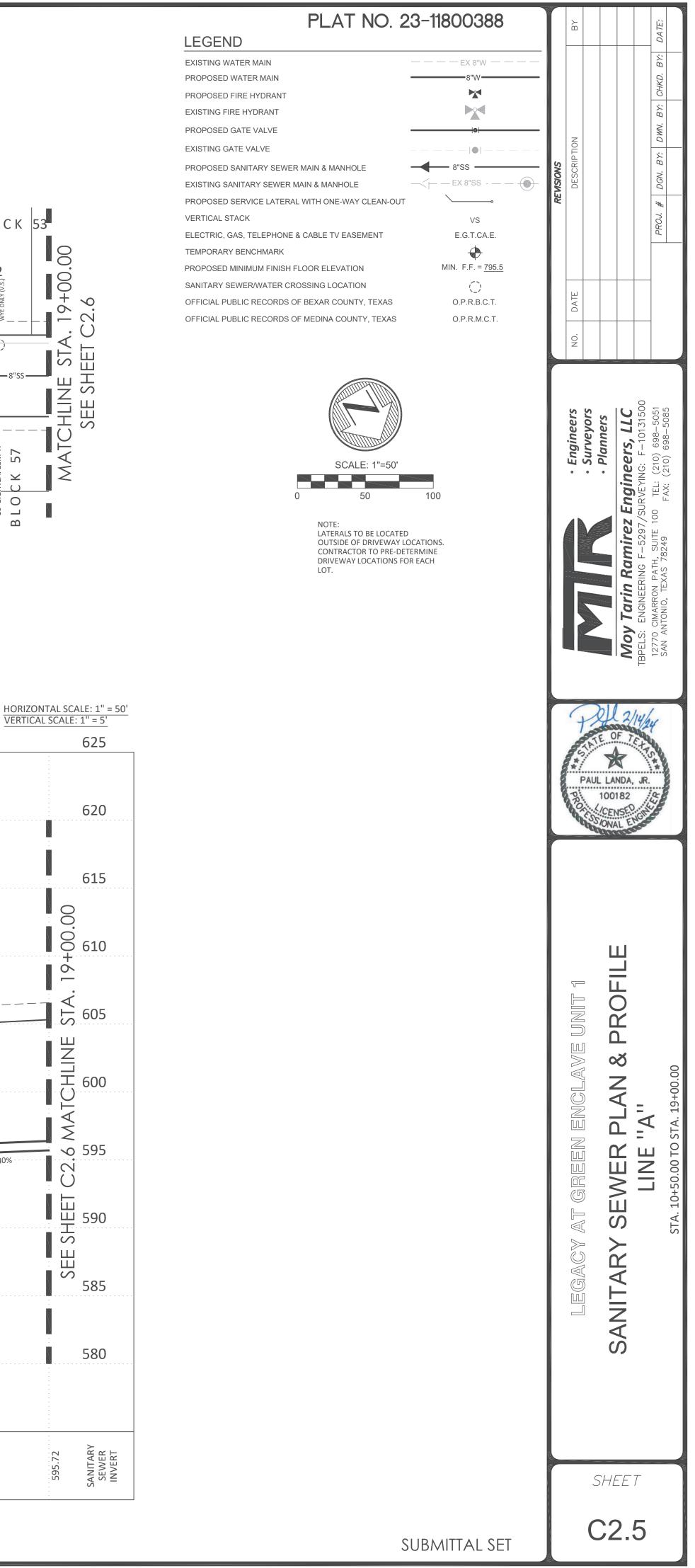




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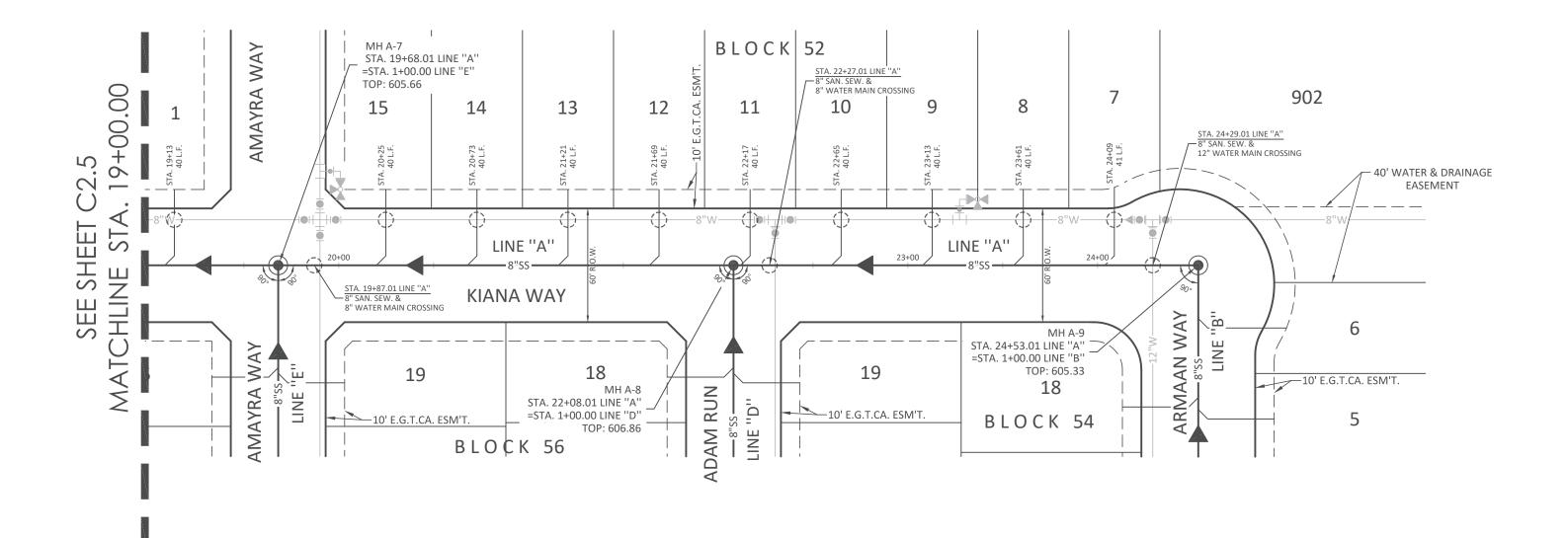
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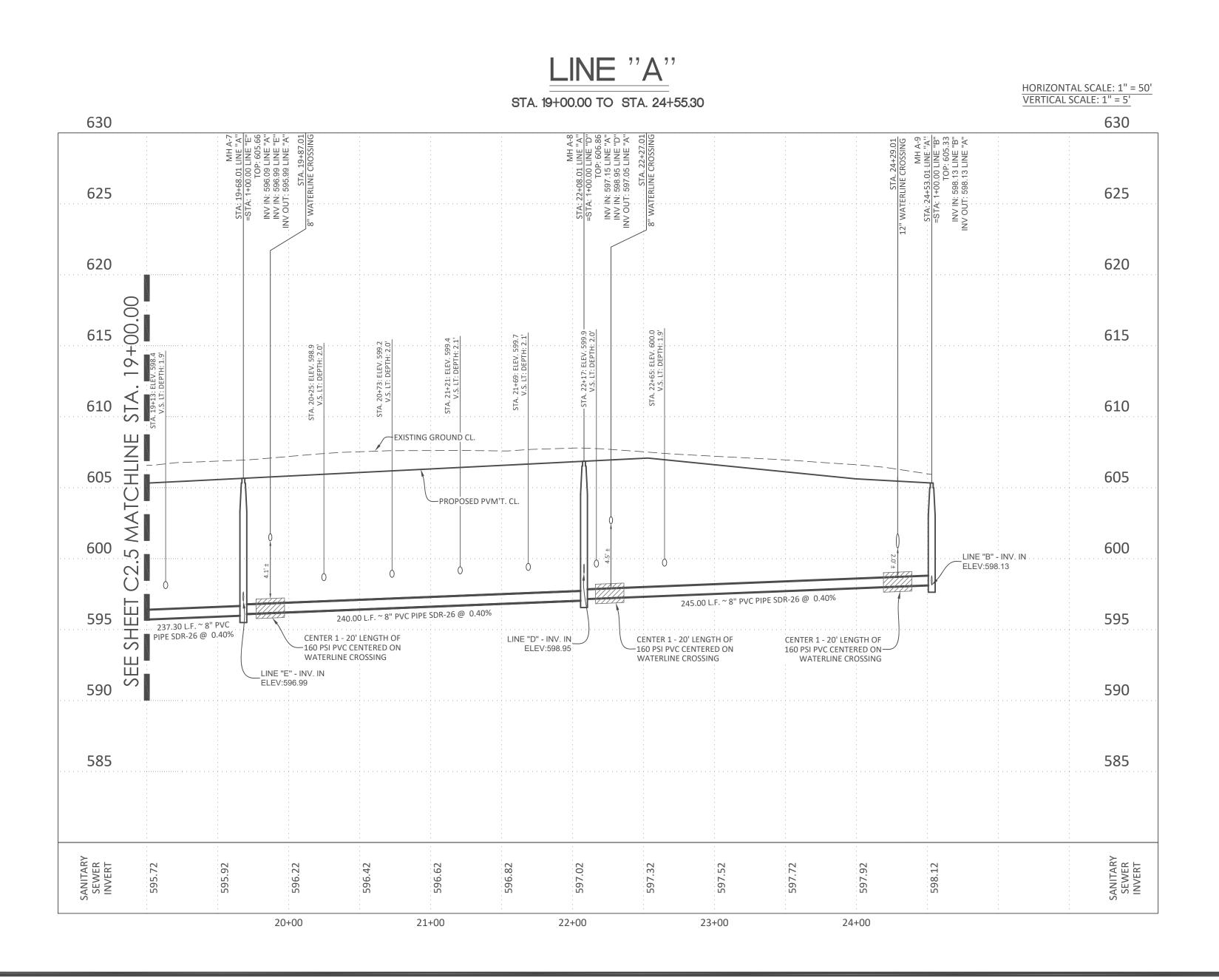
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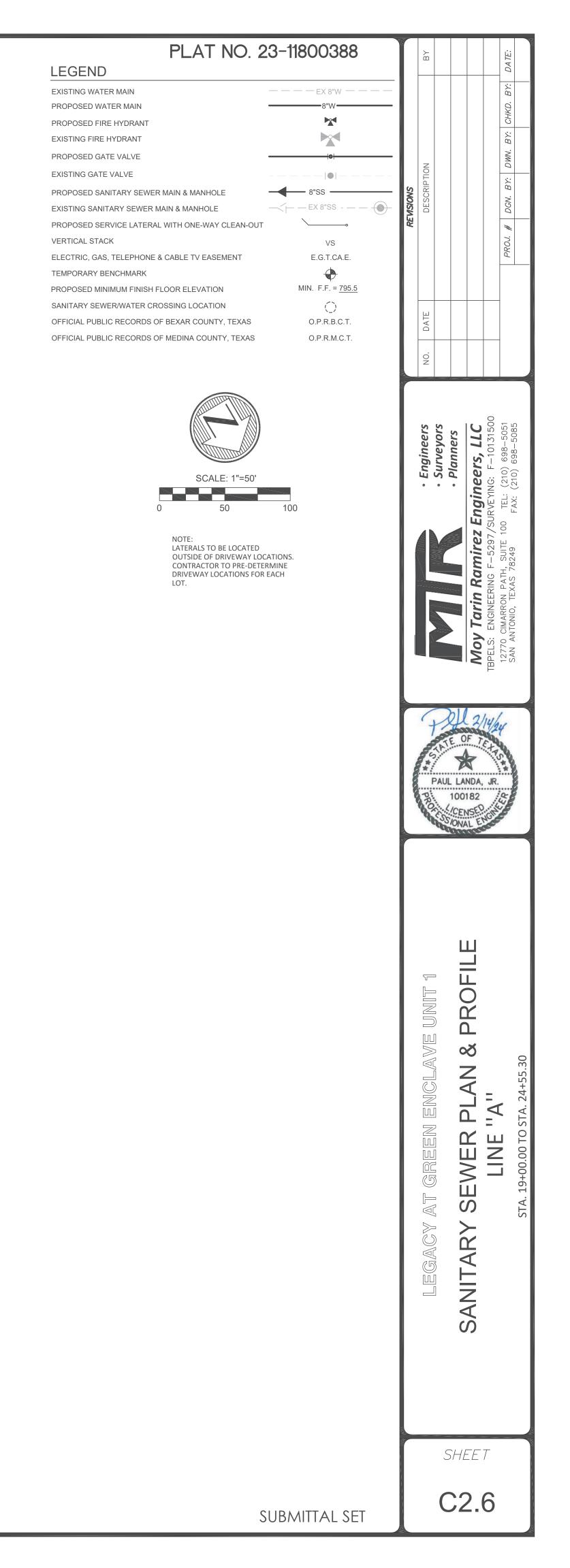
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	APPENDIX D SEPARATION DISTANCE
0	< 10'
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TYPICAL SANITARY SEWER/ WATER CROSSING DETAIL







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APPENDIX D SEPARATION DISTANCE	<b>8</b> 10' E.G.T.CA. ESM'T. —
TYPICAL SANITARY SEWER/ WATER CROSSING DETAIL N.T.S.	
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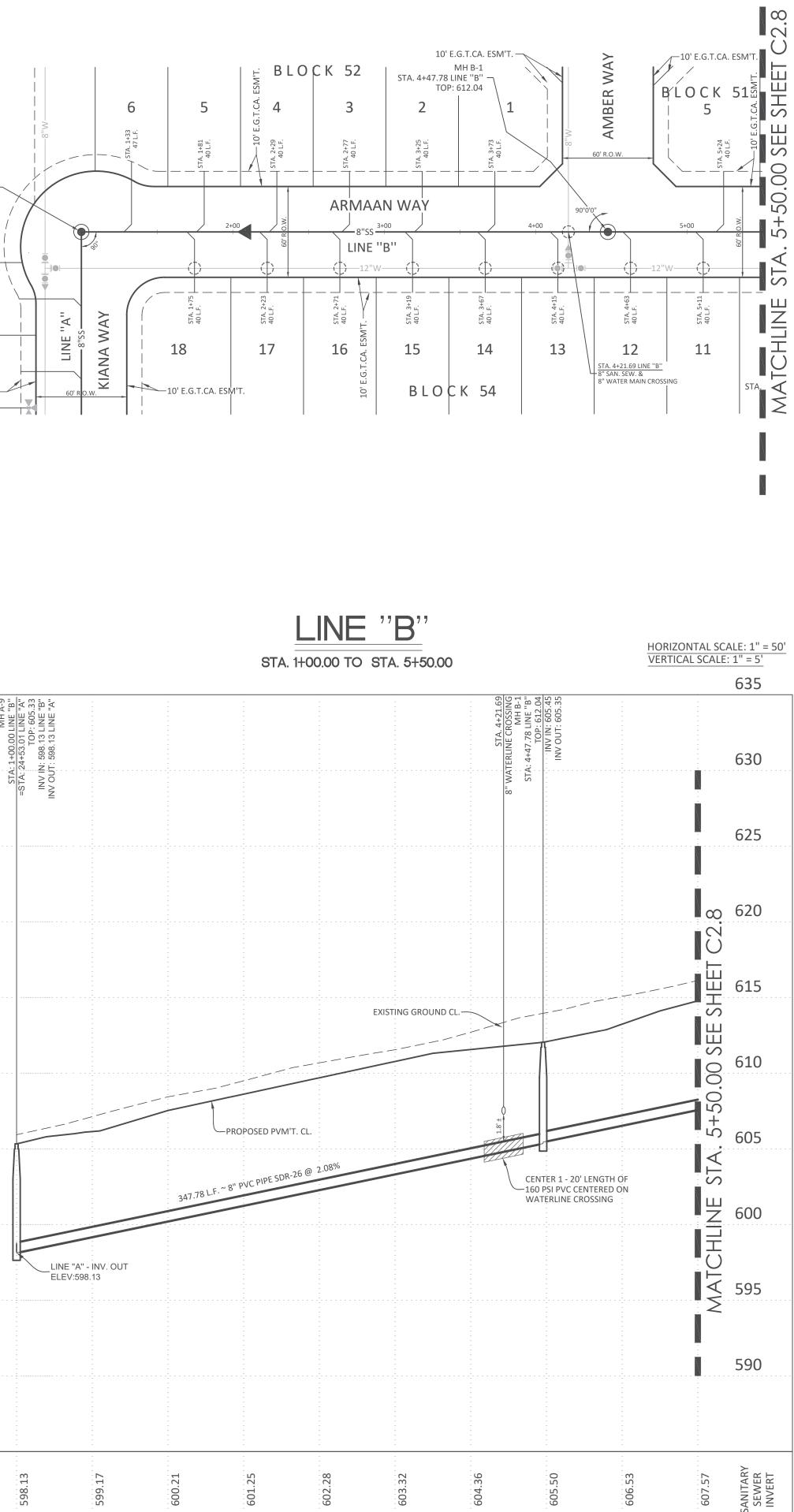
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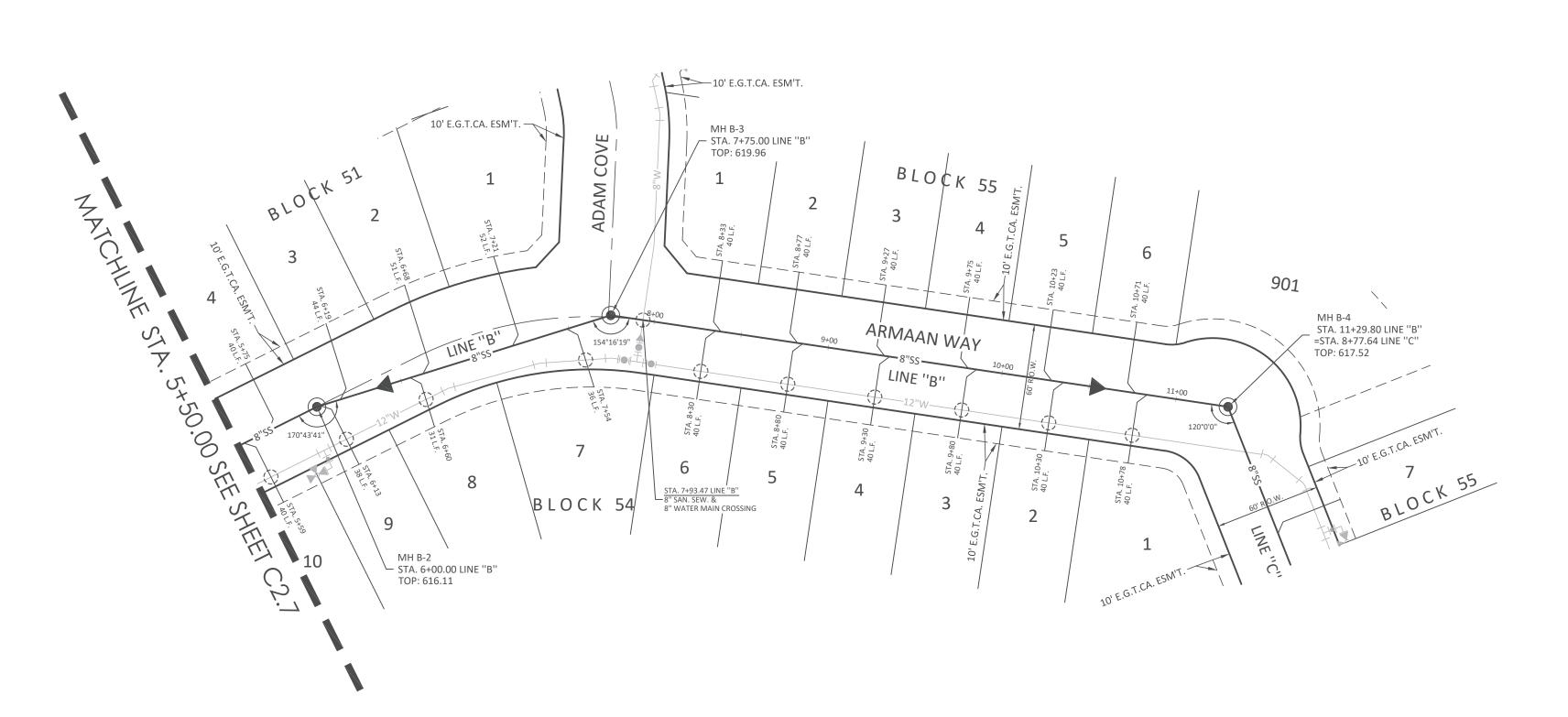
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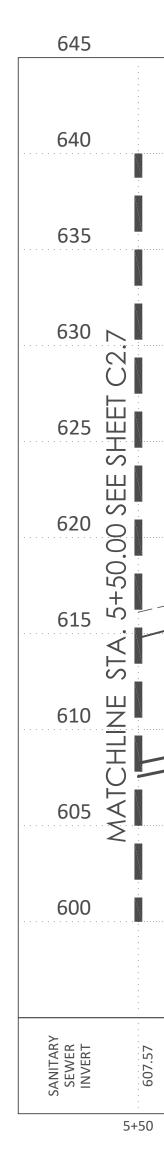
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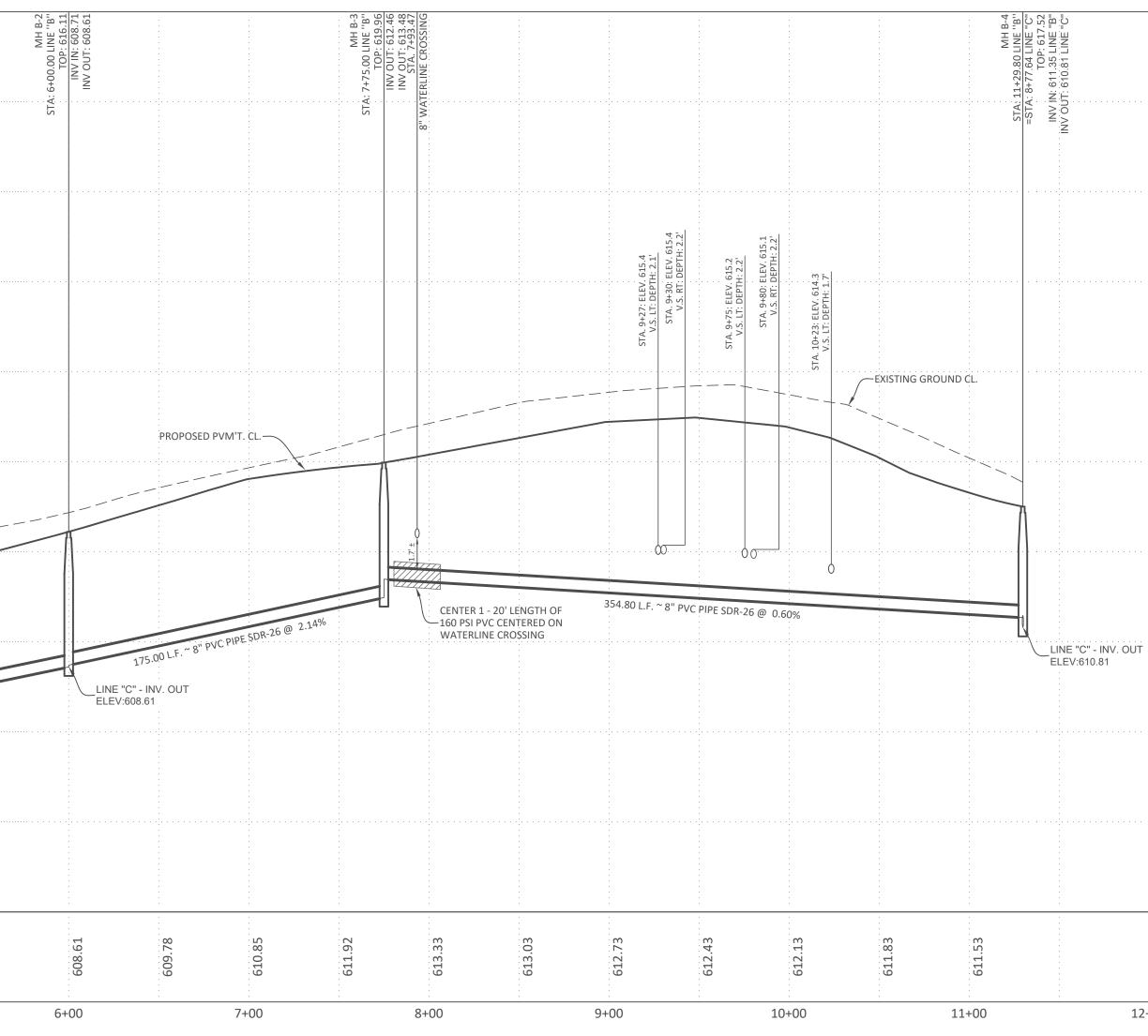


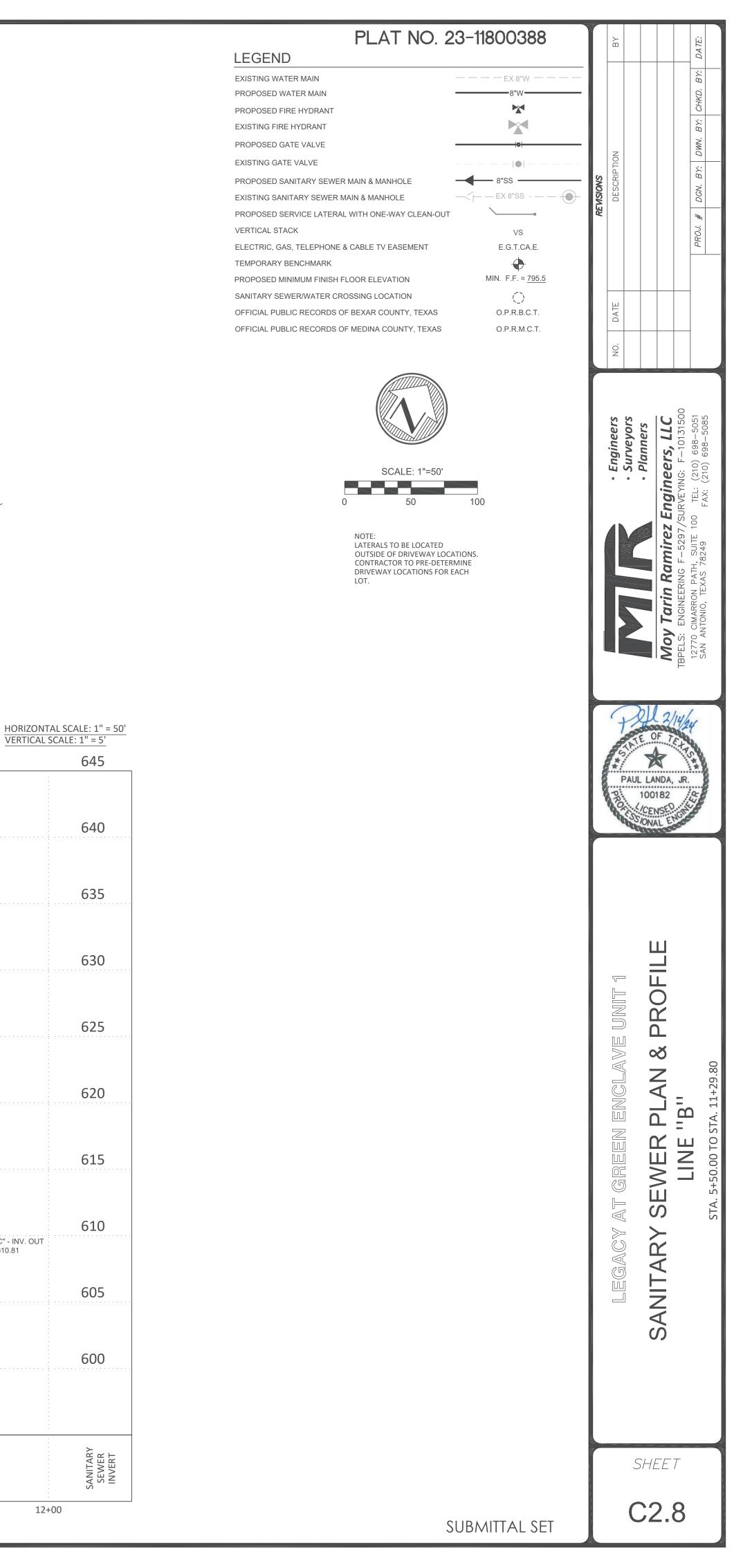


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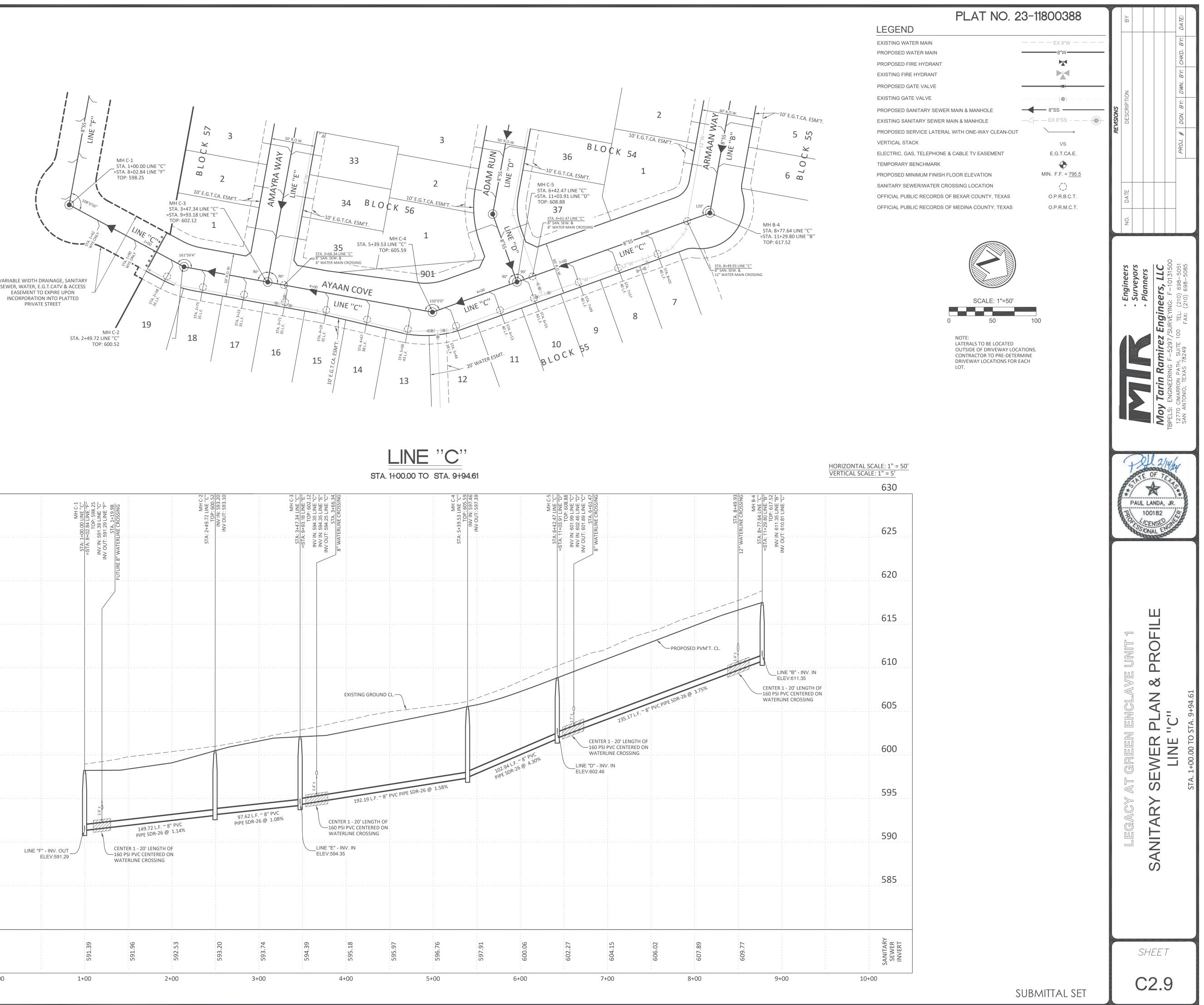


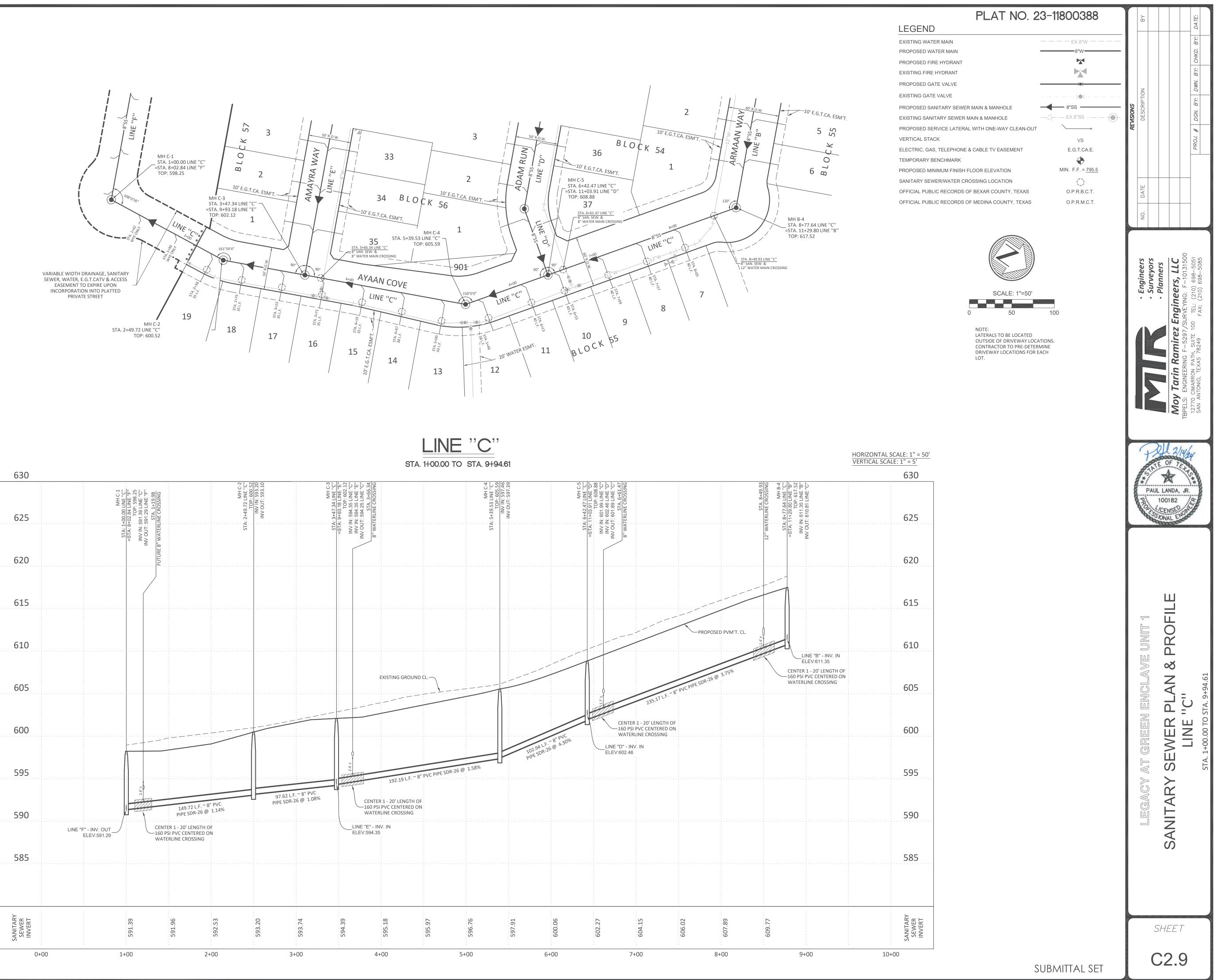
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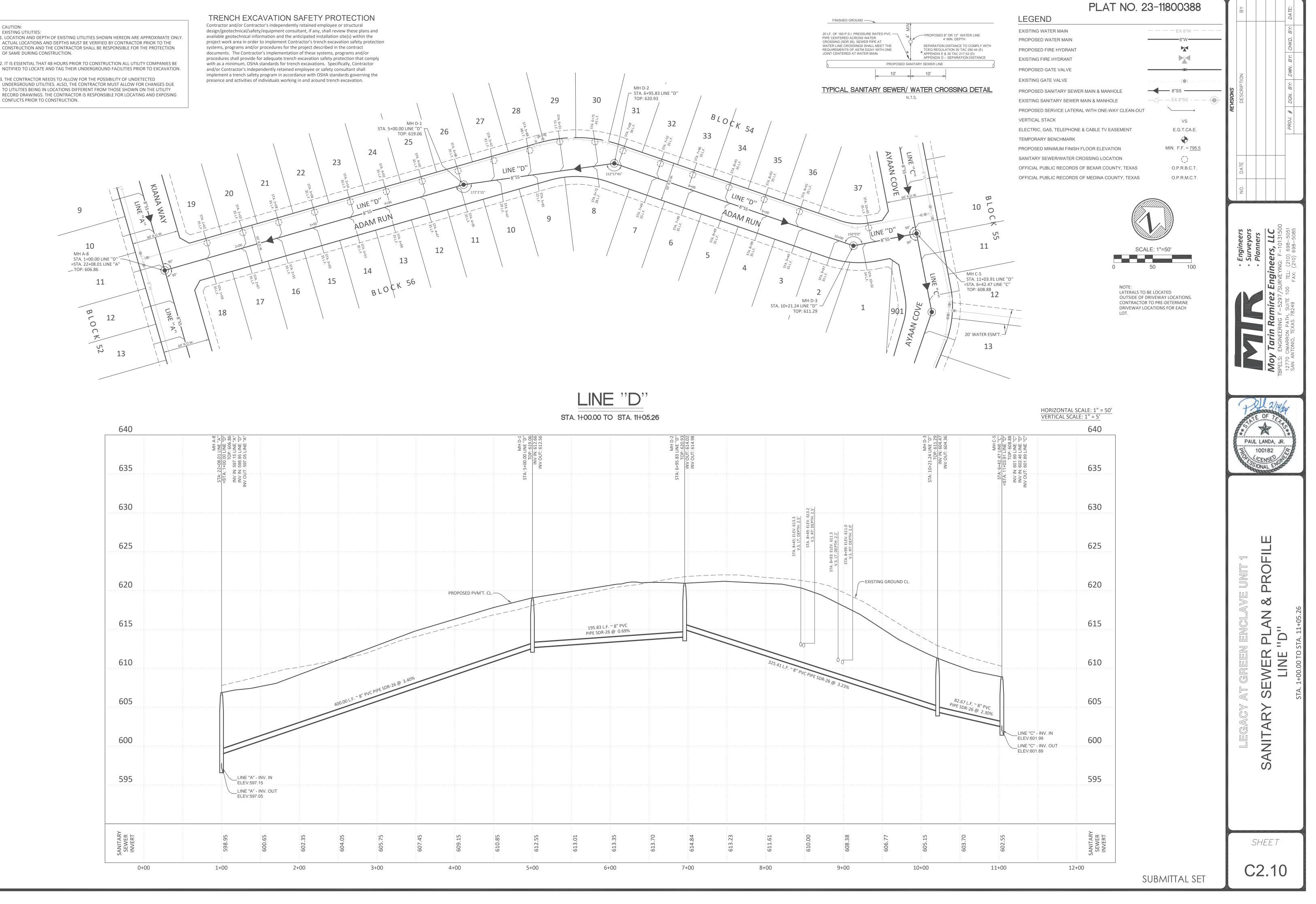
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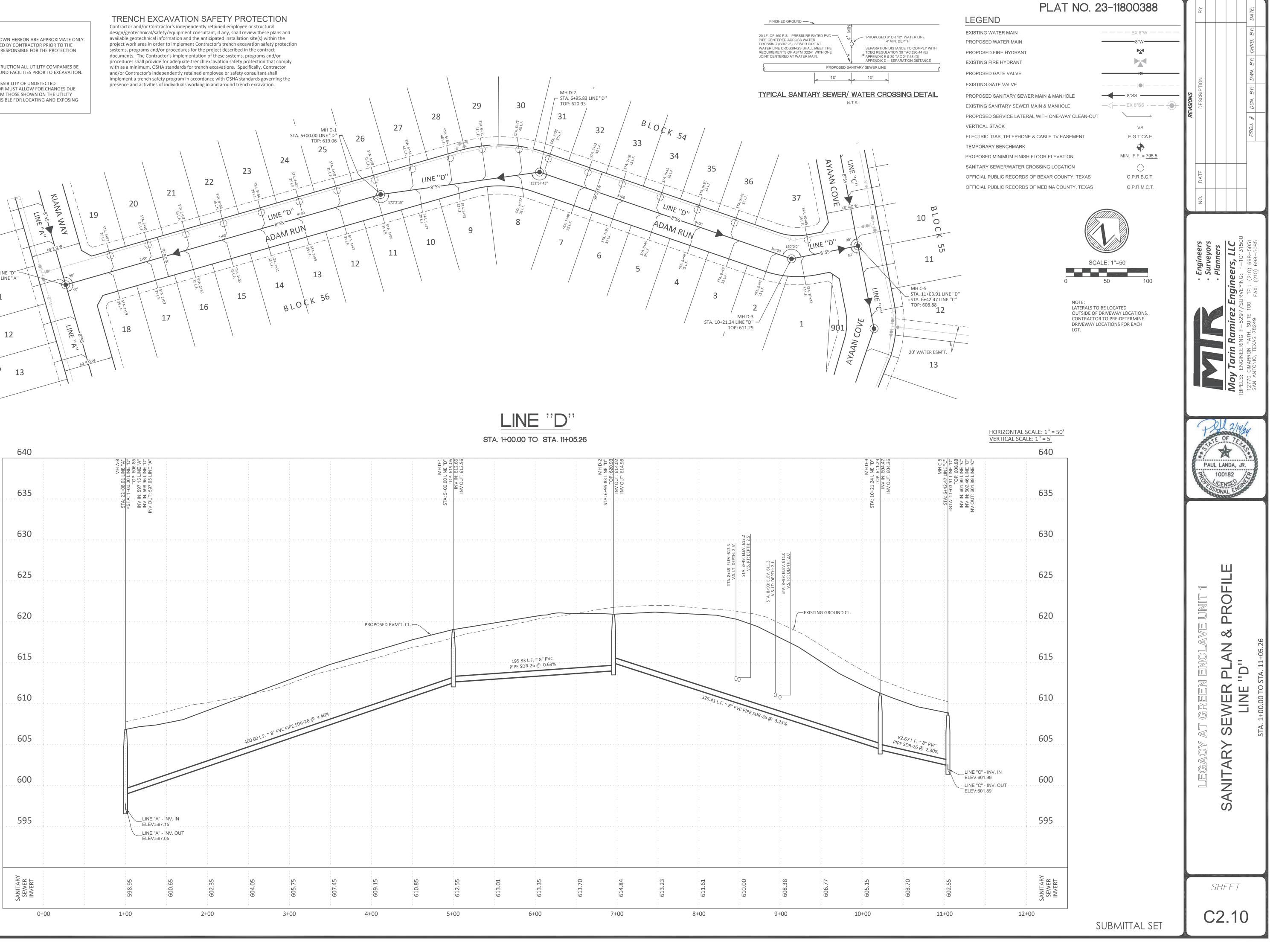
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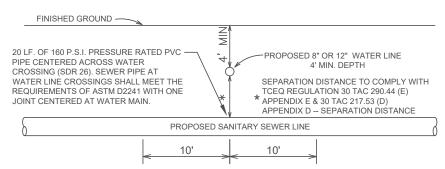
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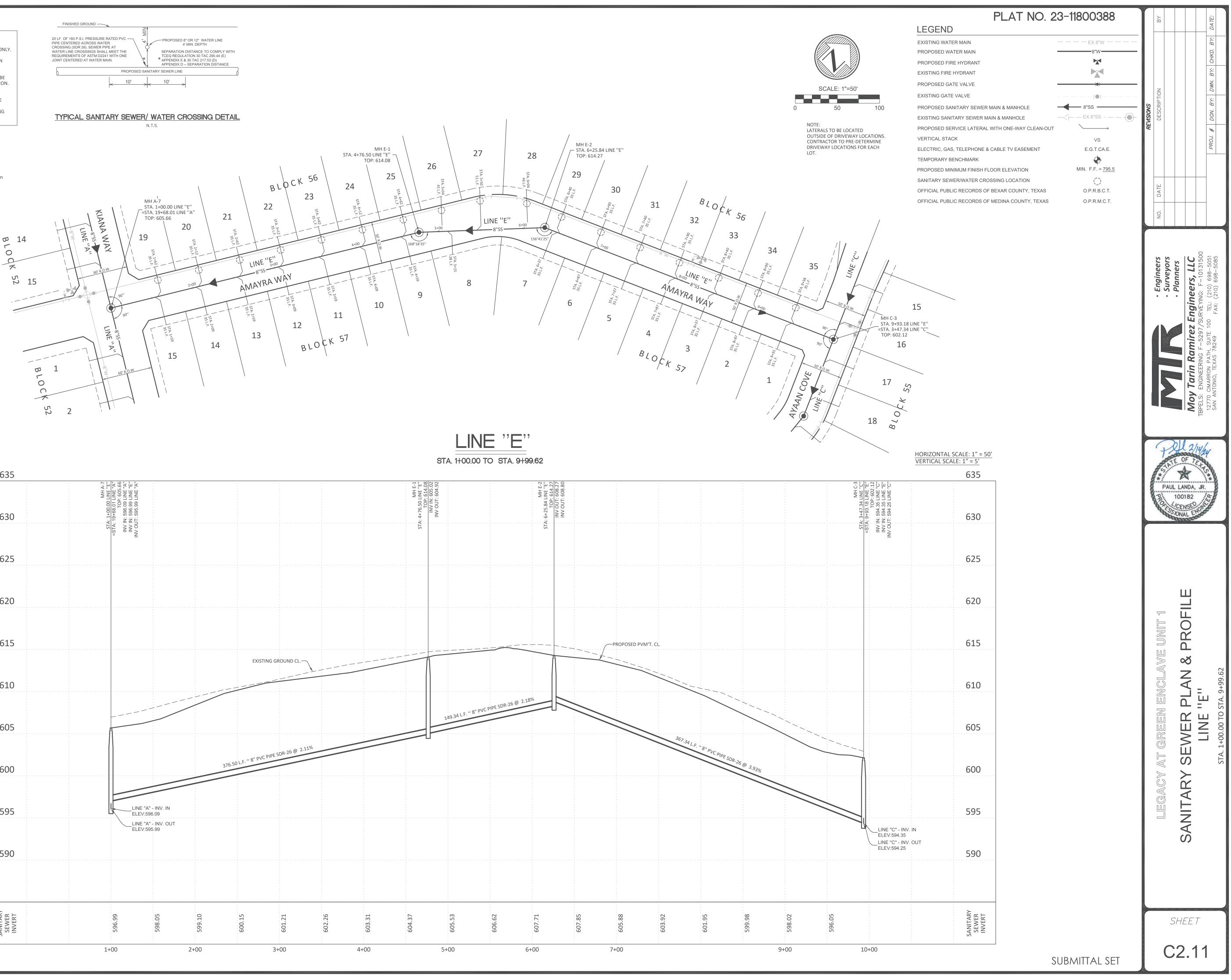


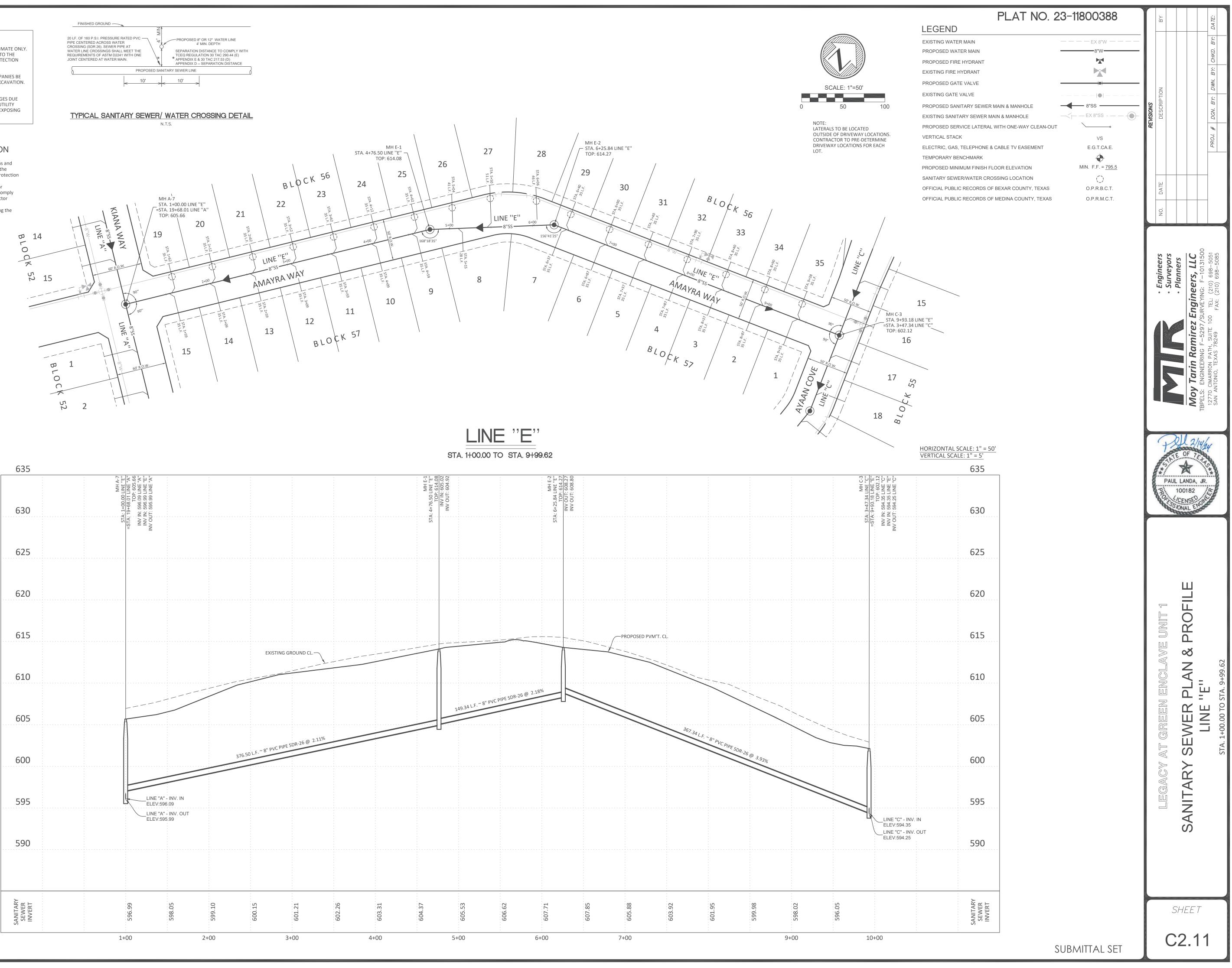


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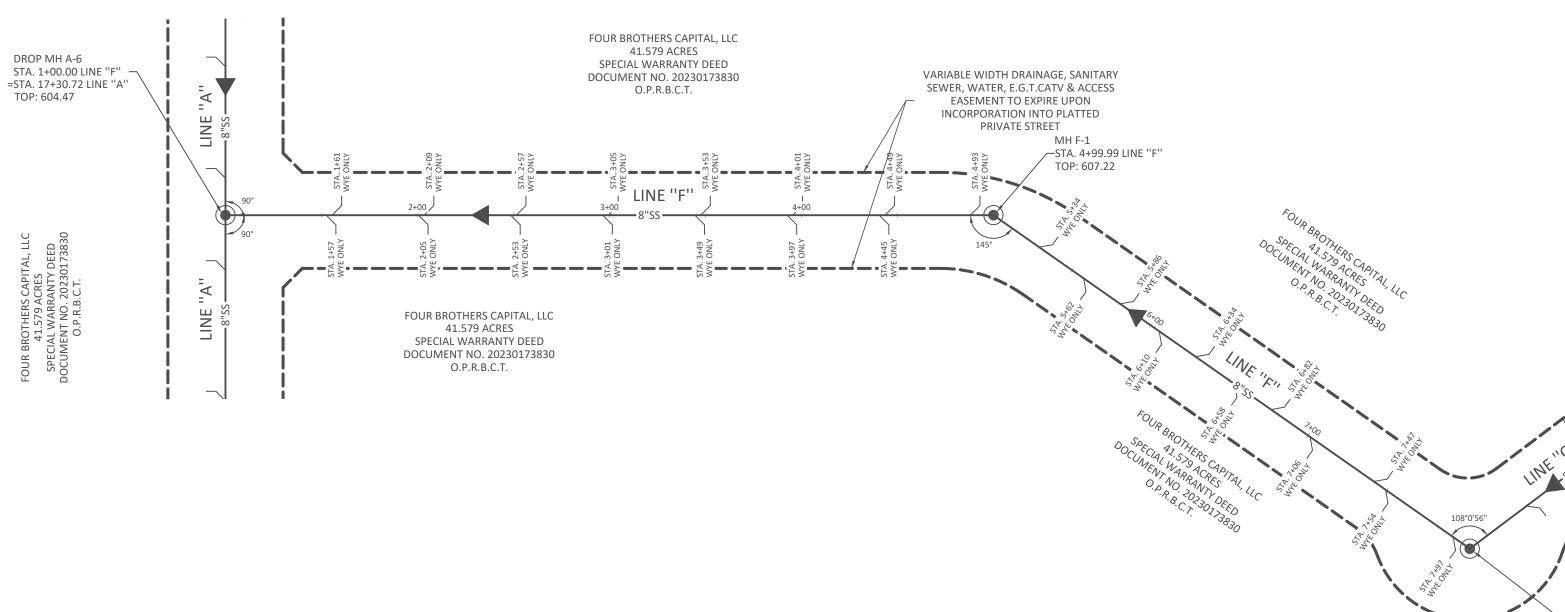
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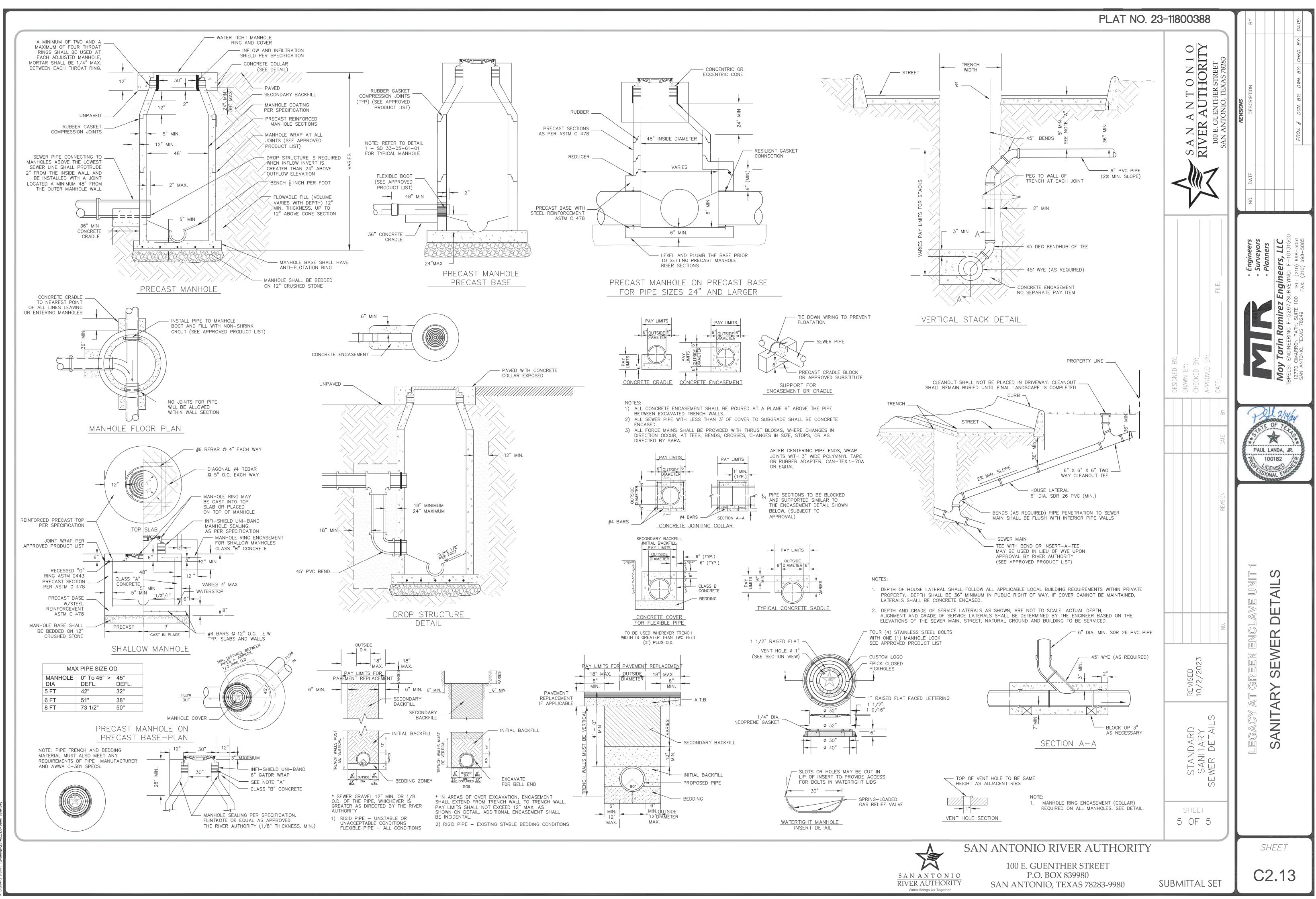
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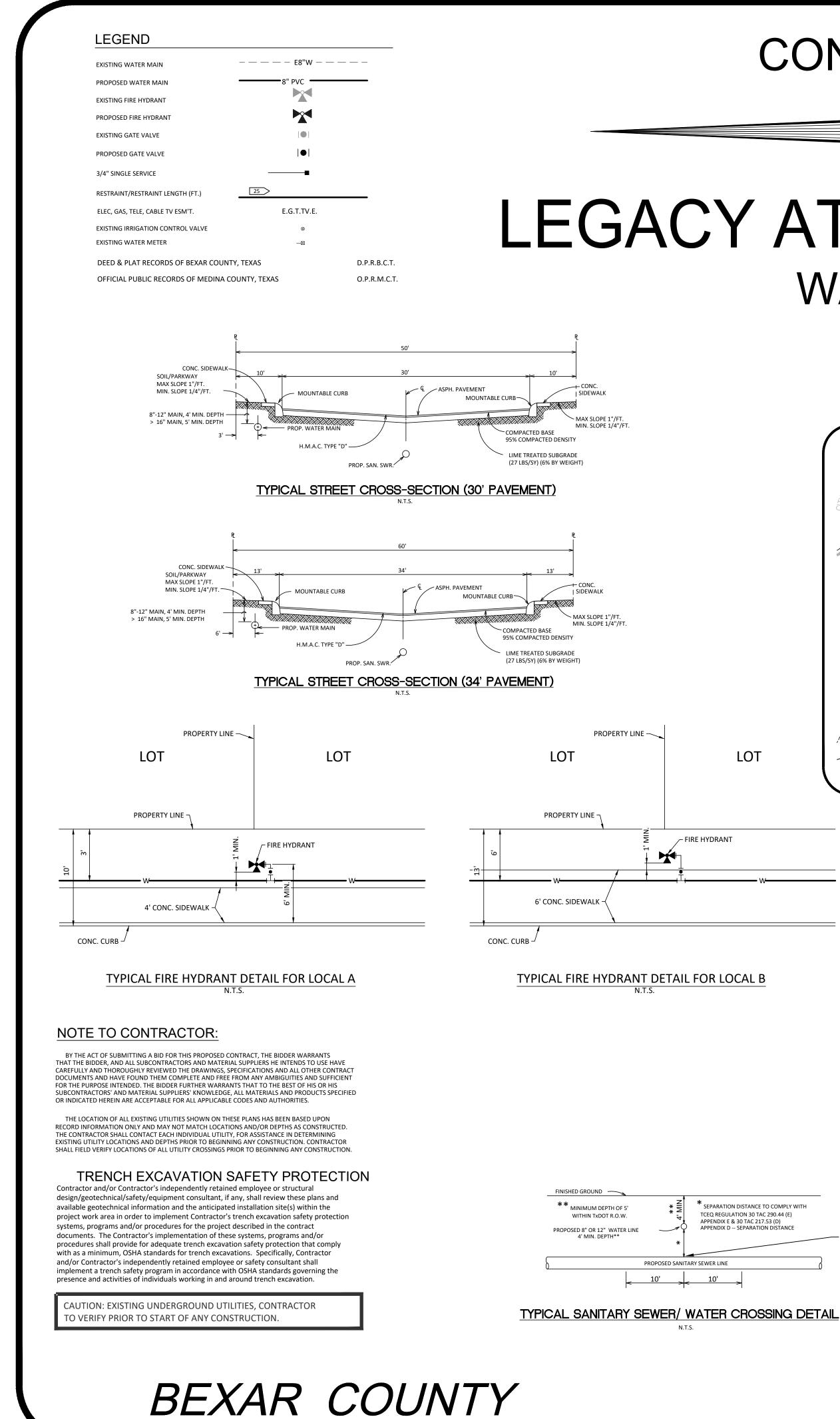
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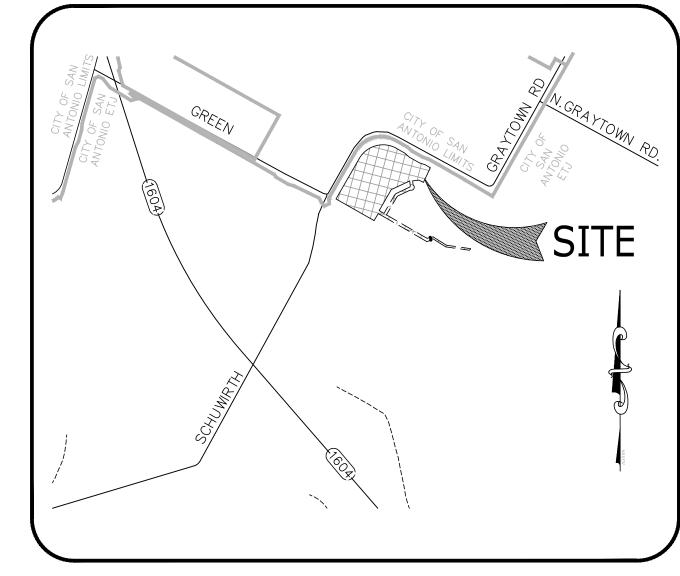
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SANITARY INVERTINARY         South of the state of	500 SUBMITTAL SET C2.12







# **CONSTRUCTION PLANS FOR** SUBMITTED BY: MOY TARIN RAMIREZ ENGINEERS, LLC 12770 CIMARRON PATH, SUITE 100 SAN ANTONIO, TEXAS 78249 TEL: (210) 698-5051 FAX: (210) 698-5085 OWNER/DEVELOPER LEGACY AT GREEN ENCLAVE, UNIT 1 FOUR BROTHERS CAPITAL, LLC 85 N.E. LOOP 410, SUITE 203 SAN ANTONIO, TX 78216 WATER IMPROVEMENTS SHEET C3 SHEET C3.2 SITE INDEX MAP NOT TO SCALE



VICINITY MAP N.T.S.

# SUBMITTAL DATE:

# LEGAL DESCRIPTION:

BEING A TOTAL OF 26.726 ACRE TRACT OF LAND PARTIALLY SITUATED IN THE ANDREW JF PHELAN SURVEY NO. 45, ABSTRACT NO. 580, COUNTY BLOCK 5107, AND PARTIALLY IN THE PI CO SURVEY NO. 4, ABSTRACT NO. 909, COUNTY BLOCK 5107, BOTH OF BEXAR COUNTY, TEXAS, BEING A PORTION OF A 125.588 ACRE TRACT AS CONVEYED TO HELEN RAKOWITZ BY WARRANTY DEED WITH VENDOR'S LIEN AS RECORDED IN VOLUME 1741, PAGE 299, OF THE OFFICIAL PUBLIC RECORDS OF BEXAR COUNTY, TEXAS.

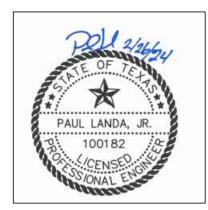
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# SHEET INDEX

Sheet No.	Sheet Title
C3.0	WATER COVER
C3.1	WATER OVERALL
C3.2	WATER OVERALL
C3.3	C3.3 - WATER DETAILS

PLAT NO. 23-11800388

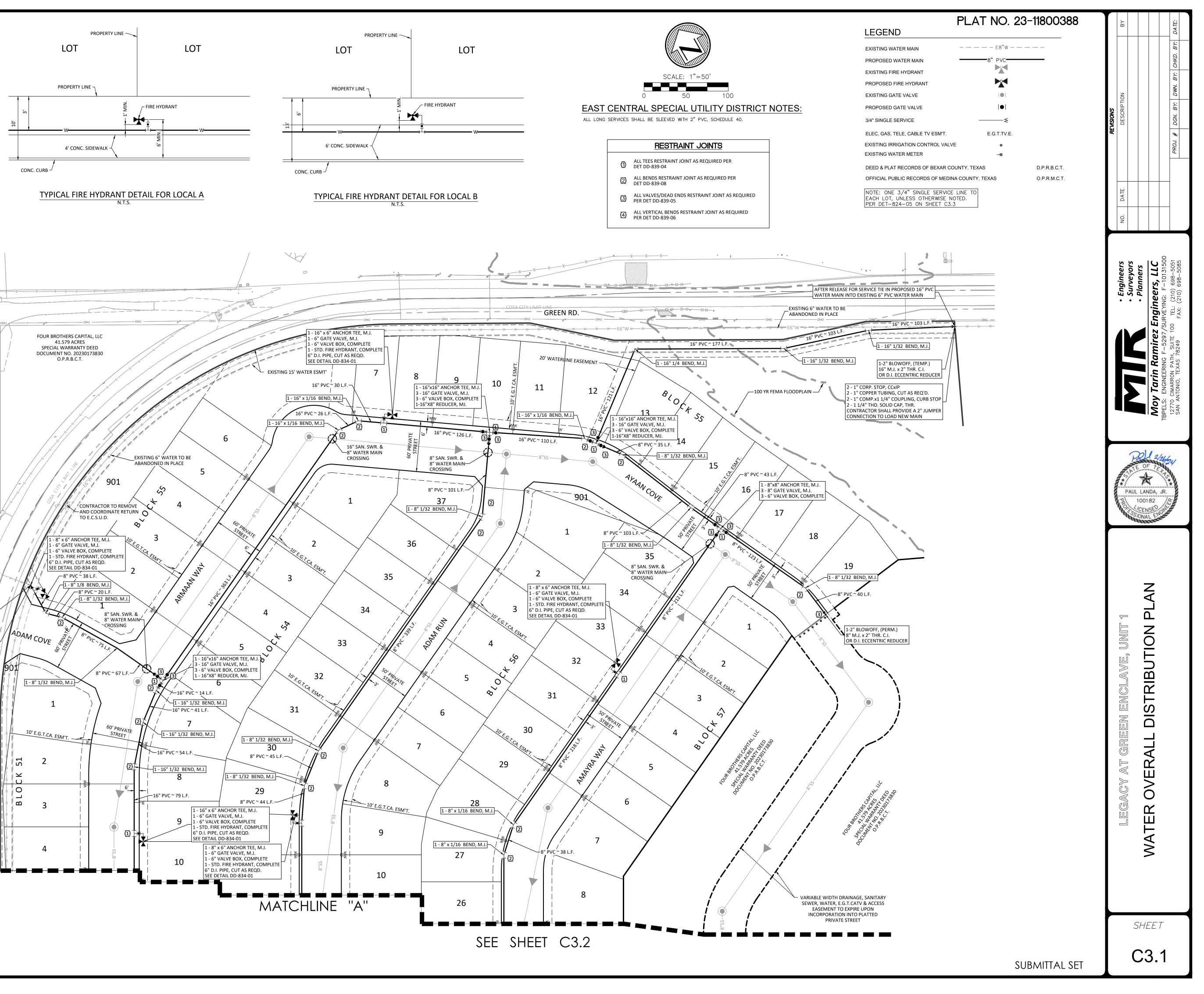


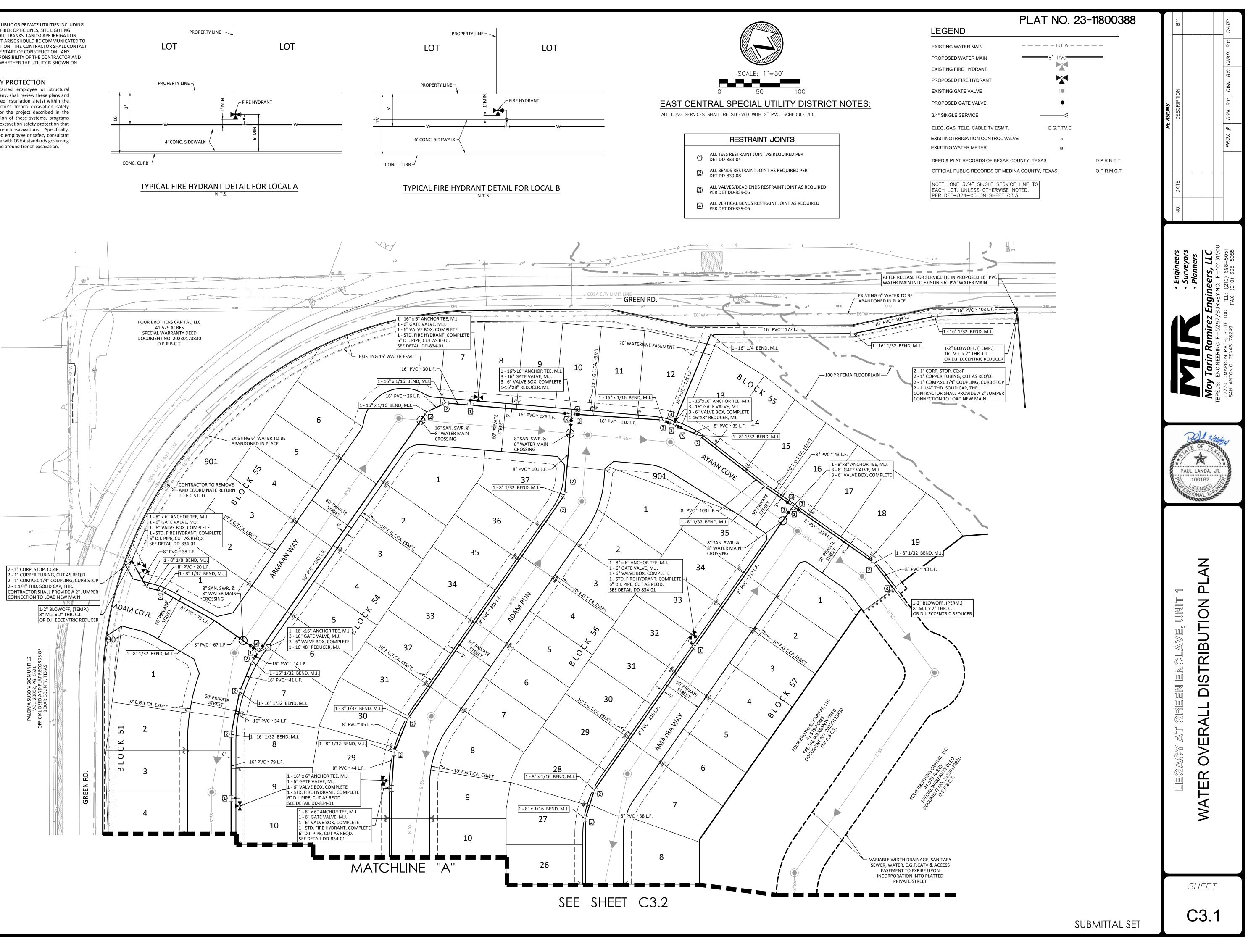


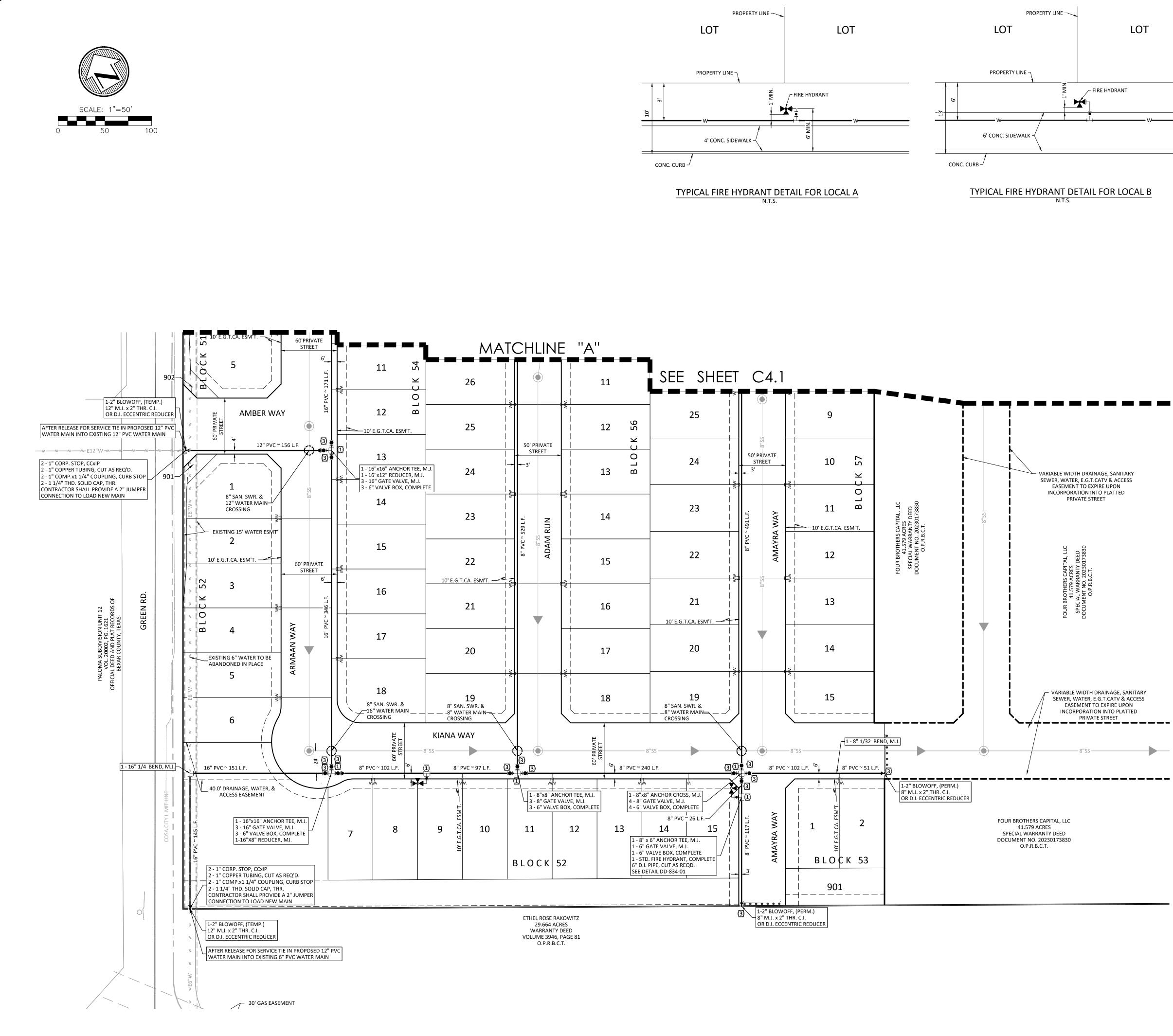
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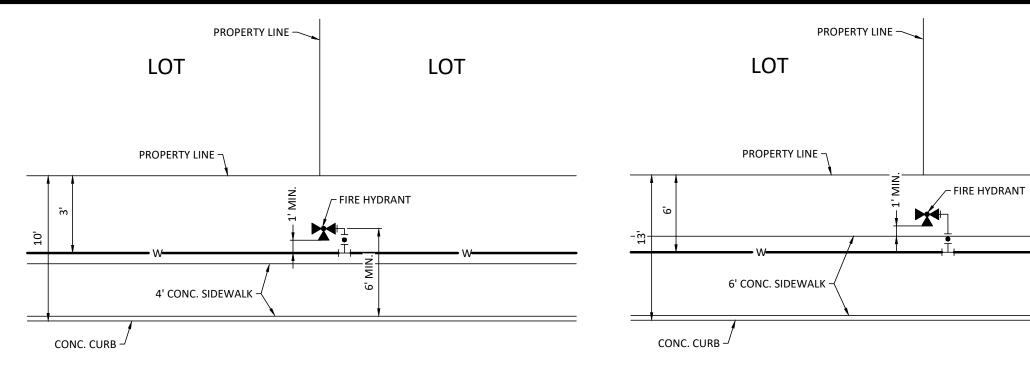
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### LOT

LEGEND

## PLAT NO. 23-11800388

O.P.R.M.C.T.

EXISTING WATER MAIN	— — — — E8"W — — ·	
PROPOSED WATER MAIN		
EXISTING FIRE HYDRANT		
PROPOSED FIRE HYDRANT		
EXISTING GATE VALVE		
PROPOSED GATE VALVE		
3/4" SINGLE SERVICE	≲	
ELEC, GAS, TELE, CABLE TV ESM'T.	E.G.T.TV.E.	
EXISTING IRRIGATION CONTROL VALVE	8	
EXISTING WATER METER	— <b>⊠</b>	
DEED & PLAT RECORDS OF BEXAR COUNTY,	TEXAS	D.P.R.B.C.T.

OFFICIAL PUBLIC RECORDS OF MEDINA COUNTY, TEXAS

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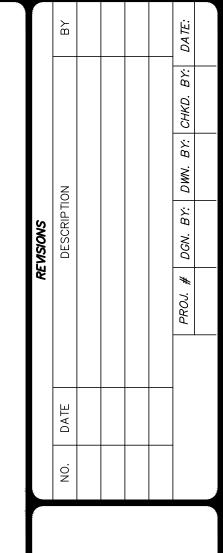
### TRENCH EXCAVATION SAFETY PROTECTION Contractor and/or Contractor's independently retained employee or structural design/geotechnical/safety/equipment consultant, if any, shall review these plans and available geotechnical information and the anticipated installation site(s) within the

project work area in order to implement Contractor's trench excavation safety protection systems, programs and/or procedures for the project described in the contract documents. The Contractor's implementation of these systems, programs and/or procedures shall provide for adequate trench excavation safety protection that comply with as a minimum, OSHA standards for trench excavations. Specifically, Contractor and/or Contractor's independently retained employee or safety consultant shall implement a trench safety program in accordance with OSHA standards governing the presence and activities of individuals working in and around trench excavation.

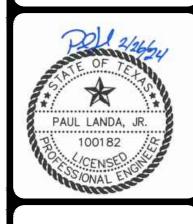
### EAST CENTRAL SPECIAL UTILITY DISTRICT NOTES: ALL LONG SERVICES SHALL BE SLEEVED WITH 2" PVC, SCHEDULE 40.

RESTR	AINT	JOIN	ΓS

- ALL TEES RESTRAINT JOINT AS REQUIRED PER
   DET DD-839-04
- ALL BENDS RESTRAINT JOINT AS REQUIRED PER DET DD-839-08
- ALL VALVES/DEAD ENDS RESTRAINT JOINT AS REQUIRED PER DET DD-839-05
- ALL VERTICAL BENDS RESTRAINT JOINT AS REQUIRED PER DET DD-839-06
- NOTE: ONE 3/4" SINGLE SERVICE LINE TO EACH LOT, UNLESS OTHERWISE NOTED. PER DET-824-05 ON SHEET C3.3







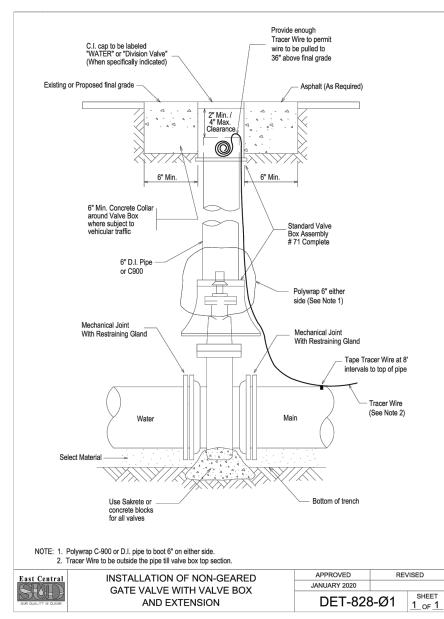
AN Ω UNIT **TRIBUTION**  $\triangleleft$  $\mathbb{Z}$ . С [LL] Z \_ G 4 Ŕ  $\mathbb{A}^{\mathbb{T}}$ Ш >0 G WATER 

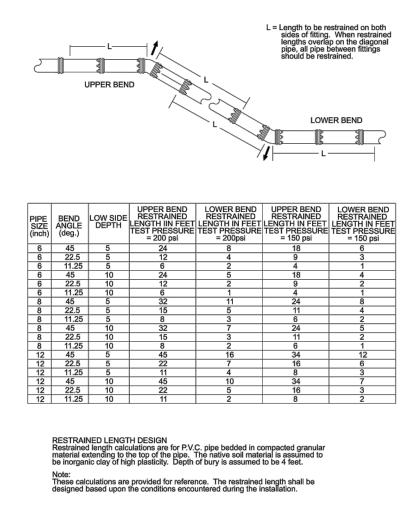
SHEET

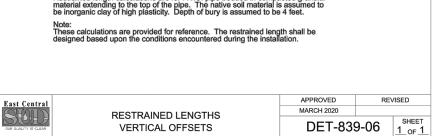
C3.2

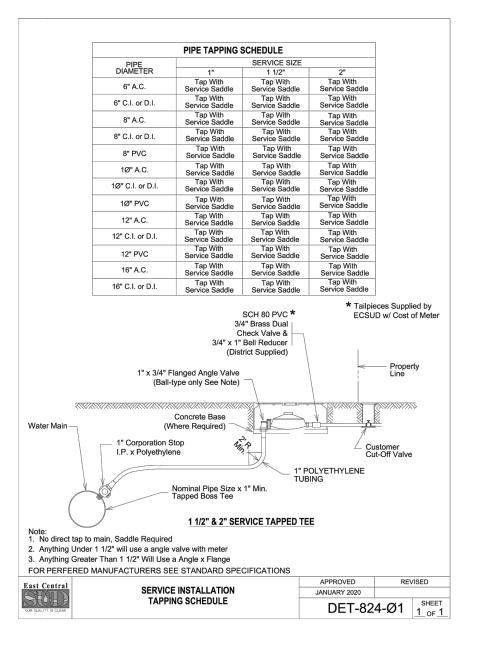
SUBMITTAL SET

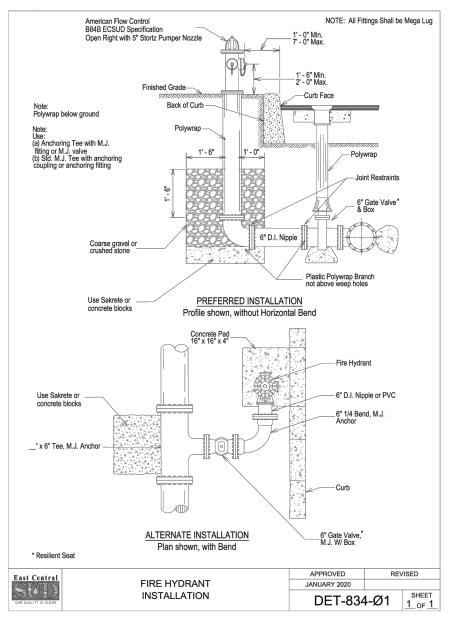


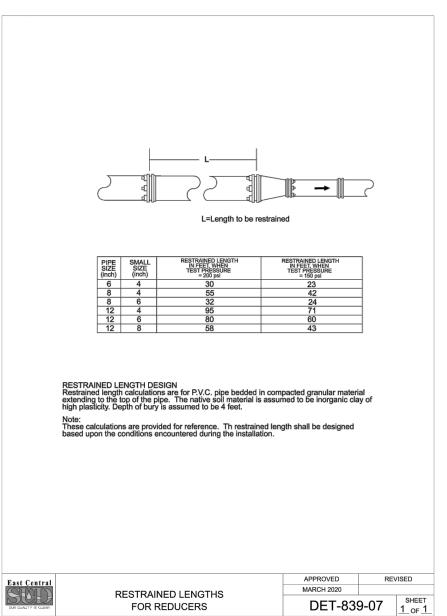


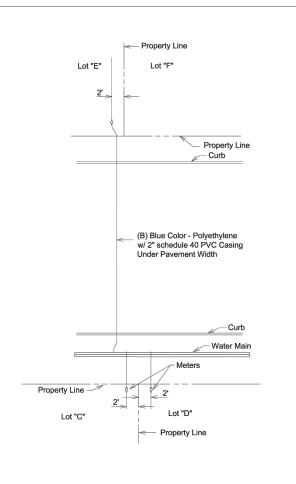




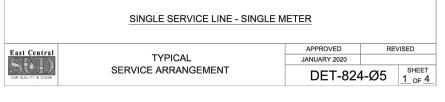


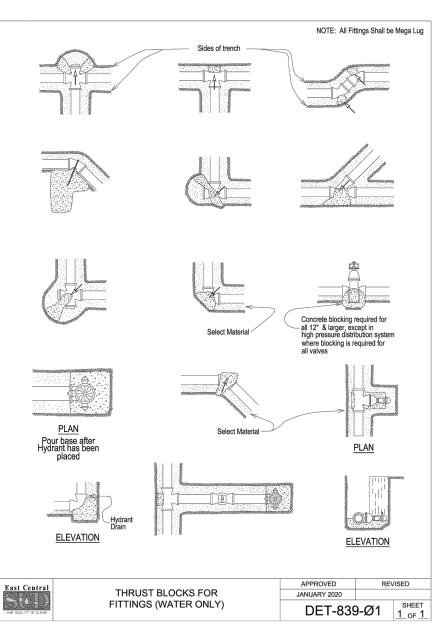


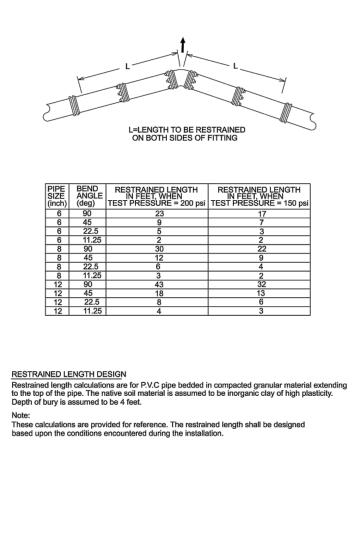




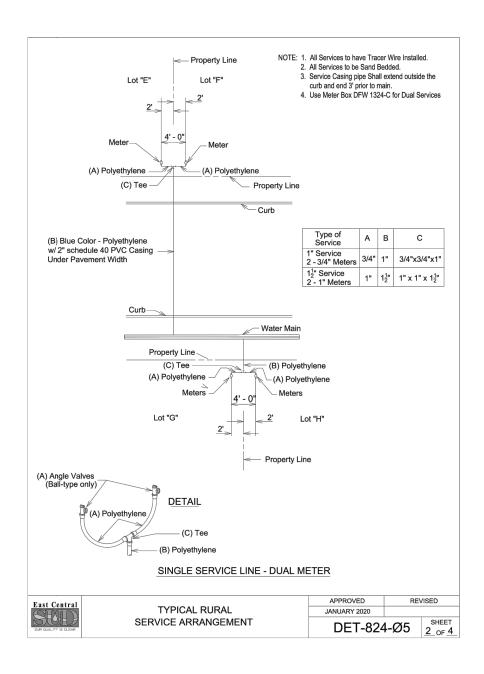
NOTE: 1. All Services to have Tracer Wire Installed. All Services to be Sand Bedded.
 Service Casing pipe Shall extend outside the curb and end 3' prior to main.

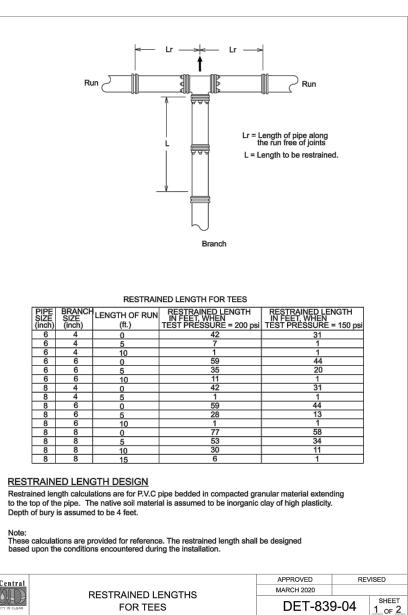


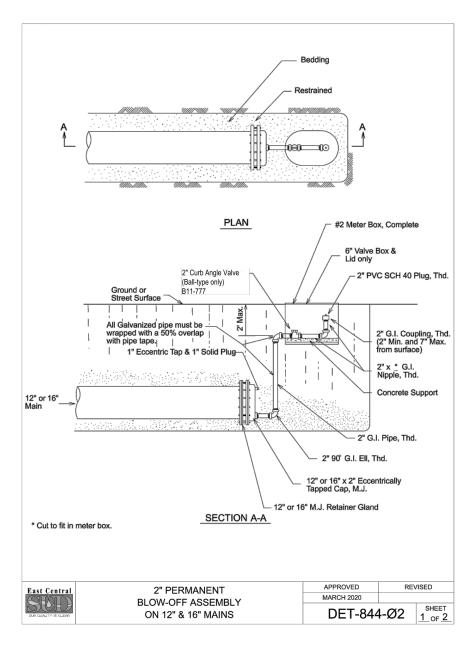


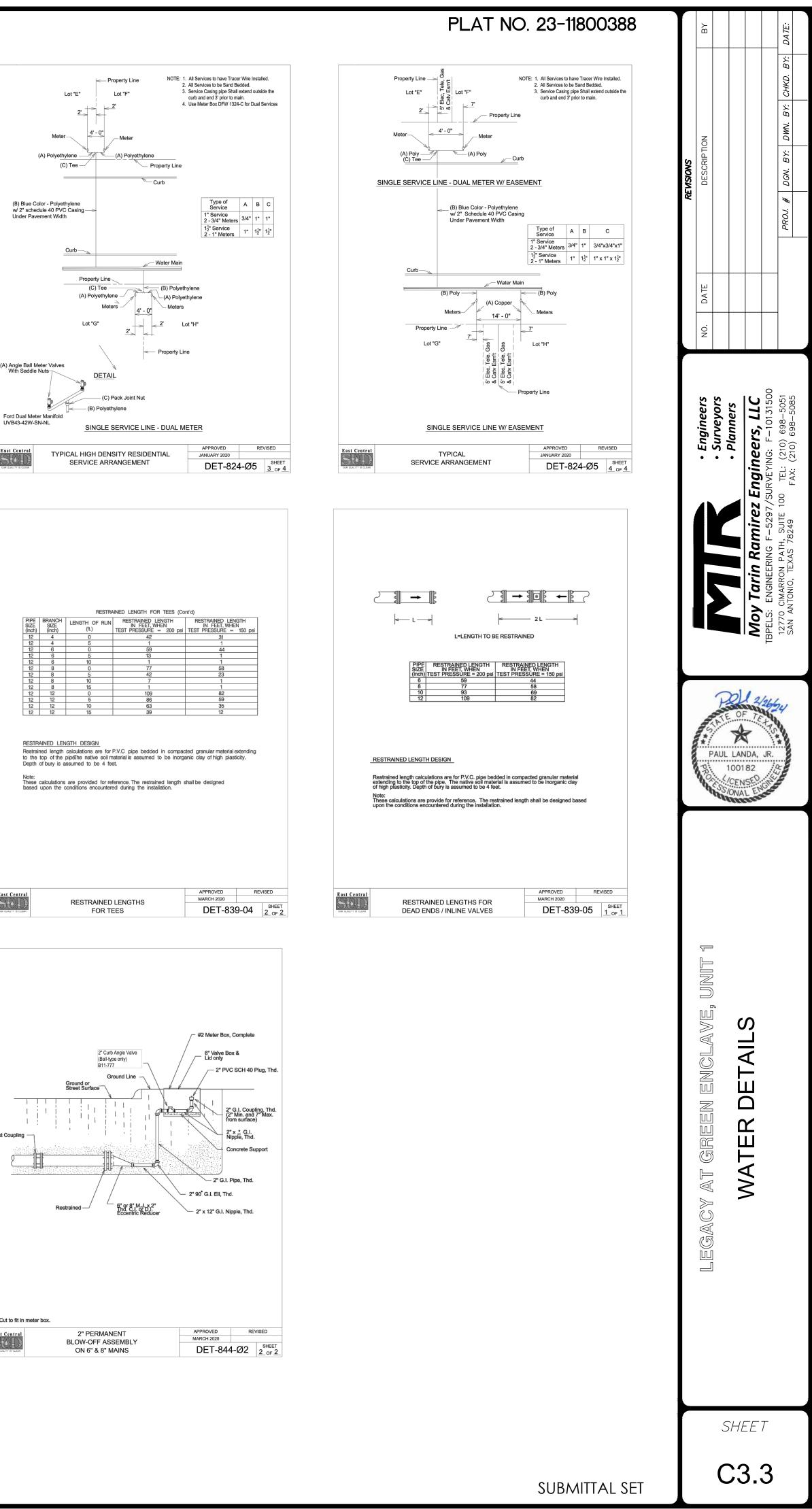


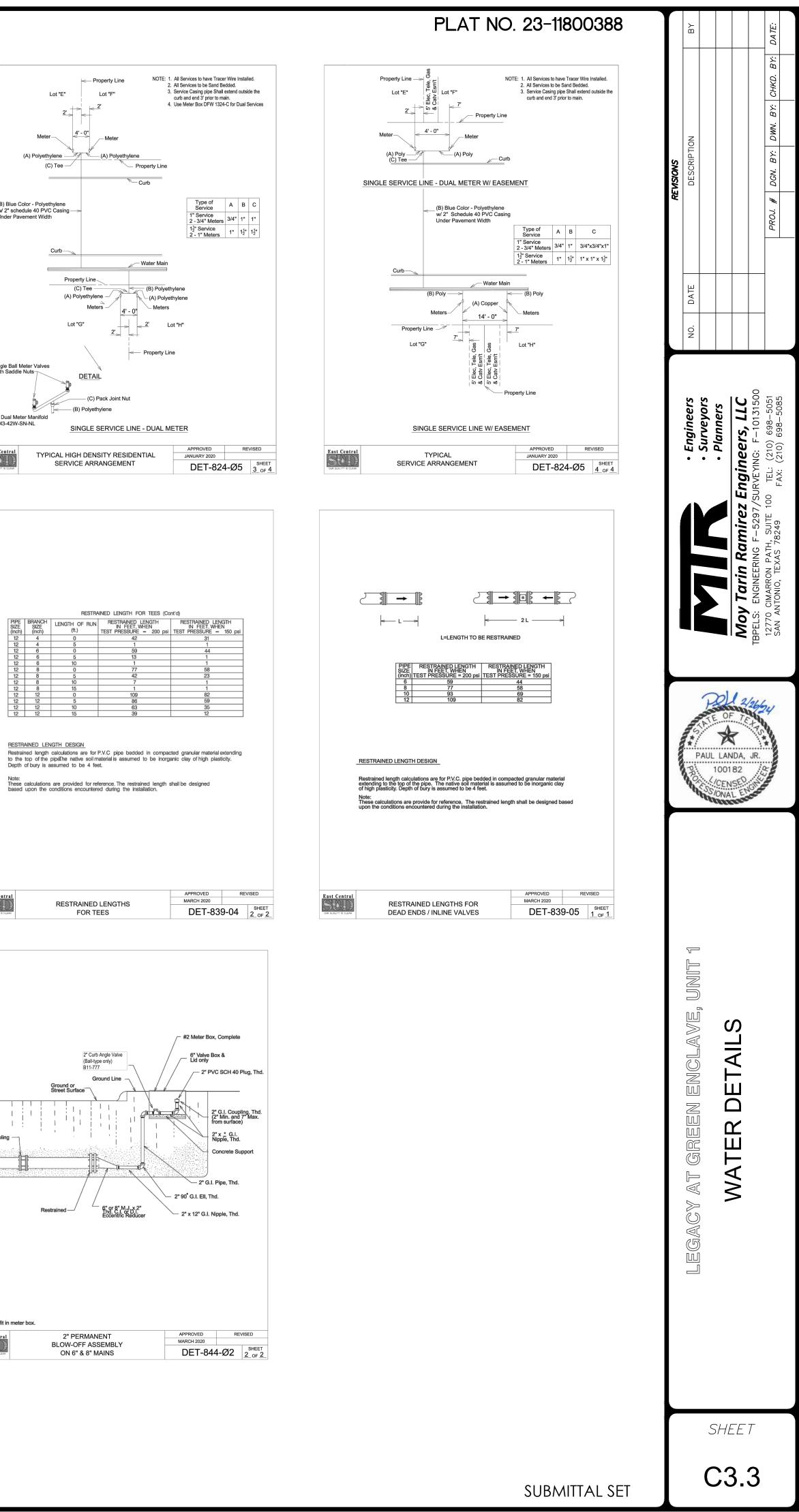




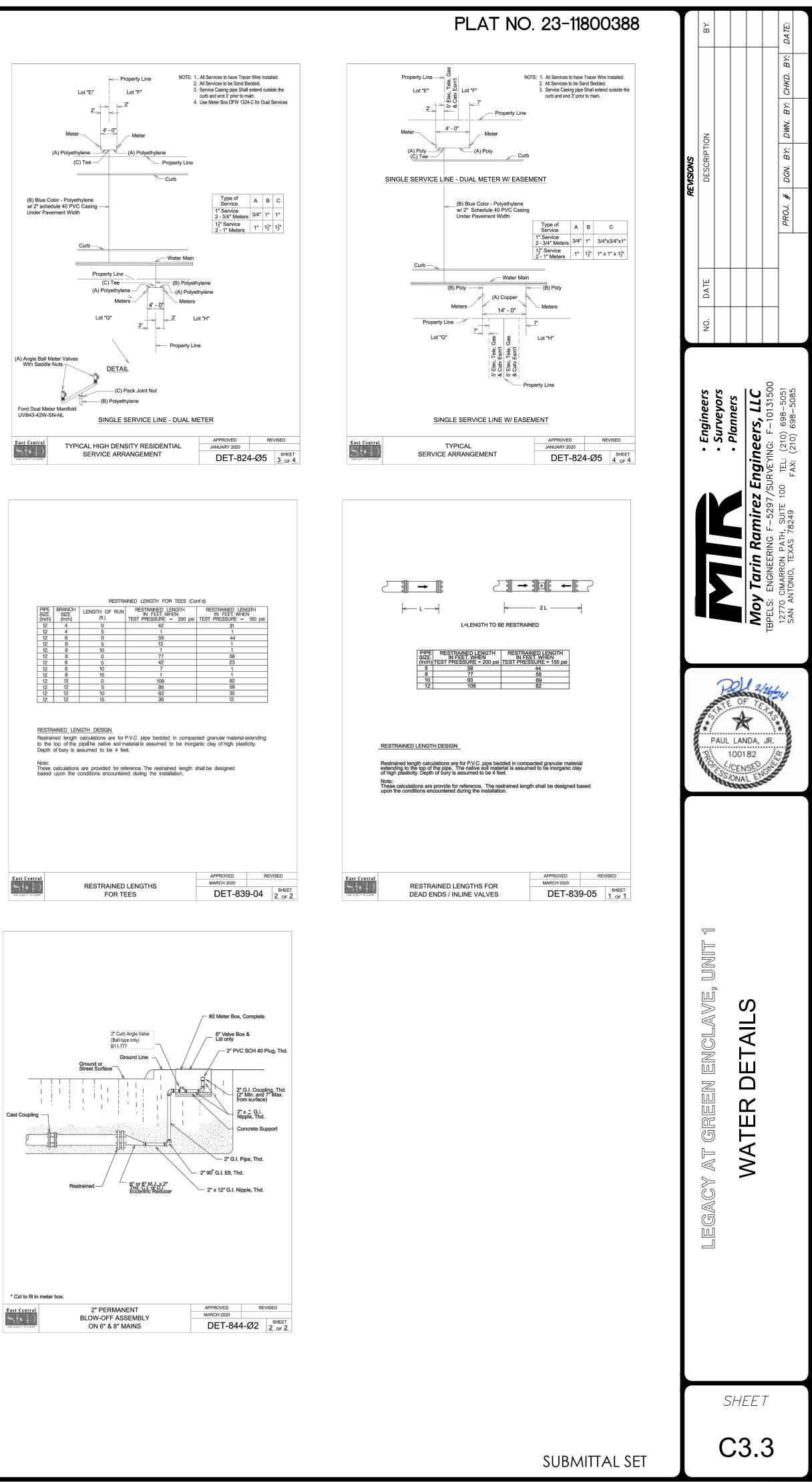








East Central	RESTRAINED LENG FOR TEES
L	



### NOTES:

- 1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE TO THE CITY OF SAN ANTONIO SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION.
- 2. ALL CONSTRUCTION IS SUBJECT TO INSPECTION AND APPROVAL BY THE CITY OF SAN ANTONIO.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTING EXISTING UTILITIES DURING CONSTRUCTION. THE LOCATION AND DEPTH OF EXISTING UTILITIES SHOWN ON THE PLANS ARE APPROXIMATE ONLY. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING THE LOCATION AND DEPTH OF EXISTING UTILITIES PRIOR TO BEGINNING CONSTRUCTION: SAN ANTONIO WATER SYSTEM
  - TELE. NO.: 210-704-7109 TEXAS STATE WIDE ONE CALL LOCATOR TELE. NO.: 800-545-6005 CITY PUBLIC SERVICE TIME WARNER CABLE
- 4. DUE TO FEDERAL REGULATIONS TITLE 49, PART 192.181, CPS MUST MAINTAIN ACCESS TO GAS VALVES AT ALL TIMES. THE CONTRACTOR MUST PROTECT AND WORK AROUND ANY GAS VALVES THAT ARE IN THE PROJECT AREA.
- 5. THE CONTRACTOR HAS THE RESPONSIBILITY TO PROTECT AND SUPPORT THE TELEPHONE COMPANY DURING CONSTRUCTION.
- 6. THE CONTRACTOR HAS THE RESPONSIBILITY OF RESTORING TO ITS ORIGINAL OR BETTER CONDITION, ANY DAMAGE DONE TO THE EXISTING PAVEMENT, STRUCTURES OR FENCES (NO SEPARATE PAY ITEM).
- 7. MATERIAL SPECIFICATIONS:

AT&T

- CONCRETE/CONCRETE RIPRAP: CLASS A 3000 PSI IN 28 DAYS UNLESS OTHERWISE NOTED ON PLANS
- REINFORCING STEEL: CONFORM TO A.S.T.M. A-615, GRADE 60 (2" COVER UNLESS OTHERWISE NOTED ON PLANS) PIPE RAILING: CONFORM TO A.S.T.M. A-53, GRADE B, OR A-501
- STRUCTURAL STEEL: CONFORM TO A.S.T.M. A-36
- 8. CONTRACTOR TO COORDINATE CONCRETE CURB DEPRESSIONS WITH THE DEVELOPER (NO SEPARATE PAY ITEM).
- 9. TRANSITION TO/FROM WASHOUT CROWNS IN TWENTY-FIVE FEET (25').
- 10. IMPROVED EARTHEN CHANNELS WILL BE VEGETATED BY SEEDING OR SODDING. EIGHTY-FIVE PERCENT OF THE CHANNEL SUBGRADE AREA MUST HAVE ESTABLISHED VEGETATION BEFORE THE CHANNEL IS ACCEPTED FOR MAINTENANCE. REFER TO APPENDIX H, CHAPTER 16, SECTION C2.1 - GRASSES OF THE CITY OF SAN ANTONIO. NO EXTRA PAY ITEM.

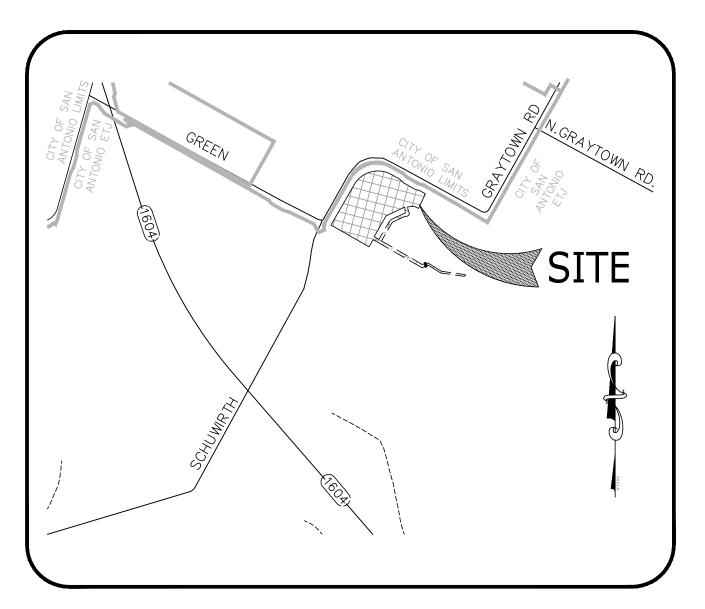
### LEGEND CONTRACTOR TO TIE EXISTING AND PROPOSED 1 CURB/SIDEWALK. PRIOR TO CONSTRUCTION CONTRACTOR SHALL VERIFY ELEVATIONS. SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (SINGLE) SIDEWALK WHEELCHAIR RAMP - TYPE 10 B DIRECTIONAL RAMPS (DUAL) SIDEWALK WHEELCHAIR RAMP - TYPE II (c)(DEVELOPER INSTALLED) SIDEWALK WHEELCHAIR RAMP - TYPE I (DEVELOPER INSTALLED) SIDEWALK WHEELCHAIR RAMP - TYPE 11 (E) (DEVELOPER INSTALLED) OFFSET PARALLEL RAMP F SIDEWALK PASSING SPACE EXISTING TOP OF CURB ELEVATION 805.81TC PROPOSED TOP OF CURB ELEVATION 805.81 HOME BUILDER INSTALLED SIDEWALK DEVELOPER INSTALLED SIDEWALK SIDEWALK WHEEL CHAIR RAMP (DEVELOPER INSTALLED) WASH-OUT CROWN POSSIBLE DRIVEWAY LOCATION PROPERTY LINE EXISTING CONTOUR - — — — 1120 — — — — \_\_\_\_\_\_1120 PROPOSED CONTOUR PROPOSED CONCRETE CURB FLOW ARROW

COSA CITY LIMIT LINE

# LEGACY AT GREEN ENCLAVE, UNIT 1 STREET AND DRAINAGE IMPROVEMENTS



# **CONSTRUCTION PLANS FOR**



VICINITY MAP N.T.S.

# SUBMITTAL DATE:

# LEGAL DESCRIPTION:

BEING A TOTAL OF 26.726 ACRE TRACT OF LAND PARTIALLY SITUATED IN THE ANDREW JF PHELAN SURVEY NO. 45, ABSTRACT NO. 580, COUNTY BLOCK 5107, AND PARTIALLY IN THE PI CO SURVEY NO. 4, ABSTRACT NO. 909, COUNTY BLOCK 5107, BOTH OF BEXAR COUNTY, TEXAS, BEING A PORTION OF A 125.588 ACRE TRACT AS CONVEYED TO HELEN RAKOWITZ BY WARRANTY DEED WITH VENDOR'S LIEN AS RECORDED IN VOLUME 1741, PAGE 299, OF THE OFFICIAL PUBLIC RECORDS OF BEXAR COUNTY, TEXAS.



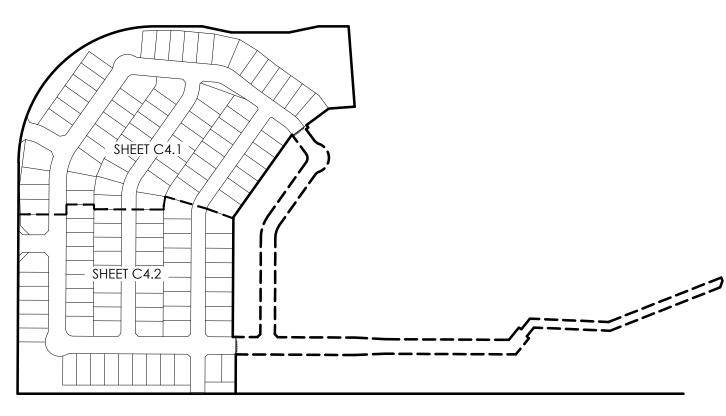
## PLAT NO. 23-11800388

SUBMITTED BY:

MOY TARIN RAMIREZ ENGINEERS, LLC. 12770 CIMARRON PATH, SUITE 100 SAN ANTONIO, TEXAS 78249 TEL: (210) 698-5051 FAX: (210) 698-5085

### OWNER/DEVELOPER

FOUR BROTHERS CAPITAL, LLC 85 N.E. LOOP 410, SUITE 203 SAN ANTONIO, TX 78216



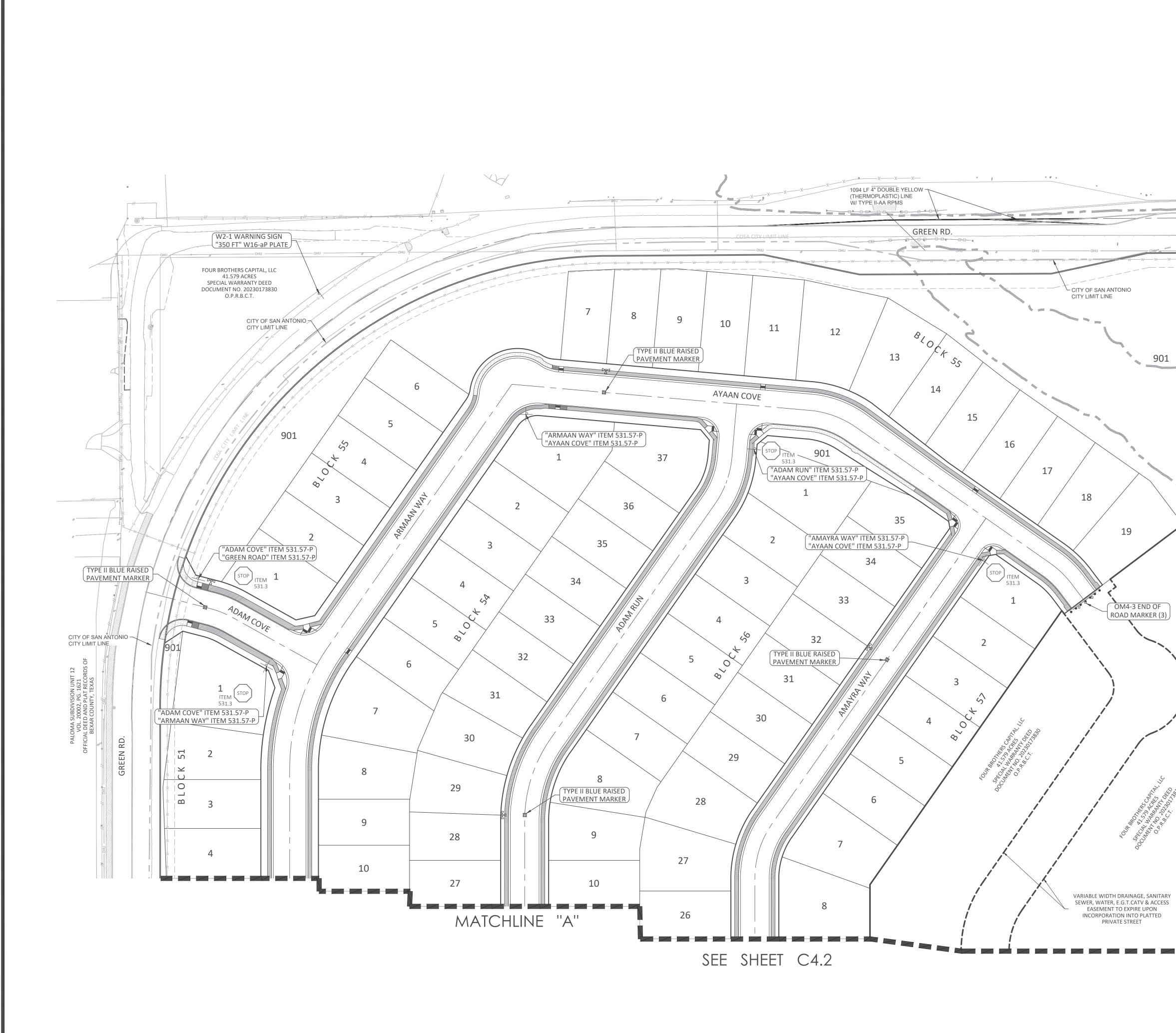
INDEX MAP NOT TO SCALE

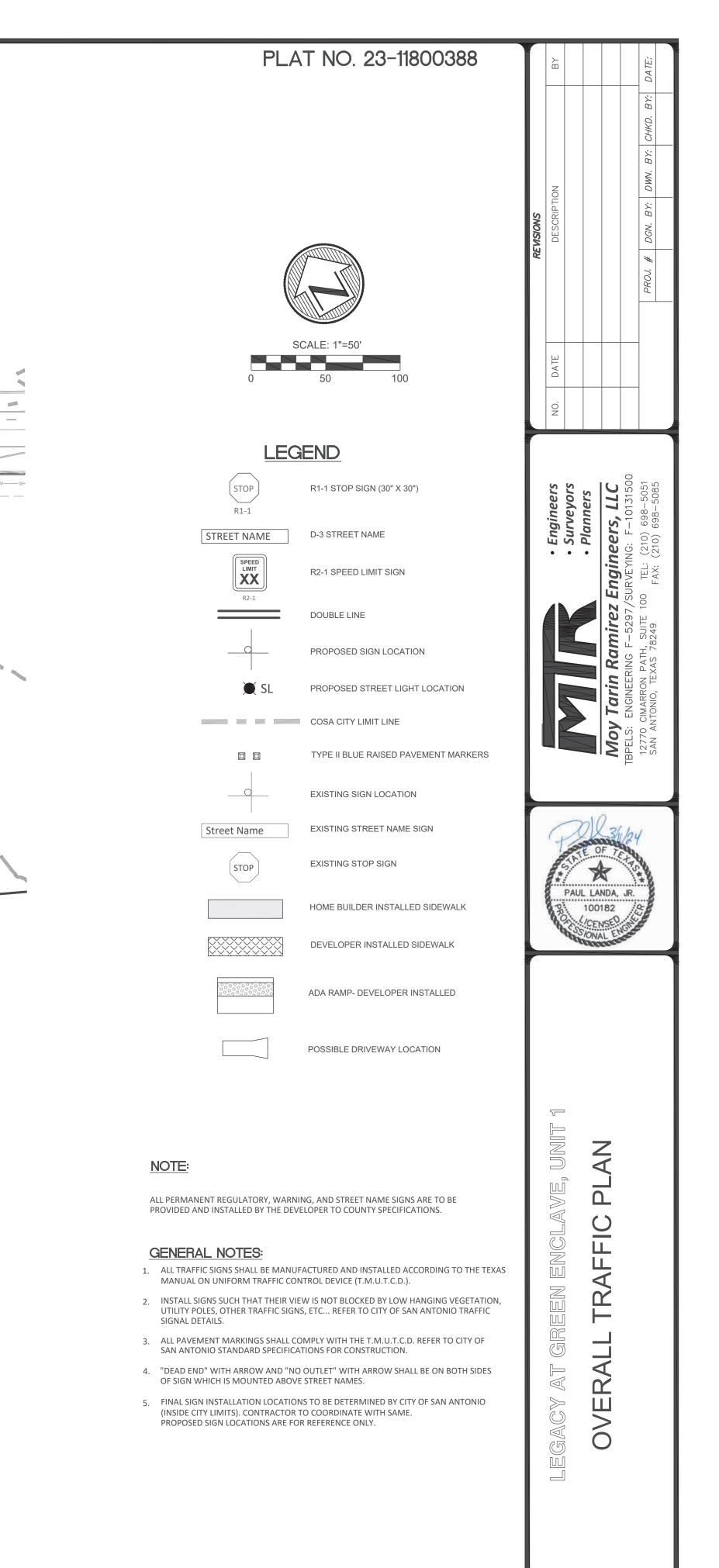
# SHEET INDEX

Sheet No. Sheet Title

Sheet No.	Sheet Title
STREET & DR	RAIN PLANS
C4.0	STREET COVER
C4.1	TRAFFIC PLAN
C4.2	TRAFFIC PLAN
C4.3	TRAFFIC DETAILS
C4.4	TRAFFIC DETAILS
C4.5	ADAM COVE & AMBER WAY (PRIVATE STREETS) PLAN & PROFILE
C4.6	AMAYRA WAY (PRIVATE STREET) PLAN & PROFILE
C4.7	AMAYRA WAY (PRIVATE STREET) PLAN & PROFILE
C4.8	ARMAAN WAY (PRIVATE STREET) PLAN & PROFILE
C4.9	ARMAAN WAY (PRIVATE STREET) PLAN & PROFILE
C4.10	ADAM RUN (PRIVATE STREET) PLAN & PROFILE
C4.11	ADAM RUN (PRIVATE STREET) PLAN & PROFILE
C4.12	KIANA WAY (PRIVATE STREET) PLAN & PROFILE
C4.13	AYAAN COVE (PRIVATE STREET) PLAN & PROFILE
C4.14	GREEN ROAD STREET WIDEN
C4.15	GREEN ROAD STREET WIDEN
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C4.17A	STANDARD DETAILS
C4.17B	STANDARD DETAILS
C4.18	STANDARD DETAILS
C4.19	TYPICAL STREET SECTIONS
C4.20	DRAIN "A"
C4.21	DRAIN "B"
C4.22	DRAINAGE DETAILS
C4.23	DRAINAGE DETAILS
C4.24	DRAINAGE DETAILS
C4.25	DRAINAGE DETAILS
GRADING PL	-
C5.0	GRADING PLAN
C5.1	GRADING PLAN
SW3P PLANS	
C6.0	SW3P PLAN
C6.1	SW3P PLAN
C6.2	SW3P DETAILS

SUBMITTAL SET TEXAS C4.0



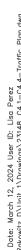


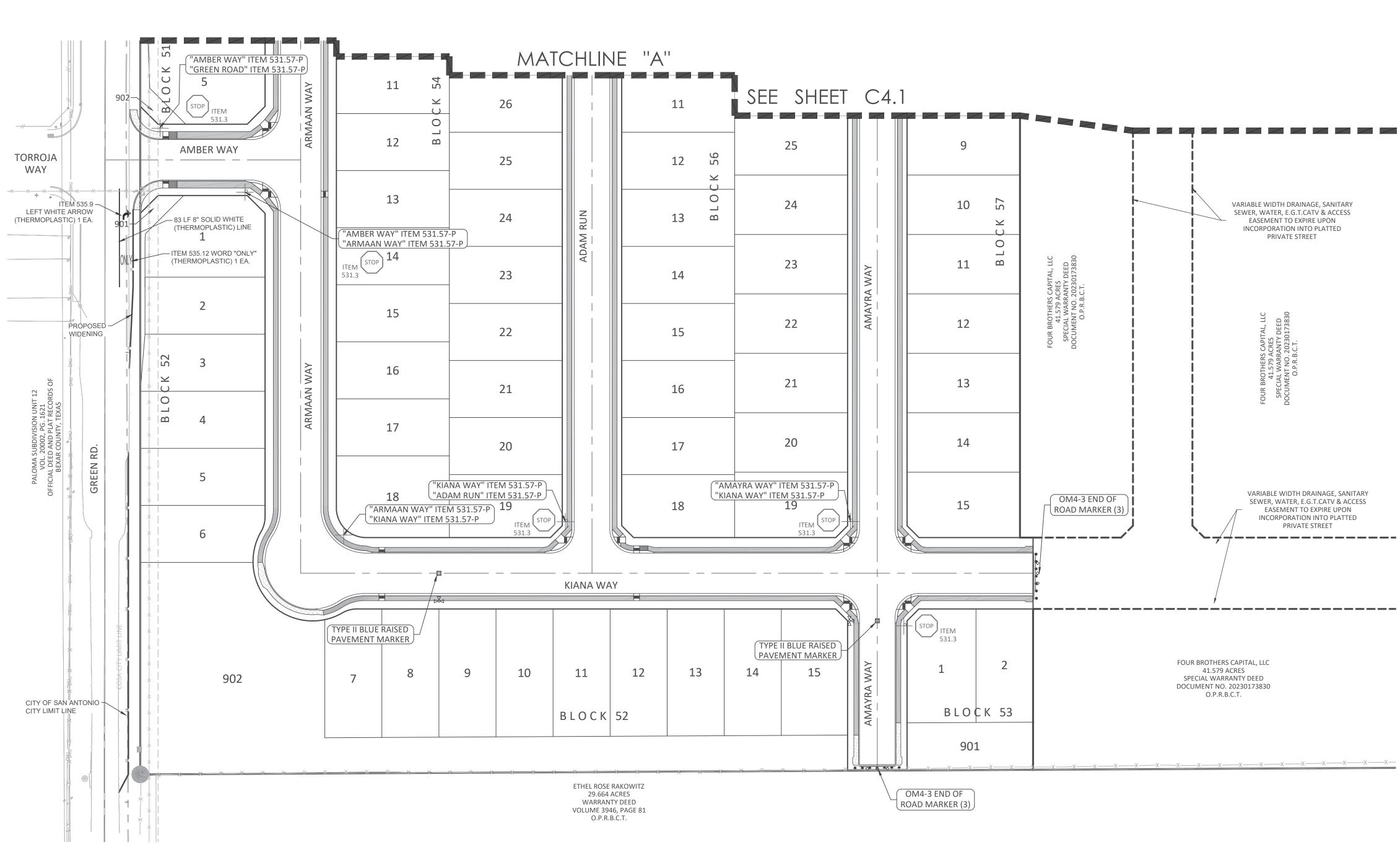
901

SUBMITTAL SET

C4.1

SHEET





PLAT	NO. 23-11800388		B			DA TE:
		REVISIONS	DESCRIPTION		- - - -	PROJ. # DGN. BY: DWN. BY: CHKD. BY:
	E: 1"=50' 50 100		NO. DATE			
LEG	END					
STOP	R1-1 STOP SIGN (30" X 30")		ν. Υ		200	51 35
R1-1 STREET NAME	D-3 STREET NAME		Engineers Surveyors	S. LL(	·	698–5051 698–5085
SPEED LIMIT XX	R2-1 SPEED LIMIT SIGN		• Eng	Enaineers	SURVEYING: F	TEL: (210) FAX: (210)
R2-1	DOUBLE LINE				~ 1	100
	PROPOSED SIGN LOCATION			arin Ramirez	F-5297	4, SUITE 78249
SL	PROPOSED STREET LIGHT LOCATION			in Ra	ENGINEERING	DN PATH TEXAS
	COSA CITY LIMIT LINE				ENGINE	70 CIMARRON 1 ANTONIO, TEX
	TYPE II BLUE RAISED PAVEMENT MARKERS			Nov	IBPELS:	12770 SAN AN
	EXISTING SIGN LOCATION				Ε	
Street Name	EXISTING STREET NAME SIGN		$\cap$	210-2		
STOP	EXISTING STOP SIGN		Tring and	OF T	AT AN	b.
	HOME BUILDER INSTALLED SIDEWALK		PAUL	LANDA, 100182	JR.	
	DEVELOPER INSTALLED SIDEWALK		Contraction of the second	CENSE ONAL E	NGING	ę
00000000000000000000000000000000000000	ADA RAMP- DEVELOPER INSTALLED					
	POSSIBLE DRIVEWAY LOCATION					
MANUAL ON UNIFORM TRAFFIC CONTROL 2. INSTALL SIGNS SUCH THAT THEIR VIEW IS I	R TO COUNTY SPECIFICATIONS. RED AND INSTALLED ACCORDING TO THE TEXAS DEVICE (T.M.U.T.C.D.). NOT BLOCKED BY LOW HANGING VEGETATION, C REFER TO CITY OF SAN ANTONIO TRAFFIC		GREEN ENCLAVE, UNIT 1	LL TRAFFIC PLAN		

- INSTALL SIGNS SUCH THAT THEIR VIEW IS NOT BLOCKED BY LOW HANGING VEGETATION UTILITY POLES, OTHER TRAFFIC SIGNS, ETC... REFER TO CITY OF SAN ANTONIO TRAFFIC SIGNAL DETAILS.
- 3. ALL PAVEMENT MARKINGS SHALL COMPLY WITH THE T.M.U.T.C.D. REFER TO CITY OF SAN ANTONIO STANDARD SPECIFICATIONS FOR CONSTRUCTION.
- 4. "DEAD END" WITH ARROW AND "NO OUTLET" WITH ARROW SHALL BE ON BOTH SIDES OF SIGN WHICH IS MOUNTED ABOVE STREET NAMES.

PRIVATE STREET

5. FINAL SIGN INSTALLATION LOCATIONS TO BE DETERMINED BY CITY OF SAN ANTONIO (INSIDE CITY LIMITS). CONTRACTOR TO COORDINATE WITH SAME. PROPOSED SIGN LOCATIONS ARE FOR REFERENCE ONLY.

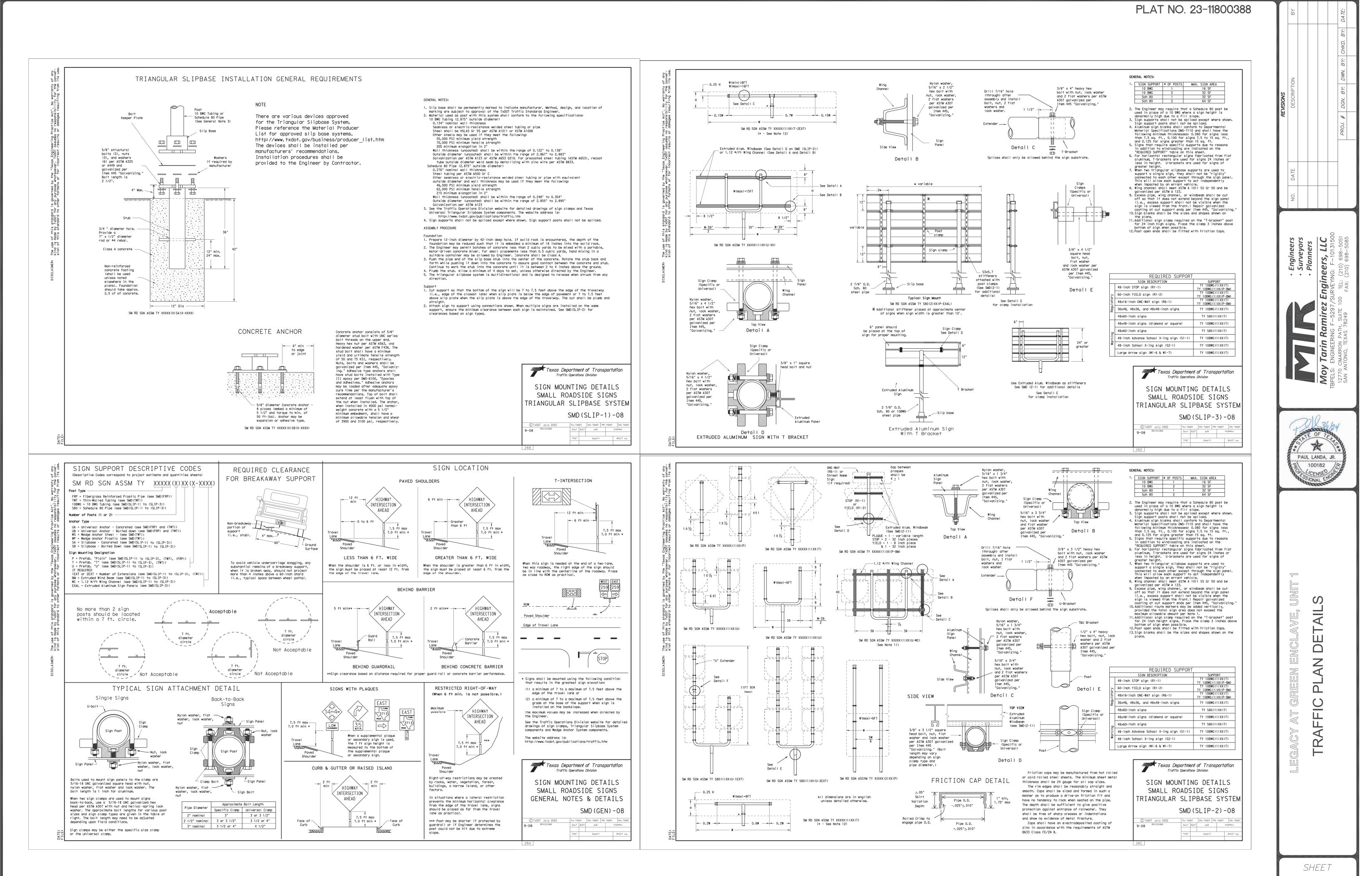
SHEET

OVERALL

AGY

B 

C4.2 SUBMITTAL SET

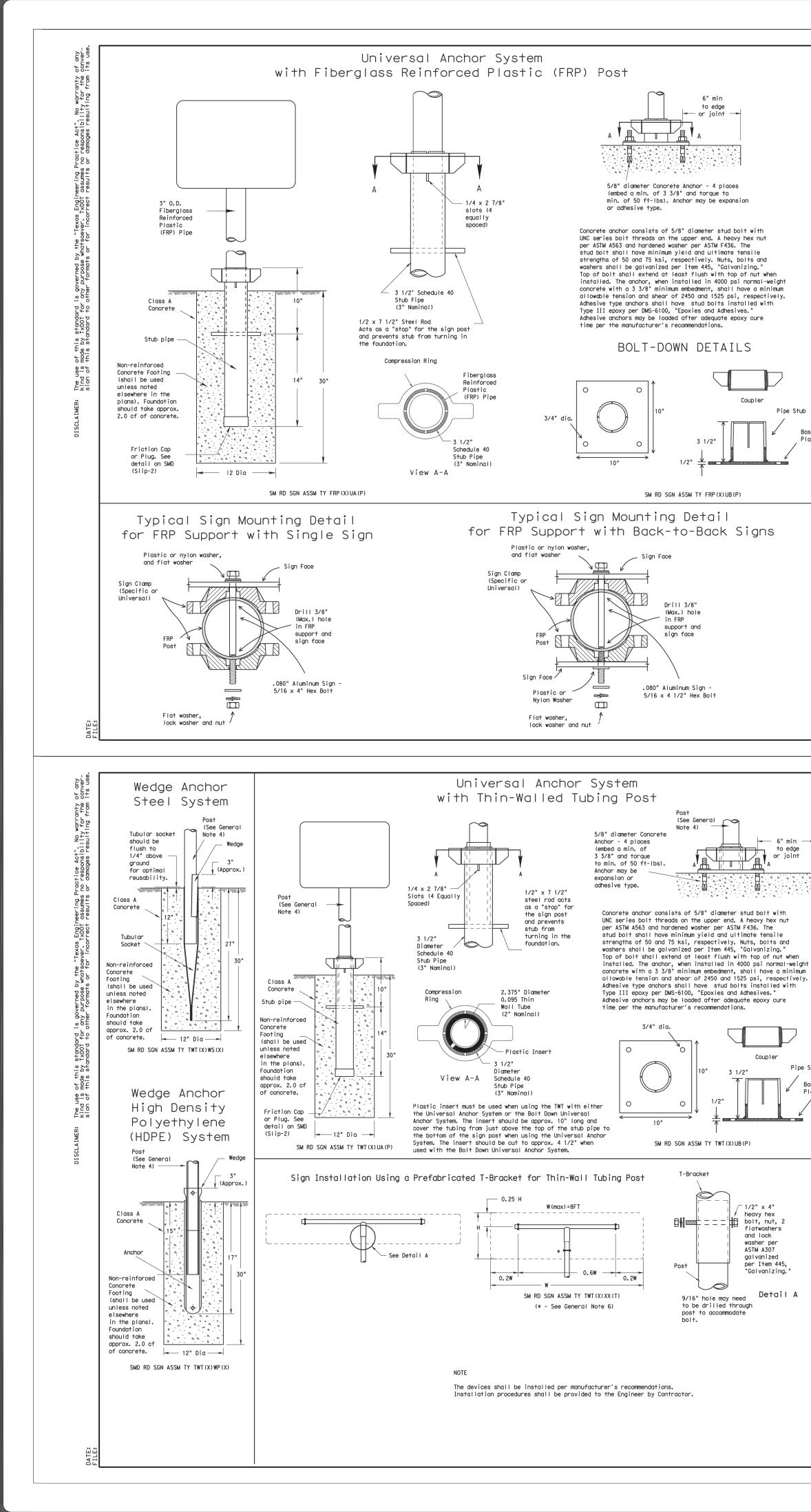


rrch 12, 2024 User ID: Lisa Perez V/Unit 1/Drawings\23148\_C4.1-C4.4-Traffic Plan.dwg

Date: March 12, 2024 User

SUBMITTAL SET

C4.3



### GENERAL NOTES: 1. FRP sign supports for a single type sign support may be used for signs up to and including 16 square feet. Dual post installation may be used for signs up to and including 32 square feet. 2. All nuts, bolts and washers shall be galvanized per Item 445, "Galvanizing." 3. See the Traffic Operations Division website for detailed drawings of sign clamps. The website address is: http://www.txdot.gov/publications/traffic.htm FRP POST REQUIREMENTS 1. Materials shall conform to the requirements of Departmental Material Specification DMS-4410 and will be furnished in a yellow or gray color as specified elsewhere in the plans. 2. Thickness of FRP sign support is 0.125" + 0.031", - 0.0". 3. FRP sign supports are prequalified by the Traffic Operations Division. Pregualification procedures are obtained by writing: Texas Department of Transportation Traffic Operations Division 125 East 11th Street Austin, Texas 78701-2483 UNIVERSAL ANCHOR SYSTEM INSTALLATION PROCEDURES 1. Dia foundation hole. Where solid rock is encountered at around level. the foundation shall be a minimum depth of 18". When solid rock is encountered below ground level, the foundation shall extend in the solid rock a minimum depth of 18" or provide a minimum foundation depth of 30". If solid rock is encountered, the socket/stub may be reduced in length as required to a minimum length of 18". Any material removed from the socket/stub shall be from the bottom and the clearance requirements given on SMD(GEN) must be followed. The inner surfaces of the socket/stub must remain free of concrete or other debris. 2. The Engineer may permit batches of concrete less than 2 cubic yards to be mixed with a portable, motor driven concrete mixer. For small placements less than 0.5 cubic yards, hand mixing in a suitable container may be allowed by Engineer. Concrete shall be Class A. 3. Insert base post in foundation hole to depths shown and fill hole with concrete. Cut base post from bottom and ensure a minimum of 18" embedment if installed in solid rock. 4. Level and plumb the base post with coupler using a torpedo level and let concrete set a minimum of 4 days, unless otherwise directed by Engineer. Bottom of base post slots shall be above the concrete footing. 5. Attach sign to FRP post. 6. Insert sign post into base post. Lower until the post comes to rest on the steel rod. 7. Use hammer to ensure the coupler is firmly seated. Top of coupler should be Pipe Stub level with top of base post in most instances. 8. Check sign to ensure there is no twist. If loose, increase the tightening of coupler. BOLT DOWN SIGN SUPPORT 1. Position base plate with coupler on existing concrete. 2. Drill holes into concrete and insert the 5/8" diameter bolts with wedge anchors, and tighten nuts. 3. Attach sign to FRP post. 4. Insert bottom of sign post into pipe stub. 5. Use hammer to ensure the coupler is firmly seated. Top of coupler should be level with top of base post in most instances. 6. Check sign to ensure there is no twist. If loose, increase the tightening of coupler. Texas Department of Transportation Traffic Operations Division SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS UNIVERSAL ANCHOR SYSTEM WITH FRP POST C TxDOT July 2002 REVISIONS 26F GENERAL NOTES: The Wedge Anchor System and the Universal Anchor System with thin wall tubing post may be used to support up to 10 square feet of sign area. . The tubular socket, wedge and prefabricated T-bracket shall be permanently marked to indicate manufacturer. Method, design, and location of marking are subject to the approval of the TxDOT Traffic Standards Engineer. Except for posts (13 BWG Tubing), clamps, nuts and bolts, all components shall be prequalified. A list of prequalified vendors may be obtained from the Material Producer List web page. The website address is: http://www.txdot.gov/business/producer list.htm to edge Material used as post with this system shall conform to the following specifications: 13 BWG Tubing (2.375" outside diameter) (TWT) or join 0.095" nominal wall thickness Seamless or electric-resistance welded steel tubing Steel shall be HSLAS Gr 55 per ASTM A1011 or ASTM A1008 Other steels may be used if they meet the following: 55,000 PSI minimum yield strength 70,000 PSI minimum tensile strength 18½ minimum elongation in 2" Wall thickness (uncoated) shall be within the range of .083" to .099" per ASTM B833. not be spliced. http://www.txdot.gov/publications/traffic.htm WEDGE ANCHOR SYSTEM INSTALLATION PROCEDURE

Coupler

- 1/2" x 4"

heavy hex

and lock washer per

ASTM A307

galvanized

per Item 445,

"Galvanizing."

Outside diameter (uncoated) shall be within the range of 2.369" to 2.381" Galvanization per ASTM 123 or ASTM A653 G210. For precoated steel tubing (ASTM A653), recoat tube outside diameter weld seam by metallizing with zinc wire . Sign blanks shall be the sizes and shapes shown on the plans. Additional sign clamp required on the "T-bracket" post for 24" high signs. Place clamp at least 3" above bottom of sign when possible. . Sign supports shall not be spliced except where shown. Sign support posts shall B. See the Traffic Operations Division website for detailed drawings of sign clamps and Wedge Anchor System components. The website address is: Dig foundation hole. Where solid rock is encountered at ground level, the foundation shall be a minimum depth of 18". When solid rock is encountered below ground level, the foundation shall extend in the solid rock a minimum depth of 18" or provide a minimum foundation depth of 30". If solid rock is encountered, the socket/stub may be reduced in length as required to a minimum length of 18". Any material removed from the socket/stub shall be from the bottom and the clearance requirements given on SMD(GEN) must be followed. The Pipe Stub inner surfaces of the socket/stub must remain free of concrete or other debris. . The Engineer may permit batches of concrete less than 2 cubic yards to be mixed Base with a portable, motor driven concrete mixer. For small placements less than Plate 0.5 cubic yards, hand mixing in a suitable container may be allowed by Engineer. Place concrete into hole until it is approximately flush with the ground. Concrete shall be Class A. . Insert tubular socket into concrete until top of socket is approximaely 1/4 " above the concrete footing. 4. Plumb the socket. Allow a minimum 4 days for concrete to set, unless otherwise directed by Engineer.. 5. Attach the sign to the sign post. Insert the sign post into socket and align sign face with roadway.
 Drive the wedge into the socket to secure post. This will leave approximately 3 inches of the wedge exposed. UNIVERSAL ANCHOR SYSTEM INSTALLATION PROCEDURE 1. Dig foundation hole. Where solid rock is encountered at ground level, the foundation shall be a minimum depth of 18". When solid rock is encountered below ground level, the foundation shall extend in the solid rock a minimum depth of 18" or provide a minimum foundation depth of 30". If solid rock is encountered, the socket/stub may be reduced in length as required to a minimum length of 18". Any material removed from the socket/stub shall be from the bottom and the clearance requirements given on SMD(GEN) must be followed. The inner surfaces of the socket/stub must remain free of concrete or other debris. Insert base post in hole to depths shown and backfill hole with concrete. 3. Level and plumb the base post using a torpedo level and allow concrete adequate time to set. The bottom of the slots provided in the stub pipe shall remain above the top of the concrete foundation. 4. Attach the sign to the sign post. 5. Install plastic insert around bottom of post. . Insert sign post into base post. Lower until the post comes to rest on steel rod. Seat compression ring using a hammer. Typically, the top of compression ring will be approximately level with top of stub post when optimally installed. 8. Check sign post by hand to ensure it is unable to turn. If loose, increase the tightening of the compression ring. Texas Department of Transportation Traffic Operations Division SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS WEDGE & UNIVERSAL ANCHOR WITH THIN WALL TUBING POST SMD(TWT) - 08 
 ウト: TXDOT
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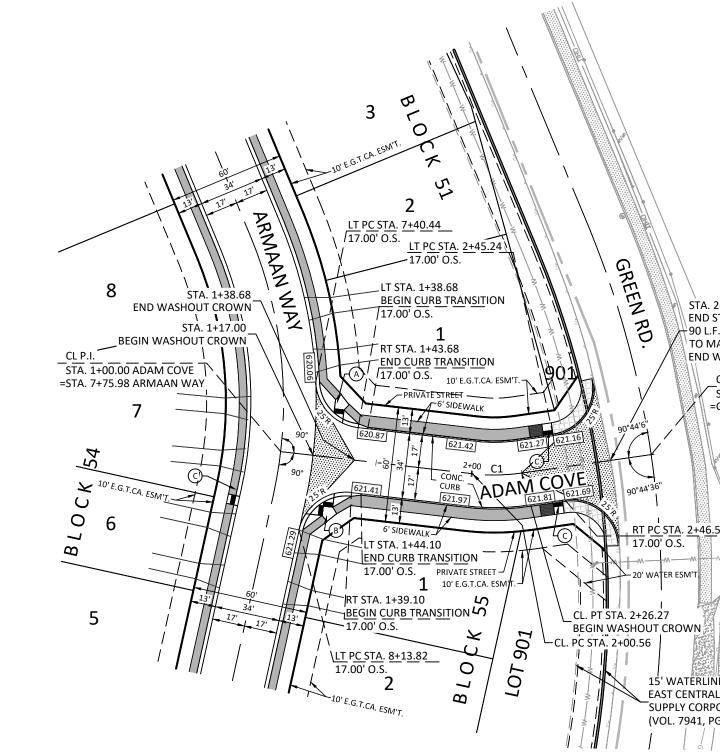
 CUNT
 SECT
 JU9
 HIGHWAY
 C TxDOT July 2002 SHEET CUUINTY

26E

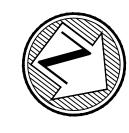
SMD(FRP) - 08

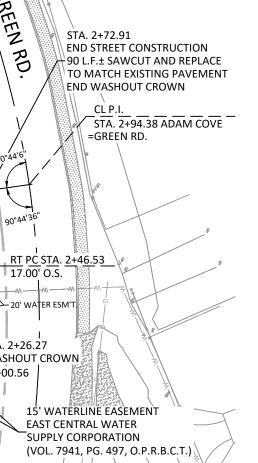
9 in 0.250" 9 in STREET NAME HWYC SIZE 4" PVT HWYC SIZE 2"

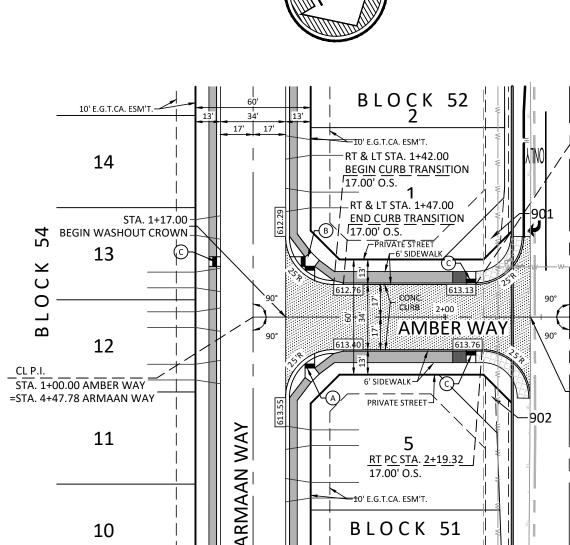
PLAT NO. 23-11800388	Figures       Engineers       No.       Date       Description         • Surveyors       • Surveyors       • Engineers       • Description       BY         • Surveyors       • Surveyors       • Description       BY         • Planners       • Description       BY       BY         • Dow Tarin Ramirez Engineers, LLC       • Dow Tarin Comarkon Path, SUITE 100       TEL: (210) 698–5051       BY         • PROJ. #       PROJ. #       Dow BY:       Dow BY:       DATE:       DATE:
US TRAIL       0.250         Dubit       0.250         S0 in       0.250	PAUL LANDA, JR. PAUL LANDA, JR. 100182 CENSED ONAL ENGL
30 in USTRAIL USTRAIL UST SUBMITTAL SET	LEGACY AT GREEN ENCLAVE, UNIT 1 TRAFFIC PLAN DETAILS SHEET C4.4



CURV IRVE LENGTH RADIUS	/E TABLE DELTA TANGENT CHORD	(PRIVAT	ESIR	EEI)	
C1 25.72' 100.00'	14°44'07" 12.93' 25.65'	STA. 1+00.0	00 TO STA. 2+72	2.91	HORIZONTAL SCA VERTICAL SCALE:
645		·····		·····	·····
640		+00.00 ADAM COVE +75.98 ARMAAN WAY RB PT STA. 1+38.68 URB TRANSITION RB PT STA. 1+39.10 URB TRANSITION URB TRANSITION URB TRANSITION CURB TRANSITION	RT PVI STA. 1+44.10 END CURB TRANSITION RT & LT PVI STA. 2+00.56 PVI ELEV (NO VC) LT CURB PC STA. 2+45.24	RT CURB PC STA. 2+46.53 STA. 2+72.91END STREET CONSTRUCTION. 90 L.F.± SAWCUT AND REPLACE TO MATCH EXIŠTING PVM'T	PVI STA. 3+50.00
635		CL. P.I. STA. 1+00.00 =STA. 7+75.98 LT CURB PT 5 BGN CURB TT BGN CURB TT BGN CURB TT LT PVI STA END CURB	RT PVI STA. 1+ END CURB TR/ RT & LT PVI STA PVI ELEV (NO V/ LT CURB PC STA.	TT CURB PC STA. STA. 2+72.91ENI CONSTRUCTION SAWCUT AND RE TO MATCH EXIST	RT & LT PVI STA PVI ELEV (NO V
630		0.50% (RT TOP OF CURB)–			
625	TRANSITION FR MOUNTABLE CURB 2% REQUIRED FOR ADA	EXISTING GROUND RT. % (RT TOP OF CURB) M STANDARD TO PAVEMENT SLOPE	PRC (ST/	DPOSED TOP OF CURB RT. ANDARD CURB) ROPOSED GUTTER RT. & LT. 0.60% (RT TOP OF CURB)	
620	PROPOSED PVM'T F			PROPOSED PVM'T RT	
615 610	PROPOSED PVM'	5.50% (LT TOP OF CURB) EXISTING GROUND LT. 0.50% (LT GUTTER) FILL @ COMPAC	PS% TION PROPOSED CL PVM'T.	PROPOSED GUTTER LT.	
605					
TOP OF CURB LT		621.17	621.42	621.16	
TOP OF CURB RT		621.41	621.98	621.69 620.97	
GUTTER LT & RT		620.59	620.84	620.55	
	0+00	1+00	2+00	3+00	4+00







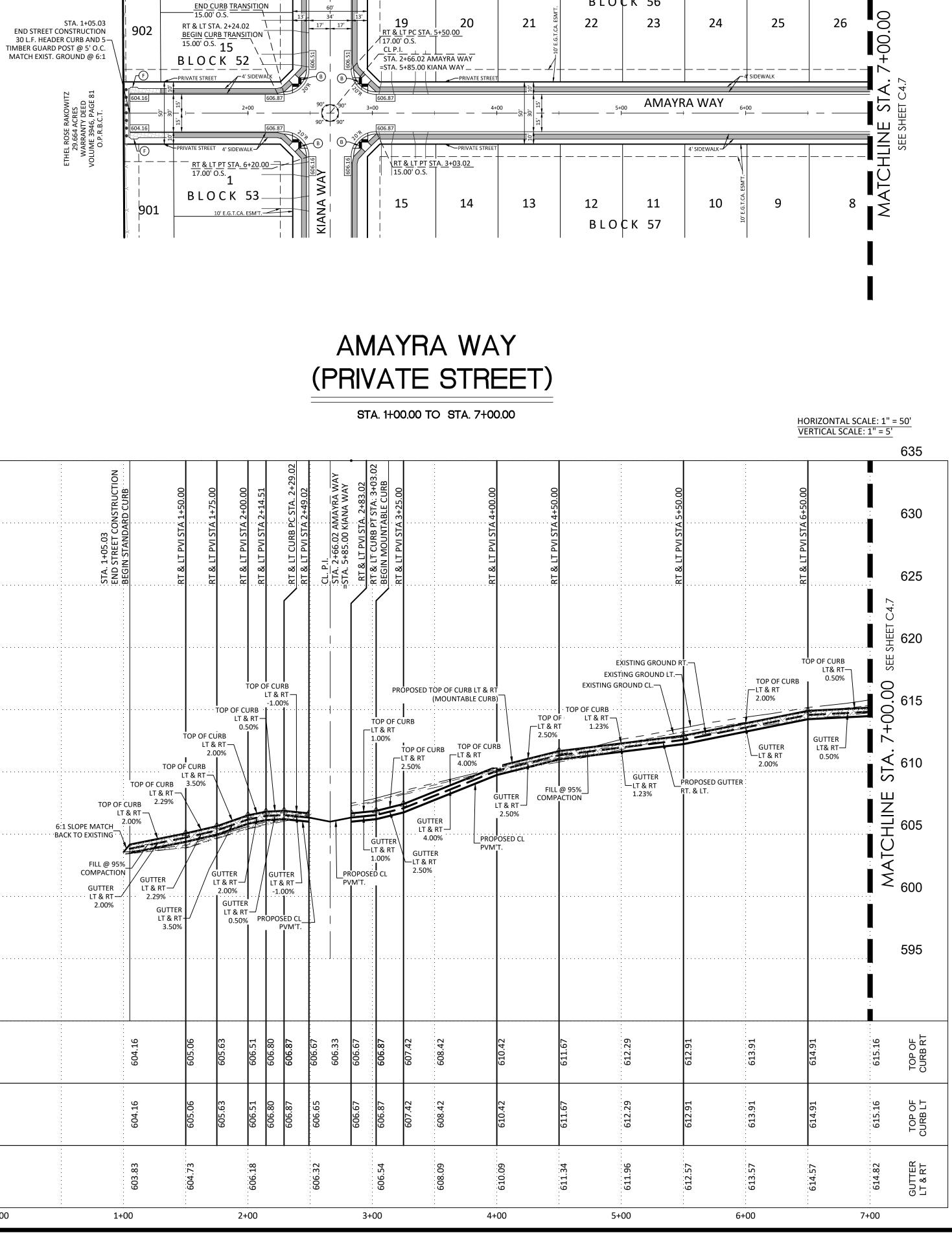
		PLAT NO. 23-1	1800388	DATE:
		LEGEND CONTRACTOR TO TIE EXISTING AND PROPOSED CURB/SIDEWALK. PRIOR TO CONSTRUCTION CONTRACTO	OR (1)	CHKD. BY:
		SHALL VERIFY ELEVATIONS. SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (SINGLE)		BÝ
Ň		SIDEWALK WHEELCHAIR RAMP - TYPE 10       50     100	B Scription	BY: DWN.
10' E.G.T.CA. ESM'T.		SIDEWALK WHEELCHAIR RAMP - TYPE II (DEVELOPER INSTALLED)	DESCRI	# DGN.
14	TO' E.G.T.CA. ESM'T. RT & LT STA. 1+42.00 BEGIN CURB TRANSITION	SIDEWALK WHEELCHAIR RAMP - TYPE I (DEVELOPER INSTALLED) SIDEWALK WHEELCHAIR RAMP - TYPE 11		PROJ.
STA. 1+17.00 BEGIN WASHOUT CROWN	17.00' O.S. RT & LT STA. 1+47.00 END CUBB TRANSITION B / 17.00' O.S. 901	(DEVELOPER INSTALLED) OFFSET PARALLEL RAMP SIDEWALK PASSING SPACE	E F	_
BEGIN WASHOUT CROWN 13 C U	PRIVATE STREET	EXISTING TOP OF CURB ELEVATION PROPOSED TOP OF CURB ELEVATION	805.81TC	_
	Image: Second condition         Image: Second			
12 <u>CL P.I.</u> STA. 1+00.00 AMBER WAY =STA. 4+47.78 ARMAAN WAY	STA. 2+44.44 6' SIDEWALK 6' SIDEWALK 7' S	LACE		
	PRIVATE STREET 902 TO MATCH EXISTING PAVEL END WASHOUT CROWN	MENT SIDEWALK WHEEL CHAIR RAMP (DEVELOPER INSTALLE	gineers rveyors -1013150	698–5051 598–5085
10	RT PC STA. 2+19.32       A         17.00' O.S.       A         10' E.G.T.CA. ESM'T.       A         BLOCK 51       B	WASH-OUT CROWN	· Eng · Sur · Pla RVEYING: F-	EL: (210) EX: (210) E
	BLOCK 51	POSSIBLE DRIVEWAY LOCATION PROPERTY LINE		E 100 TEL: FAX:
				VTH, SUITE S 78249
		PROPOSED CONCRETE CURB		ARRON P/
	MBER WAY	COSA CITY LIMIT LINE	BPELS: ENG	12770 CIMA San Anton
	VATE STREET)	HORIZONTAL SCALE: 1" = 50' VERTICAL SCALE: 1" = 5'		(U) !
640	A. 1+00.00 TO STA. 2+44.44	640	Pelly	4/201
00.00 AMBER WAY	TA. 1+42.00 ISITION TION +75.00 +16:00 +19.32 C+19.32 STREET STREET S5 L.F.± LACE ULACE	005	STATE OF TEAS	ST.
635 0.00 AMB 7.78 ARMAP	T CURB PT STA. 1+2 CURB TRANSITION +47.00 EV (NO VC) EV (NO VC) RB PC STA. 2+19.32 RB PC STA. 2+19.32 RB PC STA. 2+19.32 RB PC STA. 2+19.32 TRUCTION. 85 L.F. <u>±</u> UT AND REPLACE ATCH EXISTING PVN	635	PAUL LANDA, JR. 100182 CENSE VONAL	ALE
CL. P.I. STA. 1+00 =STA. 4+4.	RT & LT C BEGIN CL STA. 1+4 END CUR PVI ELEV STA. 2+44 TO MATC TO MATC	630	- IN CONTRACTOR	
625		625		] ]
620		620		!
6.01% (RT TOP OF CURB TRANSITION FROM STANDARD TO MOUNTABLE CURB 2% PAVEMENT SLOPE	3) PROPOSED TOP OF CURB RT. C (STANDARD CURB)			) ]
REQUIRED FOR ADA RAMPS PROVIDED	E- D0.50% (RT TOP OF CURB)	615		, , ,
PROPOSED PVM'T RT.			PR( PR( PR	.+72.91 2+44.44
610 PROPOSED PVM'T LT.		610	NAY AY	O STA. 2 TO STA.
005	FILL @ 95%     PROPOSED TOP OF CURB LT.       COMPACTION     (STANDARD CURB)	605		-00.00 T +00.00
	0.50% (LT TOP OF CURB)— ← EXISTING GROUND CL.		ANB ET F MB F	STA. 1+ & STA. 1
600		600	REI ⊗ ∧	, ~~ ;
			LEGAO STR OVE 8	>
			Ŭ	
TOP OF CURB LT	612.76 613.09 613.34 613.78 613.78 613.78	TOP OF CURB LT	ADAM	
B R T	613.73 613.98 613.98 613.06 613.06	P O C T T T T T T T T T T T T T T T T T T		
TOP OF CURB RT	613.76 613.75 613.86 613.86 613.06 613.06	TOP OF CURB RT	SHEET	
GUTTER LT & RT	612.50	GUTTER LT & RT		
0+00     1+00	2+00 3+00		MITTAL SET C4.5	)

)		HORIZ VERTI	ONTAL SCALE: 1" CAL SCALE: 1" = 5'	= 50'
			0,1200,121120	645
TO MATCH EXISTING PVM'T	T PVI STÅ. 3+50.00	PVI ELEV (NO VC)		640
TO M/	RT & I	PVI EL		635
				630
URB RT	:			625
	) D PVM'T RT.			620
ED GUT	TER LT.			
OF CUP OF CURE 3)				615
				610
				605
				TOP OF CURB LT
				TOP OF CURB RT
				GUTTER LT & RT
3+	00	4+	-00	

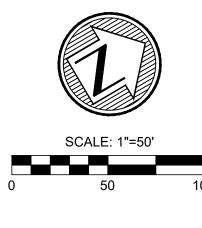
> 81 ETHEL ROSE RAKOWIT 29.664 ACRES WARRANTY DEED VOLUME 3946, PAGE 8 O.P.R.B.C.T.

10' E.G.T.CA. ESM'T.-RT & LT STA. 2+29.02

STA. 1+05.03 END STREET CONSTRUCTION BEGIN STANDARD CURB	
STA. 1+( END STF	
TOP OF CURI LT & R 6:1 SLOPE MATCH BACK TO EXISTING	
FILL @ 95%_ COMPACTION	
GUTTER LT & RT 2.00%	
604.16	
604.16	
603.83	
	· ·



BLOCK 56



635

630

625

LEE 620

595

TOP OF CURB RT

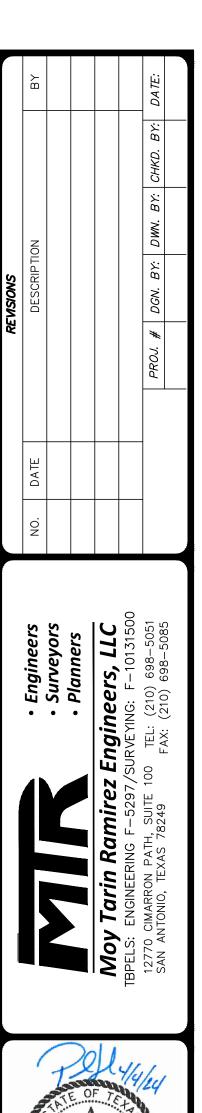
TOP OF CURB LT

GUTTER LT & RT

SEE

CALE: 1"=50'		DIREC
50	100	SIDEW DIREC
		SIDEW (DEVE
		SIDEV (DEVE
		SIDEW (DEVE
		SIDEV
		EXIST
		PROP
		HOME
		DEVE
		SIDEV
		WASH
		POSS
		PROP
		EXIST
		PROP

PLAT NO. 23-11	800388
LEGEND	
CONTRACTOR TO TIE EXISTING AND PROPOSED CURB/SIDEWALK. PRIOR TO CONSTRUCTION CONTRACTOR SHALL VERIFY ELEVATIONS.	1
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (SINGLE)	A
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (DUAL)	B
SIDEWALK WHEELCHAIR RAMP - TYPE II (DEVELOPER INSTALLED)	C
SIDEWALK WHEELCHAIR RAMP - TYPE I (DEVELOPER INSTALLED)	D
SIDEWALK WHEELCHAIR RAMP - TYPE 11 (DEVELOPER INSTALLED) OFFSET PARALLEL RAMP	E
SIDEWALK PASSING SPACE	F
EXISTING TOP OF CURB ELEVATION	805.81TC
PROPOSED TOP OF CURB ELEVATION	805.81
HOME BUILDER INSTALLED SIDEWALK	
DEVELOPER INSTALLED SIDEWALK	
SIDEWALK WHEEL CHAIR RAMP (DEVELOPER INSTALLED)	00000000000000000000000000000000000000
WASH-OUT CROWN	
POSSIBLE DRIVEWAY LOCATION	
PROPERTY LINE	
EXISTING CONTOUR -	— — — — 1120 — — — —
PROPOSED CONTOUR -	1120
PROPOSED CONCRETE CURB =	
FLOW ARROW	
COSA CITY LIMIT LINE	



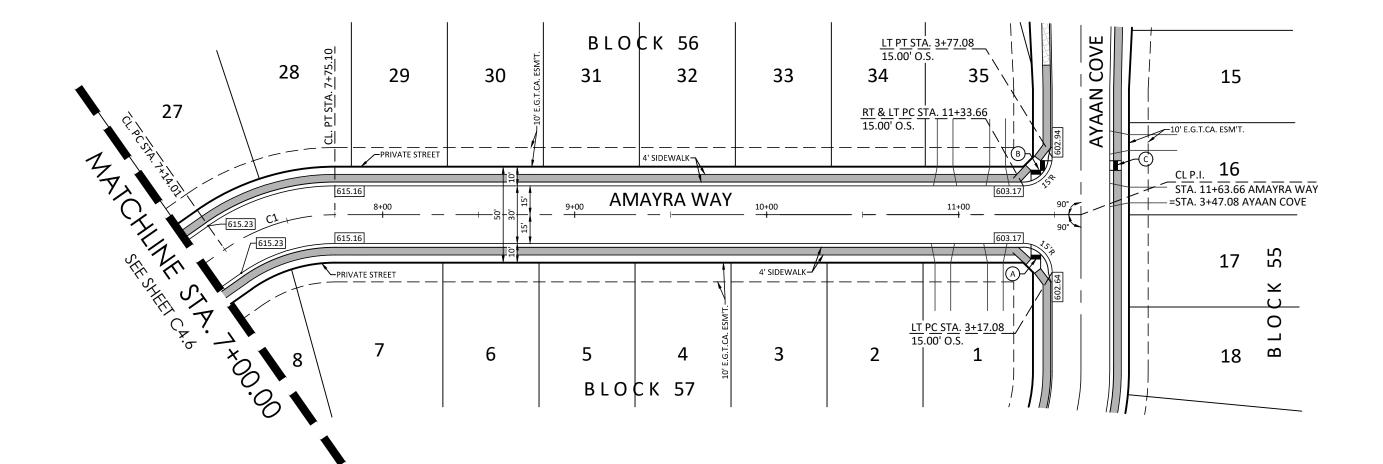


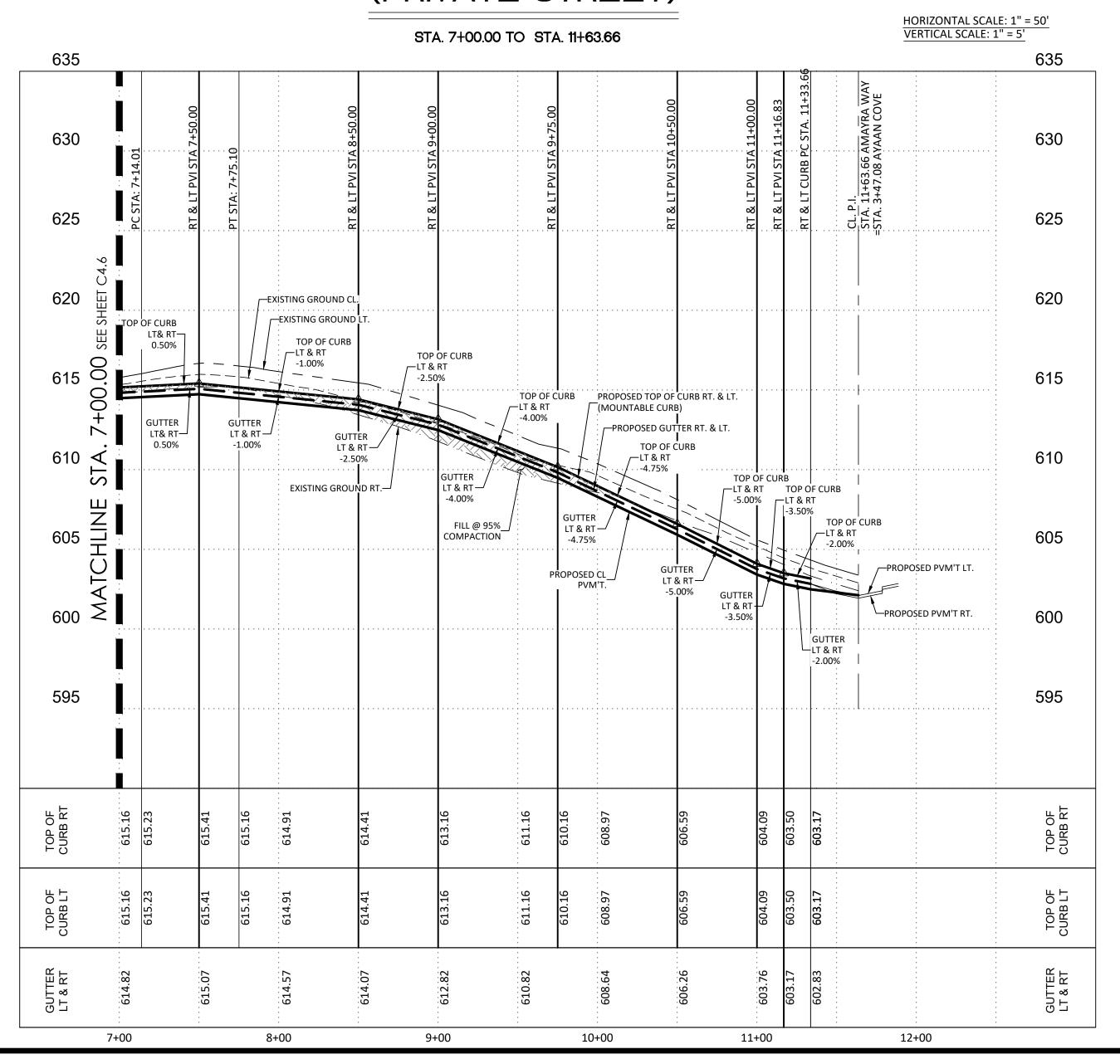
STREET PLAN & PROFILE AMAYRA WAY (PRIVATE STREET)	
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SHEET

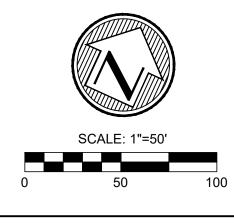
SUBMITTAL SET

C4.6





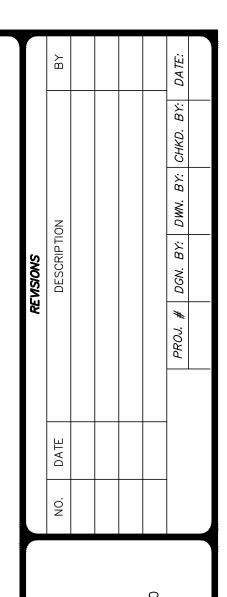




		CUR	/E TABLE		
٧E	LENGTH	RADIUS	DELTA	TANGENT	CHORD
	61.09'	100.00'	35°00'00"	31.53'	60.14'

PLAT NO. 23-1	1800388
LEGEND	
CONTRACTOR TO TIE EXISTING AND PROPOSED CURB/SIDEWALK. PRIOR TO CONSTRUCTION CONTRACTO SHALL VERIFY ELEVATIONS.	R (1
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (SINGLE)	A
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (DUAL)	В
SIDEWALK WHEELCHAIR RAMP - TYPE II (DEVELOPER INSTALLED)	©
SIDEWALK WHEELCHAIR RAMP - TYPE I (DEVELOPER INSTALLED)	D
SIDEWALK WHEELCHAIR RAMP - TYPE 11 (DEVELOPER INSTALLED) OFFSET PARALLEL RAMP	E
SIDEWALK PASSING SPACE	F
EXISTING TOP OF CURB ELEVATION	805.81TC
PROPOSED TOP OF CURB ELEVATION	805.81
HOME BUILDER INSTALLED SIDEWALK	
DEVELOPER INSTALLED SIDEWALK	
SIDEWALK WHEEL CHAIR RAMP (DEVELOPER INSTALLEI	D)
WASH-OUT CROWN	
POSSIBLE DRIVEWAY LOCATION	
PROPERTY LINE	
EXISTING CONTOUR	- — — — 1120 — — — —
PROPOSED CONTOUR -	1120
PROPOSED CONCRETE CURB	
FLOW ARROW	<b>&gt;</b>

COSA CITY LIMIT LINE





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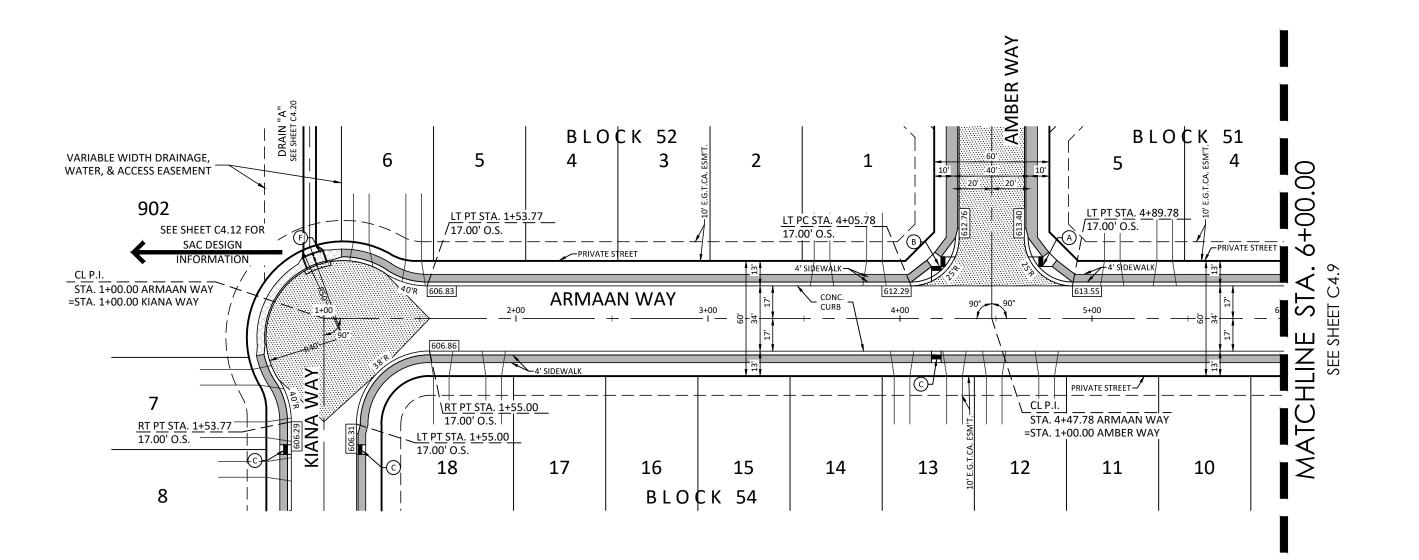
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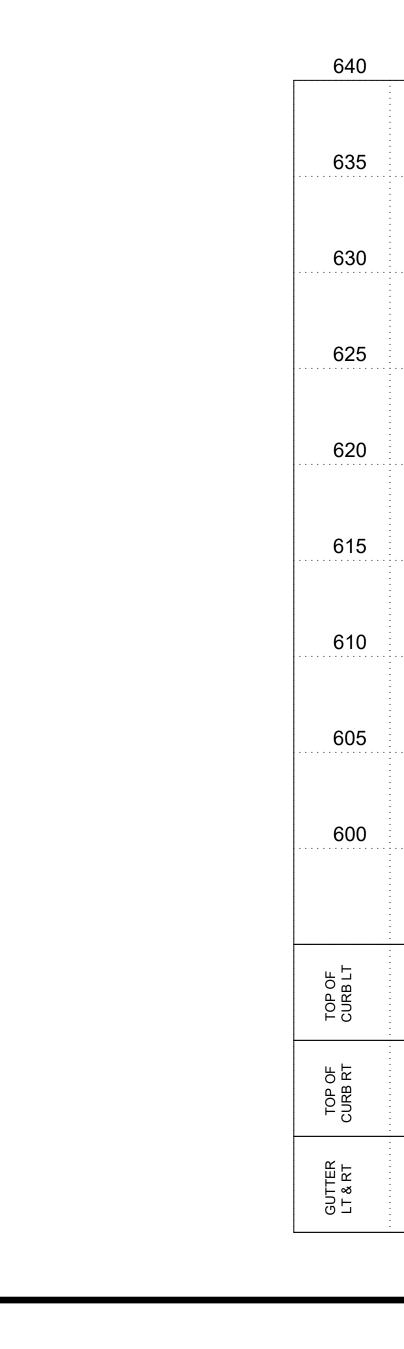
STREET) UNIT PROFILE ∃∕ (PRIVATE ళ PLAN WAY E S  $\vdash$ STREE AMAYRA V LEGAC

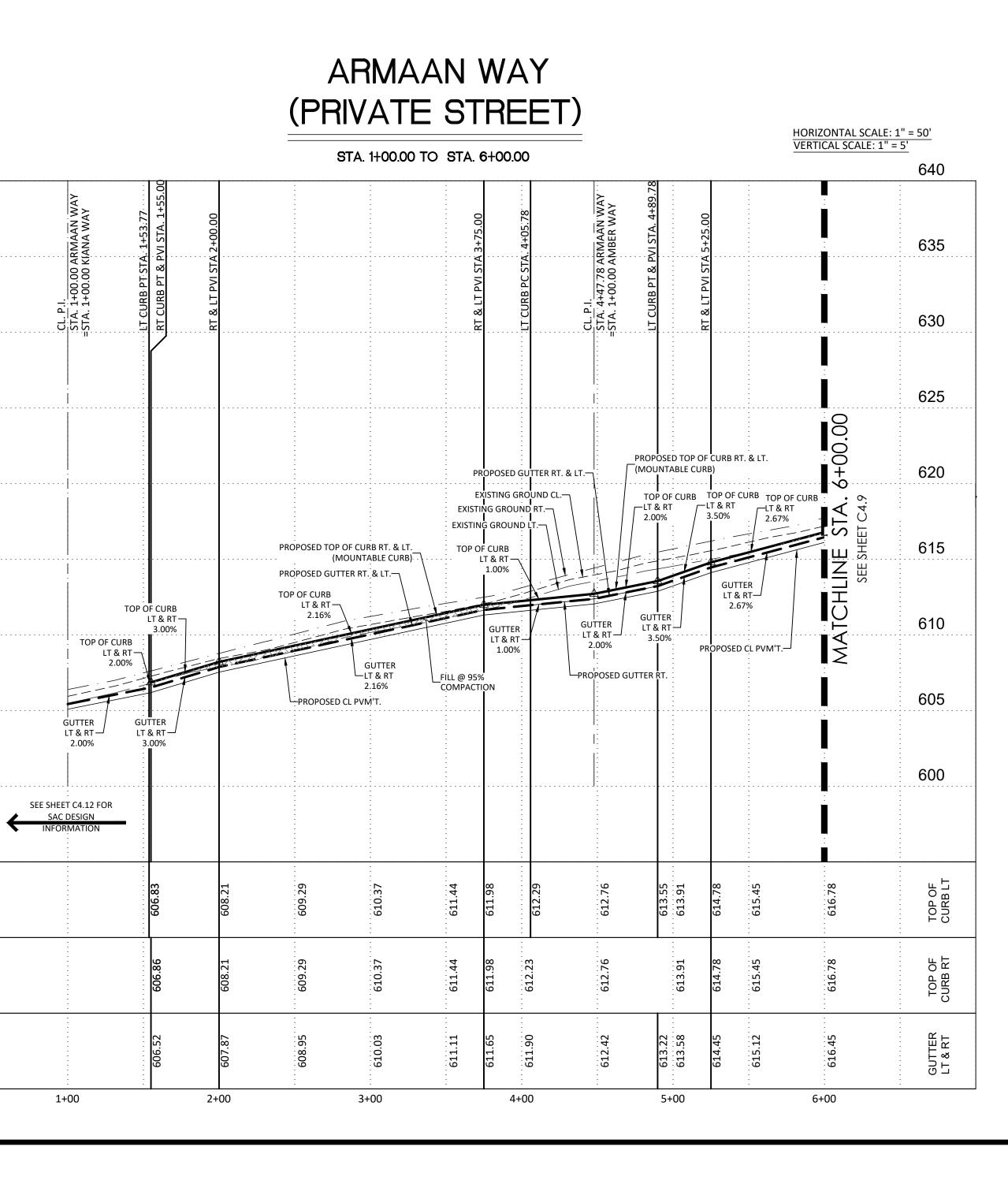
SHEET

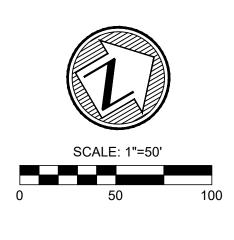
C4.7

SUBMITTAL SET





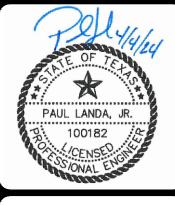




PLAT NO. 23-118	00388
LEGEND	
CONTRACTOR TO TIE EXISTING AND PROPOSED CURB/SIDEWALK. PRIOR TO CONSTRUCTION CONTRACTOR SHALL VERIFY ELEVATIONS.	1
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (SINGLE)	A
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (DUAL)	B
SIDEWALK WHEELCHAIR RAMP - TYPE II (DEVELOPER INSTALLED)	©
SIDEWALK WHEELCHAIR RAMP - TYPE I (DEVELOPER INSTALLED)	D
SIDEWALK WHEELCHAIR RAMP - TYPE 11 (DEVELOPER INSTALLED) OFFSET PARALLEL RAMP	E
SIDEWALK PASSING SPACE	F
EXISTING TOP OF CURB ELEVATION	805.81TC
PROPOSED TOP OF CURB ELEVATION	805.81
HOME BUILDER INSTALLED SIDEWALK	
DEVELOPER INSTALLED SIDEWALK	
SIDEWALK WHEEL CHAIR RAMP (DEVELOPER INSTALLED)	
WASH-OUT CROWN	
POSSIBLE DRIVEWAY LOCATION	
PROPERTY LINE	
EXISTING CONTOUR	
PROPOSED CONTOUR	1120
PROPOSED CONCRETE CURB	
FLOW ARROW	
COSA CITY LIMIT LINE	

			RE	REVISIONS			
	NO.	DATE		DESCRIP TION	N		ВΥ
00							
2			PROJ. #	DGN. BY:	DWN. BY:	PROJ. # DGN. BY: DWN. BY: CHKD. BY: DATE:	DA TE:





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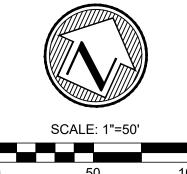
STREET) UNIT PROFILE Ш (PRIVATE ళ AN WAY  $\vdash$ STREE ARMAAN 4  $\triangleleft$ 511

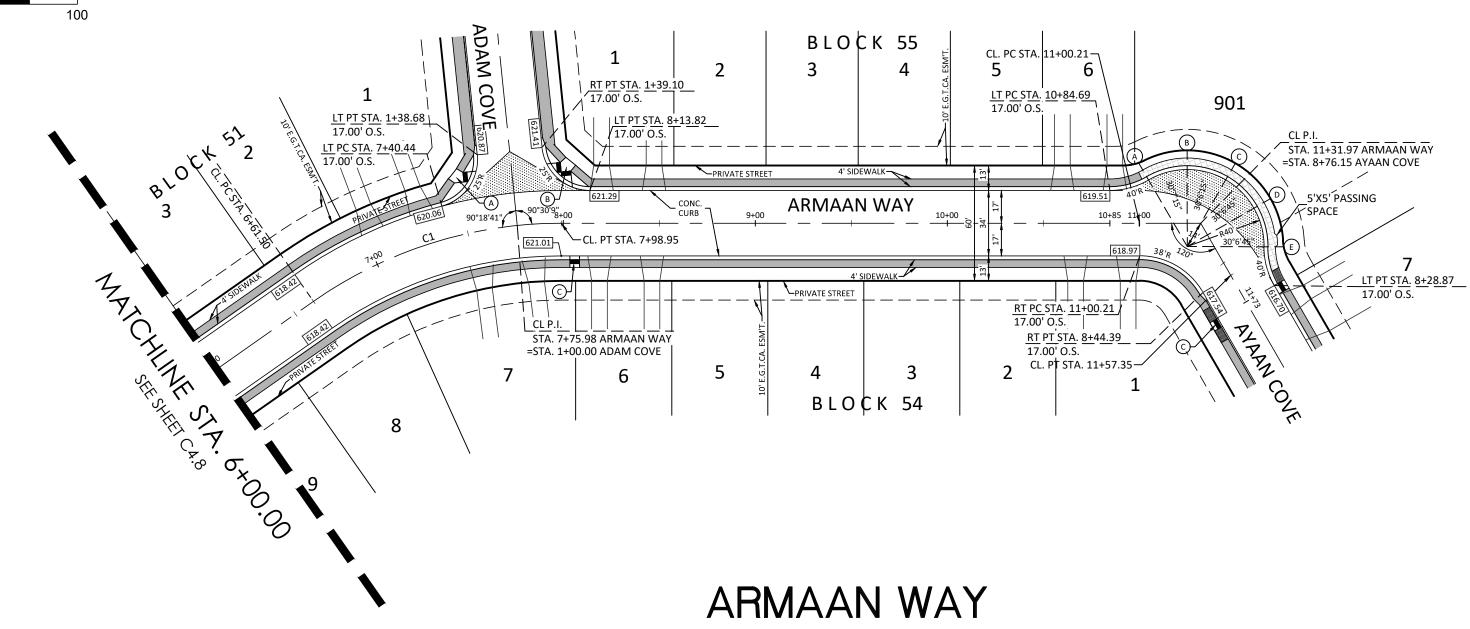
SHEET

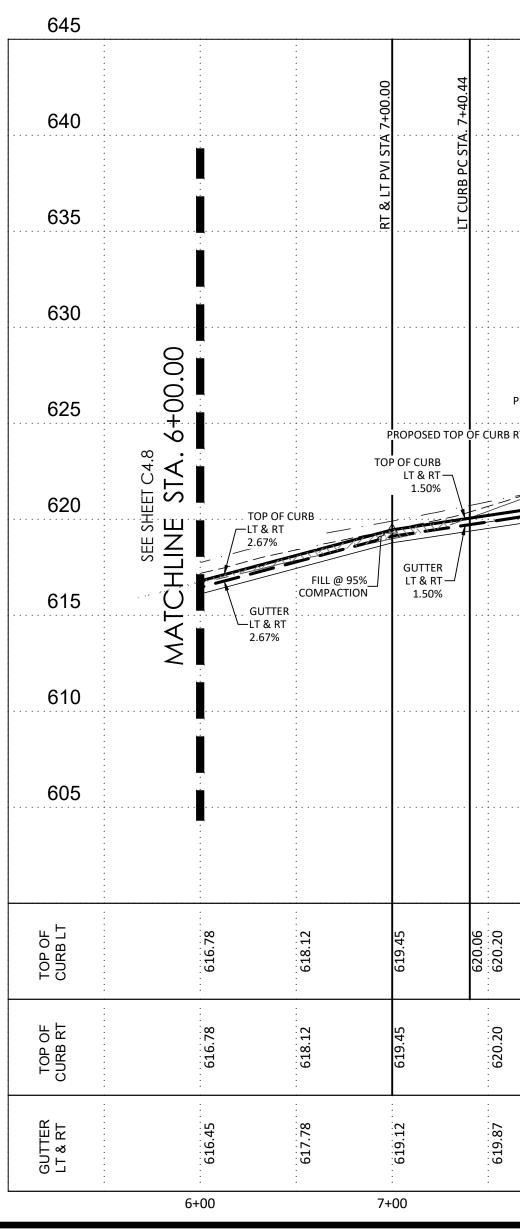
SUBMITTAL SET

C4.8

	<b>F</b>						
		CUR	/E TABLE				
CURVE	LENGTH	RADIUS	DELTA	TANGENT	CHORD		
C1	137.44'	225.00'	35°00'00"	70.94'	135.32'		







(PRIVATE STREET)

	STA. 6+00	.00 TO STA. 11+73	.32				HORIZONTAL SCALE: 1" VERTICAL SCALE: 1" = 5	
·····	·····	·····		·····	1	·····		645
CL. P.I. STA. 7+75.98 ARMAAN WAY STA. 1+00.00 ADAM COVE LT CURB PT STA. 8+13.82	& LT PVI STA 9+00.00	STA	RT & LT PVI STA 10+00.00 RT & LT PVI STA 10+24.98	& LT PVI STA 10+50.00 & LT PVI STA 10+60.03		P.I. A. 11+31.97 ARMAAN WAY A. 8+76.15 AYAAN COVE CURB PT STA. 11+73.32		640
CL. P STA. STA. IT CL	RT &		RT &	RT &	LT CL	CL. P.I. STA. 1 =STA. 8 LT CUI		635
			-EXISTING GROUND	 RT.				630
PROPOSED TOP OF (MO PROPOSED GUTTER F B RT. TOP OF CUR LT & R	DUNTABLE CURB) TOP OF RT. & LT		TOP OF CURB	LT & RT TO	P OF CURB & RT 00% • TOP OF CU • LT & RT -5.00%	JRB · · · · · · · · · · · · · · · · · · ·		625
	GUTTER LT LT & RT	GUTTER GUTTER LT & RT T & RT 0.50%	GUTTER LT & RT -2.50% GUTTE			P OF CURB & RT 50%		620
PROPC	1.85% PROPOSED PVM'T LT & RT OSED GUTTER LT.		LT & R -4.009 GUTT LT & 5.0	T	GUTTER LT & RT ···· -2.00%			615
				-3.50%	-3.18 CL PVM PROPOSED P ALONG VA	% 'TL PAVEMENT CL ALLEY GUTTER		610
						   		605
						<u> </u>		
621.03	621.29 621.96	622.88 623.13	622.63	622.01 621.01	620.06 618.84	617.89 617.43 616.97	616.02	TOP OF CURB LT
621.03	621.96	622.88 623.13	622.63	622.01 621.01	620.06	618.97		TOP OF CURB RT
620.70	621.63	622.55 622.80	622.30	621.68 620.68	619.72 619.06	618.82		GUTTER LT & RT
8+00	9+	+00	10+00		11+	00	12+00	

10+85 11+00 RADIUS POINT 1

6

KNU POINT T PC STA. 10+84 А В С D E F G Н LT PT STA. 8+28.8

CONTRACTOR TO THE EXISTING AND PROPOSED	(1)
CURB/SIDEWALK. PRIOR TO CONSTRUCTION CONTRACTOR SHALL VERIFY ELEVATIONS.	
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (SINGLE)	A
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (DUAL)	B
SIDEWALK WHEELCHAIR RAMP - TYPE II (DEVELOPER INSTALLED)	©
SIDEWALK WHEELCHAIR RAMP - TYPE I (DEVELOPER INSTALLED)	D
SIDEWALK WHEELCHAIR RAMP - TYPE 11 (DEVELOPER INSTALLED) OFFSET PARALLEL RAMP	E
SIDEWALK PASSING SPACE	F
EXISTING TOP OF CURB ELEVATION	805.81TC
PROPOSED TOP OF CURB ELEVATION	805.81
HOME BUILDER INSTALLED SIDEWALK	
DEVELOPER INSTALLED SIDEWALK	
SIDEWALK WHEEL CHAIR RAMP (DEVELOPER INSTALLED)	
WASH-OUT CROWN	
POSSIBLE DRIVEWAY LOCATION	
PROPERTY LINE	
EXISTING CONTOUR	— — — 1120 — — — — —
PROPOSED CONTOUR	1120
PROPOSED CONCRETE CURB	



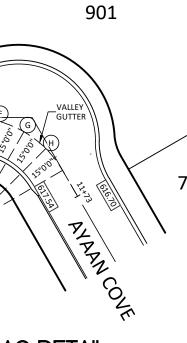
STREET

SHEET

C4.9

 $\operatorname{All}$ 

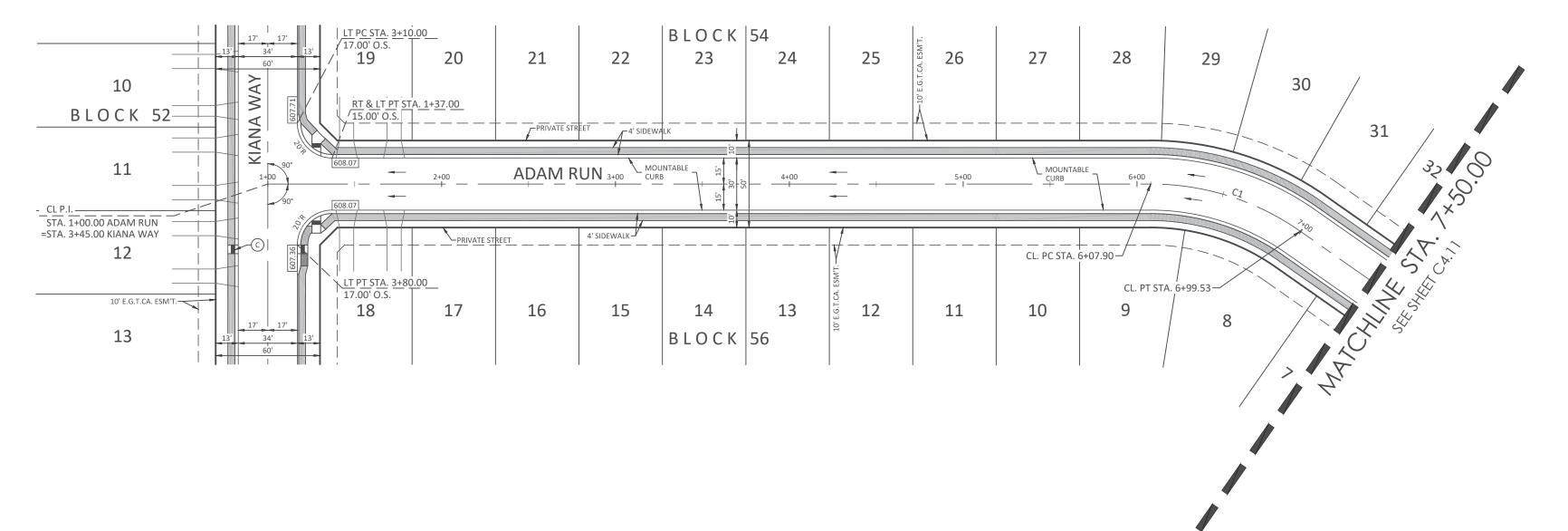
LEGAC

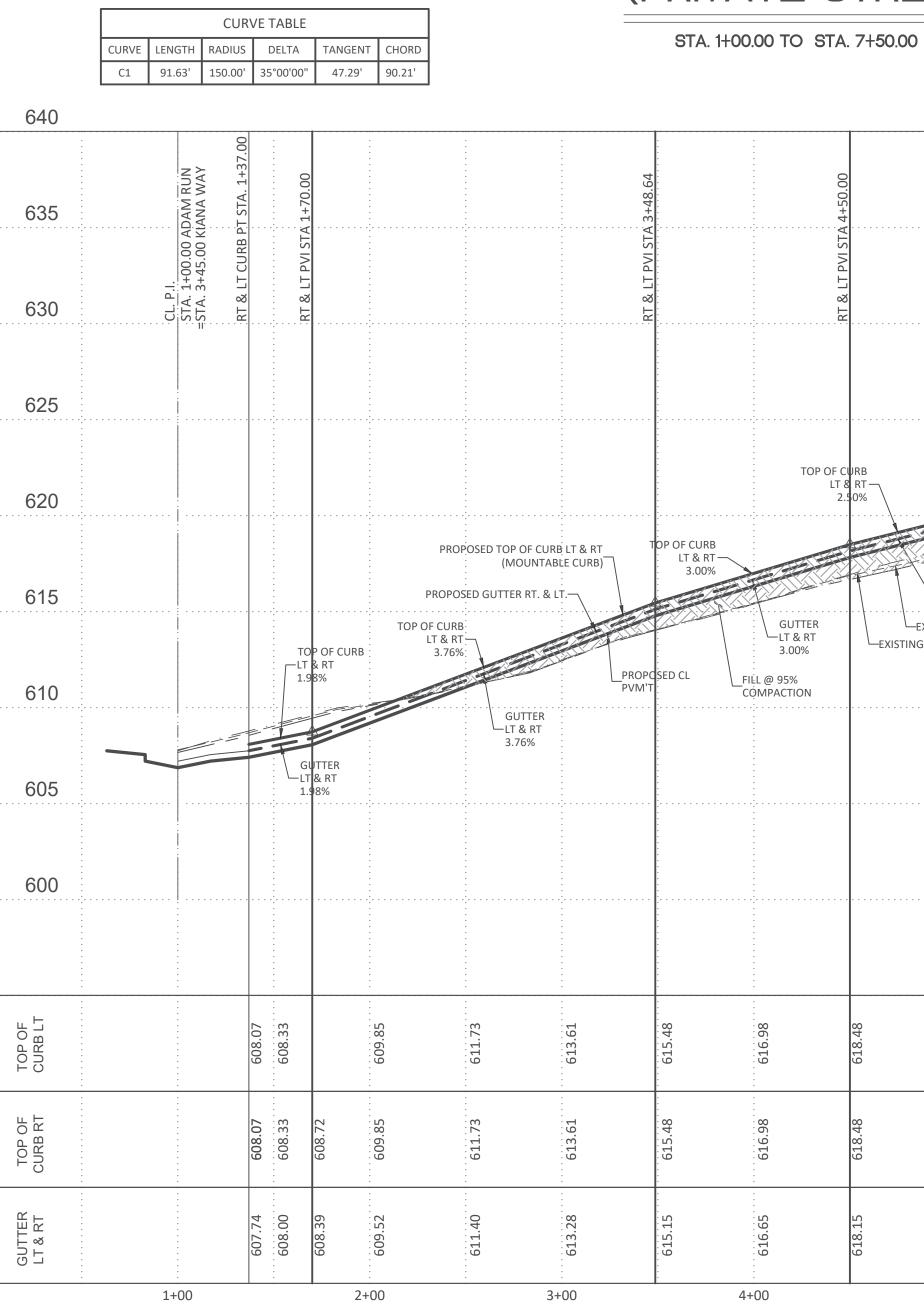


SAC DETAIL

UCKLE SAC TABLE			
	TOP OF CURB	GUTTER	
4.69	619.51	619.18	
	619.04	618.71	
	618.57	618.24	
	618.10	617.77	
	617.63	617.30	
	617.17	616.84	
	617.89	555.34	
	617.43	555.52	
	616.97	555.70	
8.87	616.70	616.36	

SUBMITTAL SET

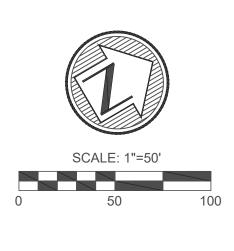






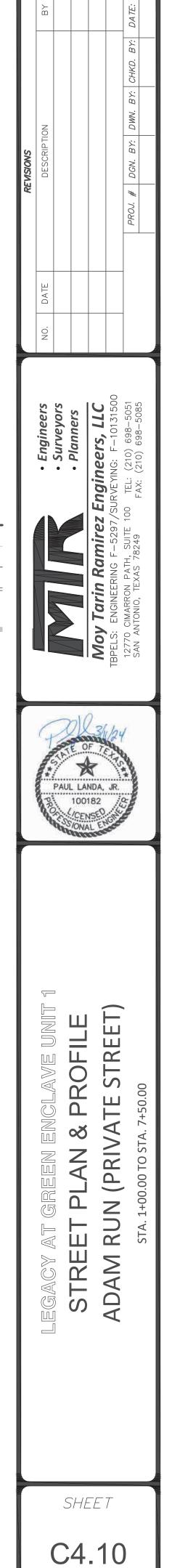
640 635 630 625 8 TOP OF CURB LT & RT 0.50% TOP OF CURB LT & RT 1.31% 50 TOP OF CURB LT 8 RT — 620 ETTA. GUTTER LT & RT 0.50% GUTTER LT & RT 1.31% JTTER EXISTING GROUND LT. 615 GUTTER LT & RT 3.00% EXISTING GROUND CL. EXISTING GROUND RT. 610  $\leq$ 605 600 TOP OF CURB LT 620.39 621.36 <u>621.04</u> 621.15 21 TOP OF CURB RT 620.39 621.36 10 GUTTER LT & RT 05 02 52 520 621 8+00 5+00 6+00 7+00

HORIZONTAL SCALE: 1" = 50' VERTICAL SCALE: 1" = 5'

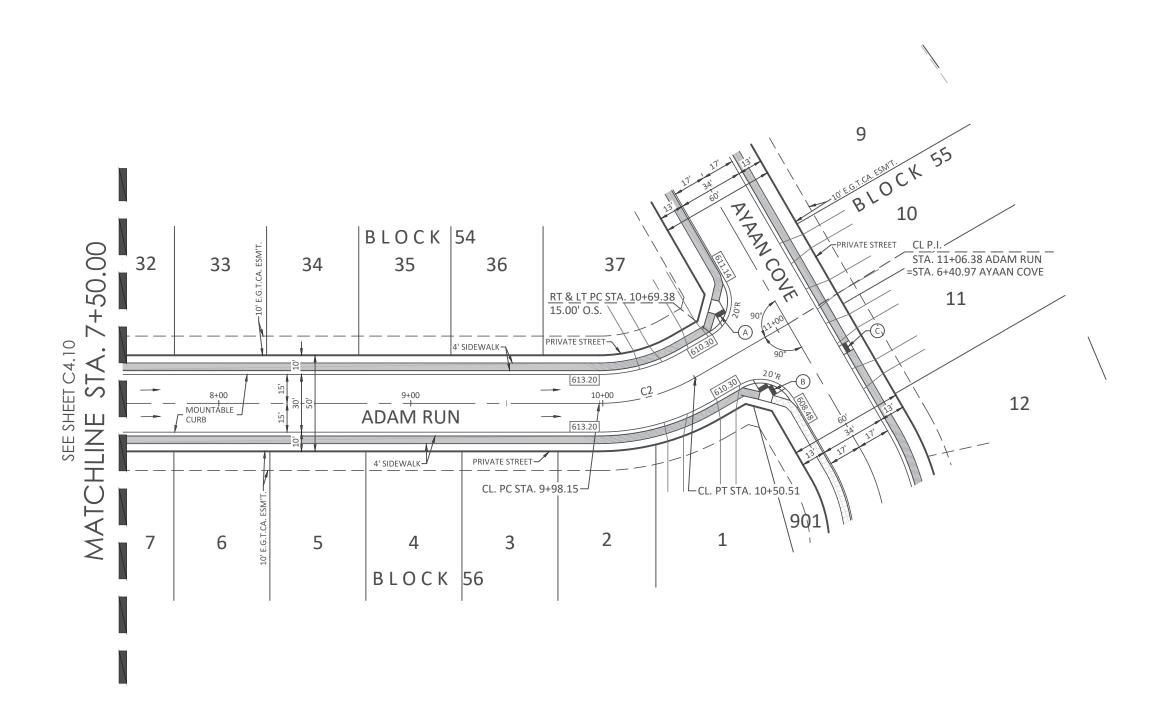


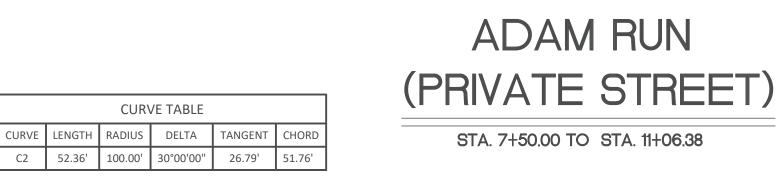
PLAT NO. 23-118	00388
LEGEND	
CONTRACTOR TO TIE EXISTING AND PROPOSED CURB/SIDEWALK. PRIOR TO CONSTRUCTION CONTRACTOR SHALL VERIFY ELEVATIONS.	(1)
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (SINGLE)	A
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (DUAL)	B
SIDEWALK WHEELCHAIR RAMP - TYPE II (DEVELOPER INSTALLED)	C
SIDEWALK WHEELCHAIR RAMP - TYPE I (DEVELOPER INSTALLED)	D
SIDEWALK WHEELCHAIR RAMP - TYPE 11 (DEVELOPER INSTALLED) OFFSET PARALLEL RAMP	E
SIDEWALK PASSING SPACE	F
EXISTING TOP OF CURB ELEVATION	805.81TC
PROPOSED TOP OF CURB ELEVATION	805.81
HOME BUILDER INSTALLED SIDEWALK	
DEVELOPER INSTALLED SIDEWALK	
SIDEWALK WHEEL CHAIR RAMP (DEVELOPER INSTALLED)	
WASH-OUT CROWN	
POSSIBLE DRIVEWAY LOCATION	
PROPERTY LINE	
EXISTING CONTOUR	- — -1120 - — -
PROPOSED CONTOUR	1120
PROPOSED CONCRETE CURB	
FLOW ARROW	
COSA CITY LIMIT LINE	

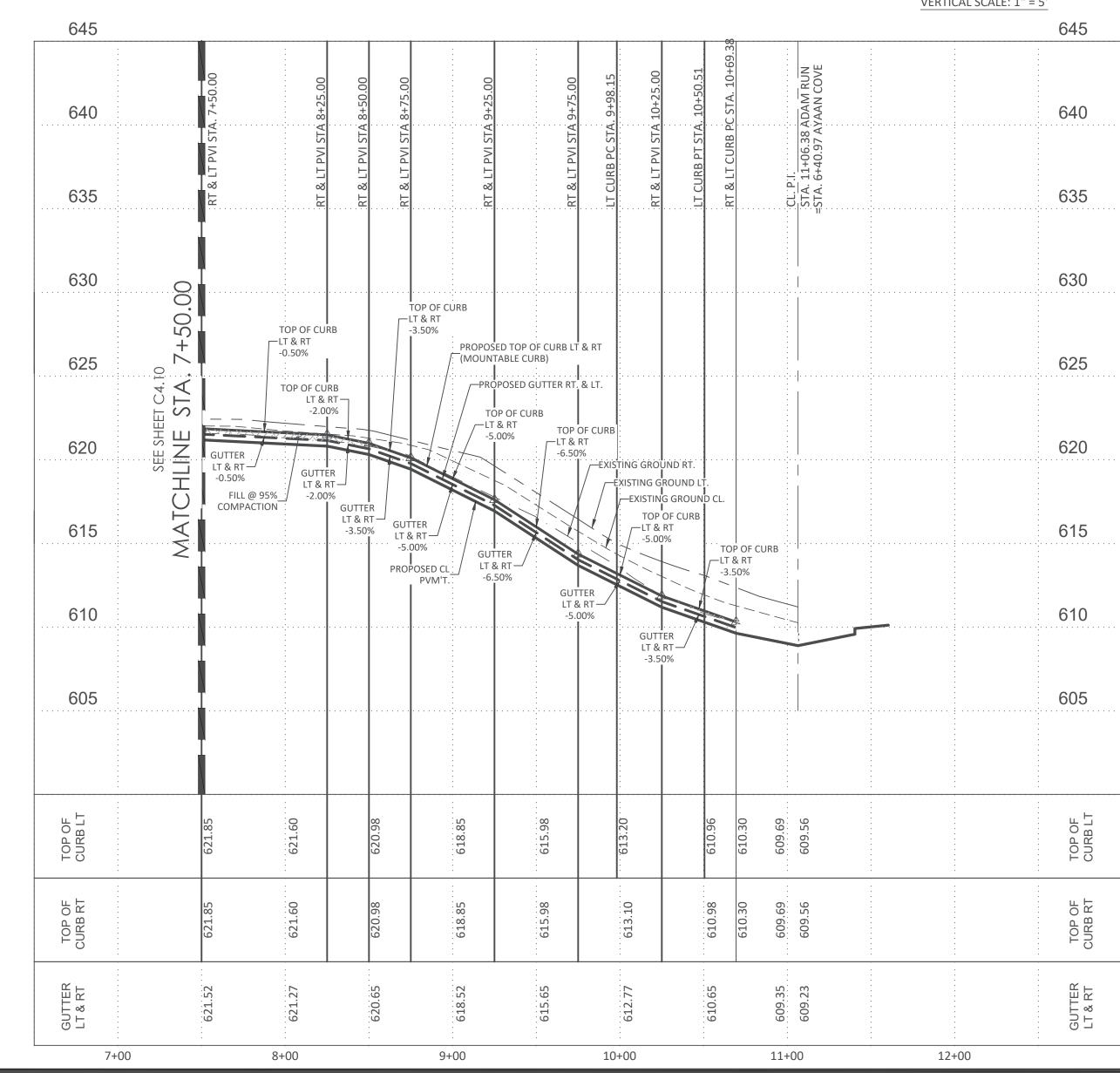
CAUTION!!! CONTRACTOR TO VERIFY EXISTING CONDITIONS BEFORE CONSTRUCTION. IF ANY DISCREPANCIES NOTIFY ENGINEER



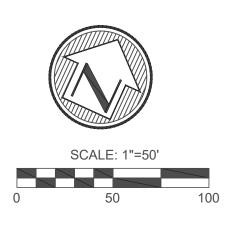
SUBMITTAL SET





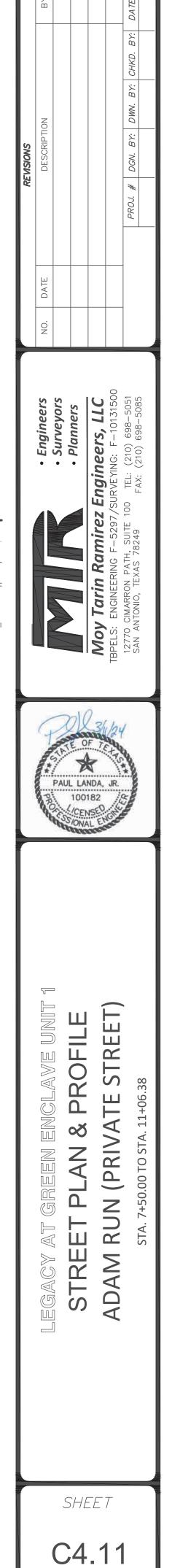


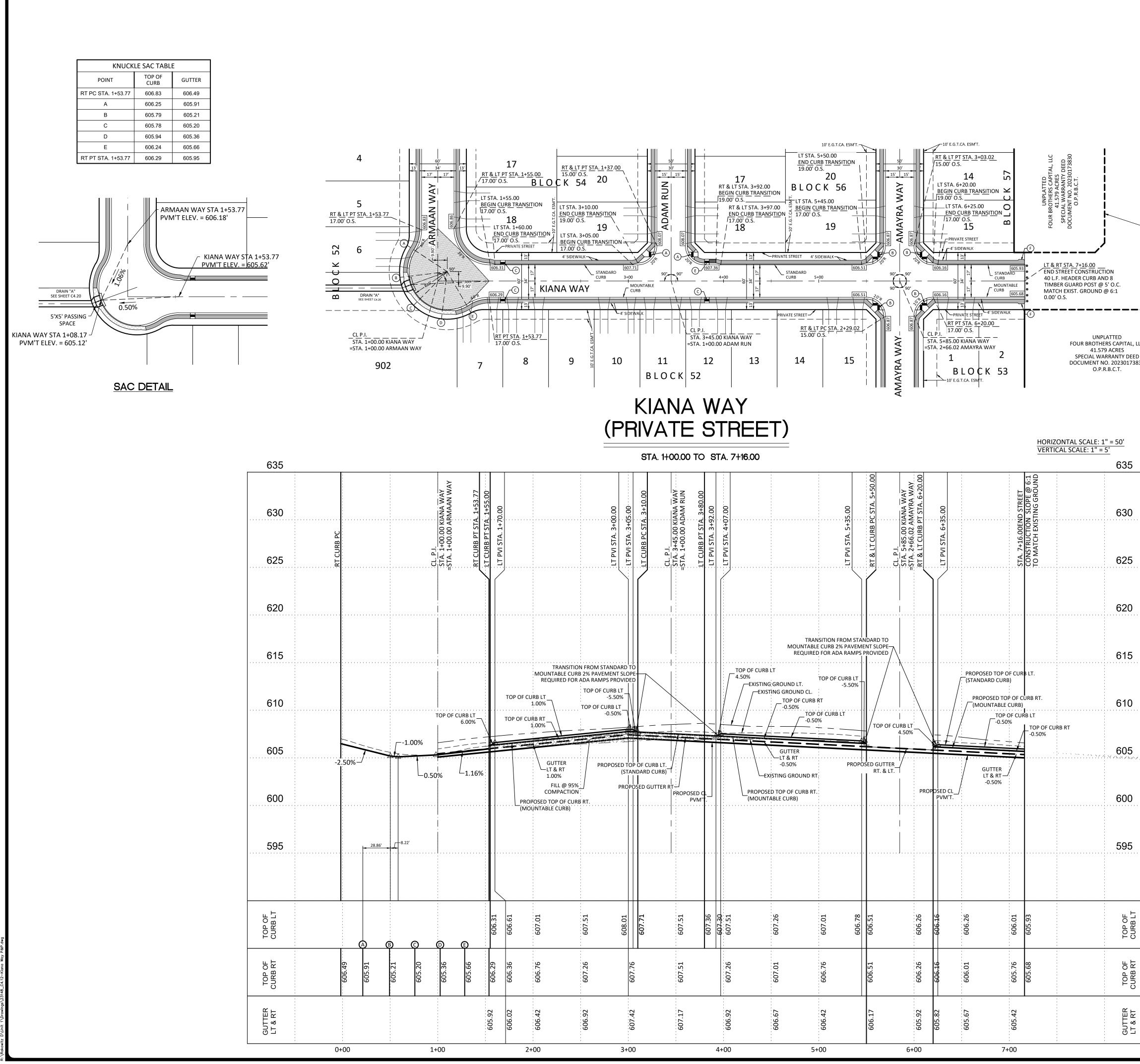
HORIZONTAL SCALE: 1" = 50' VERTICAL SCALE: 1" = 5'



PLAT NO. 23-118	00388
LEGEND	
CONTRACTOR TO TIE EXISTING AND PROPOSED CURB/SIDEWALK. PRIOR TO CONSTRUCTION CONTRACTOR SHALL VERIFY ELEVATIONS.	1
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (SINGLE)	A
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (DUAL)	B
SIDEWALK WHEELCHAIR RAMP - TYPE II (DEVELOPER INSTALLED)	C
SIDEWALK WHEELCHAIR RAMP - TYPE I (DEVELOPER INSTALLED)	D
SIDEWALK WHEELCHAIR RAMP - TYPE 11 (DEVELOPER INSTALLED) OFFSET PARALLEL RAMP	E
SIDEWALK PASSING SPACE	F
EXISTING TOP OF CURB ELEVATION	805.81TC
PROPOSED TOP OF CURB ELEVATION	805.81
HOME BUILDER INSTALLED SIDEWALK	
DEVELOPER INSTALLED SIDEWALK	
SIDEWALK WHEEL CHAIR RAMP (DEVELOPER INSTALLED)	
WASH-OUT CROWN	
POSSIBLE DRIVEWAY LOCATION	
PROPERTY LINE	
EXISTING CONTOUR	- — -1120 - — -
PROPOSED CONTOUR	1120
PROPOSED CONCRETE CURB	
FLOW ARROW	
COSA CITY LIMIT LINE	

CAUTION!!! CONTRACTOR TO VERIFY EXISTING CONDITIONS BEFORE CONSTRUCTION. IF ANY DISCREPANCIES NOTIFY ENGINEER





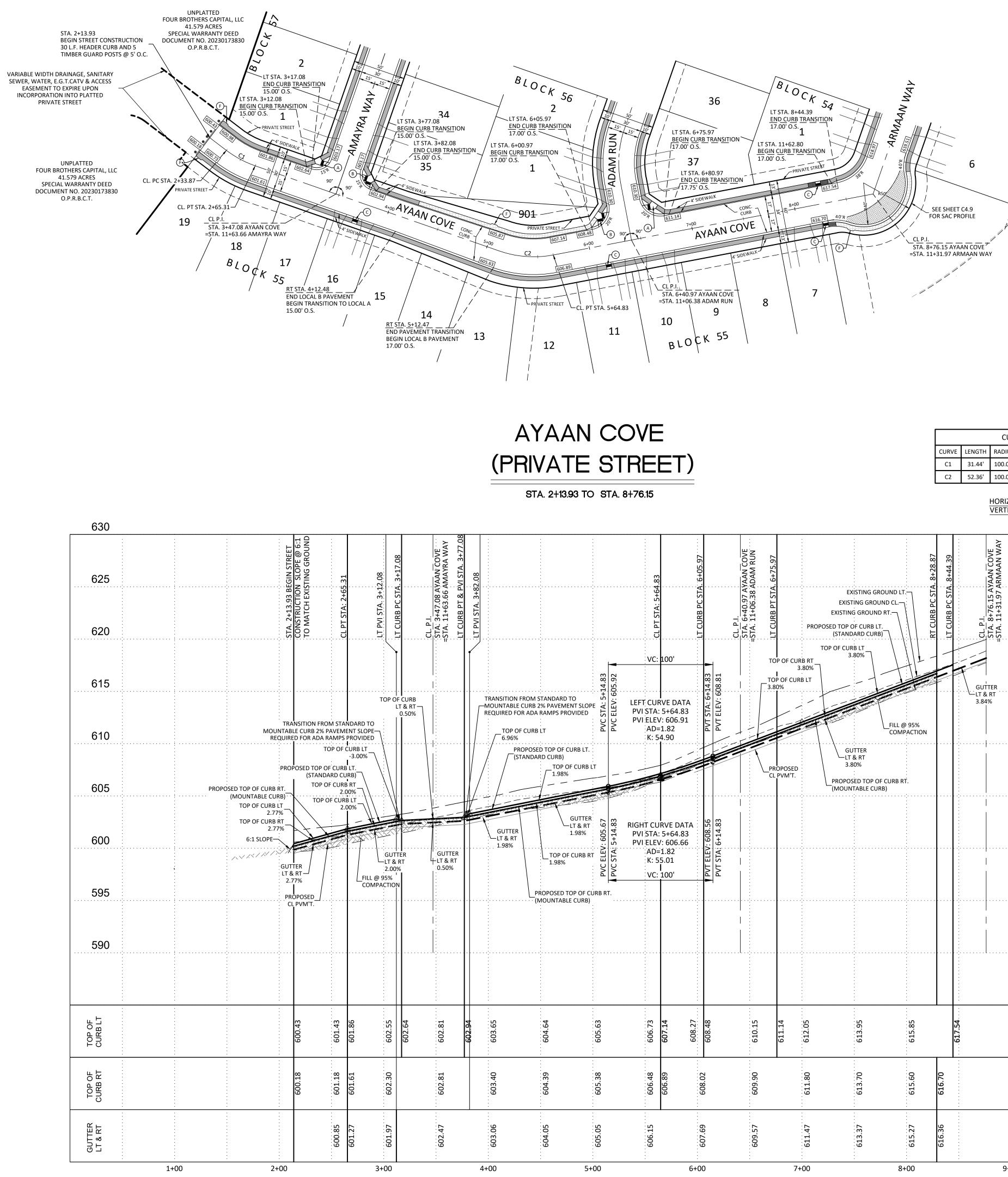
e: April 4, 2024 User ID: Samuel Garcia witz D\Unit 1\Drawings\23148\_C4.12-Kiana Way

	PLAT NO. 23-118	00388	T	BҮ		Ц.
		-				<ul> <li>∴ DATE:</li> </ul>
	CONTRACTOR TO TIE EXISTING AND PROPOSED CURB/SIDEWALK. PRIOR TO CONSTRUCTION CONTRACTOR SHALL VERIFY ELEVATIONS.	1				СНКО. ВҮ:
SCALE: 1"=50'	SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (SINGLE)	۸				DWN. BY:
0 50 100	SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (DUAL)	B	S	DESCRIPTION		BY: DI
	SIDEWALK WHEELCHAIR RAMP - TYPE II (DEVELOPER INSTALLED)	©	REVISIONS	DESCI		DGN.
	SIDEWALK WHEELCHAIR RAMP - TYPE I (DEVELOPER INSTALLED)	D	8			PROJ. #
	SIDEWALK WHEELCHAIR RAMP - TYPE 11 (DEVELOPER INSTALLED) OFFSET PARALLEL RAMP	E				ď
	SIDEWALK PASSING SPACE	F				
UNPLATTED FOUR BROTHERS CAPITAL, LLC	EXISTING TOP OF CURB ELEVATION	805.81TC		DATE		
41.579 ACRES SPECIAL WARRANTY DEED	PROPOSED TOP OF CURB ELEVATION	805.81		N		
DOCUMENT NO. 20230173830 O.P.R.B.C.T.	HOME BUILDER INSTALLED SIDEWALK			Z		
VARIABLE WIDTH DRAINAGE, SANITARY SEWER, WATER, E.G.T.CATV & ACCESS EASEMENT TO EXPIRE UPON INCORPORATION INTO PLATTED PRIVATE STREET	DEVELOPER INSTALLED SIDEWALK					0
	SIDEWALK WHEEL CHAIR RAMP (DEVELOPER INSTALLED)	000000000000000000000000000000000000000		Engineers Surveyors	Planners ers, LLC	-10131500 698-5051 698-5085
	WASH-OUT CROWN			• Eng • Sur		. F-
	POSSIBLE DRIVEWAY LOCATION				Engine	/SURVEYING 00 TEL: (2 FAX: (21
ITAL, LLC					irez	297, TE 1
S Y DEED		- — — 1120 — — — — — — — — — — — — — — — — — — —			Rami	G F-52 .TH, SUI <sup>-</sup> S 78249
0173830	PROPOSED CONTOUR					ENGINEERING IMARRON PAT FONIO, TEXAS
	FLOW ARROW	_ <b>-</b> ►			Tarin	ENGINEEI IMARRON FONIO, TE
	COSA CITY LIMIT LINE				'   <b>\</b>	TBPELS: E 12770 CII SAN ANTO
-						E ,
35				4	Dell.	Hal
30	CAUTION!!! CONTRACTOR TO VERIFY EXISTIN CONDITIONS BEFORE CONSTRUCT IF ANY DISCREPANCIES NOTIFY ENGINEER	G ION.		PROX.	LANDA, IOO182	ALL
25					IONAL EN	
20						
15				E UNIT 1		XEEI)
10					ROI	
05				AT GREEN ENCLAVE UNIT 1	STREET PLAN & PROFILE	KIANA WAY (PKIVAIE SIKEEI, STA. 1+00.00 TO STA. 7+16.00
00				T GRE		<b>VAY (H</b> TA. 1+00.00
95				LEGACY A	TREE	
					S S	KI
CURB LT						
TOP OF CURB RT						

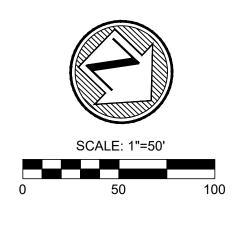
SUBMITTAL	SET

C4.12

SHEET



630					
625			ETA 2412 02 DECINI CTDEET	TO MATCH EXISTING GROUND	+65.31
620			CTA 2.42	CONSTRUCT TO MATCH E	CL PT STA: 2+65.31
615					· · · · · · · ·
610			. MOUNTA	ANSITION FROM BLE CURB 2% PA RED FOR ADA RA	AVEMEN
605		PROPC	PRO SED TOP OF CURB RT (MOUNTABLE CURB) TOP OF CURB LT	DPOSED TOP OF (STANDA TOP OF	CURB L RD CURI CURB R 2.00 F CURB I
600			10P OF CORB ET 2.77% TOP OF CURB RT 2.77% 6:1 SLOPE		2.00
595			GU LT 2	TTER & RT .77% ROPOSED L PVM'T.	· · · · · · · · · · · · · · · · · · ·
590					
					· · · · · ·
TOP OF CURB LT				600.43	601.43 601.86
TOP OF CURB RT				600.18	601.18 601.61
GUTTER LT & RT					600.85 601 27
	1+	00	2+00	•	Į



CURVE TABLE				
DIUS	DELTA	TANGENT	CHORD	
.00'	18°00'56"	15.85'	31.31'	
.00'	30°00'00"	26.79'	51.76'	

IZONTAL SCALE: 1" = 50'
TICAL SCALE: 1" = 5'

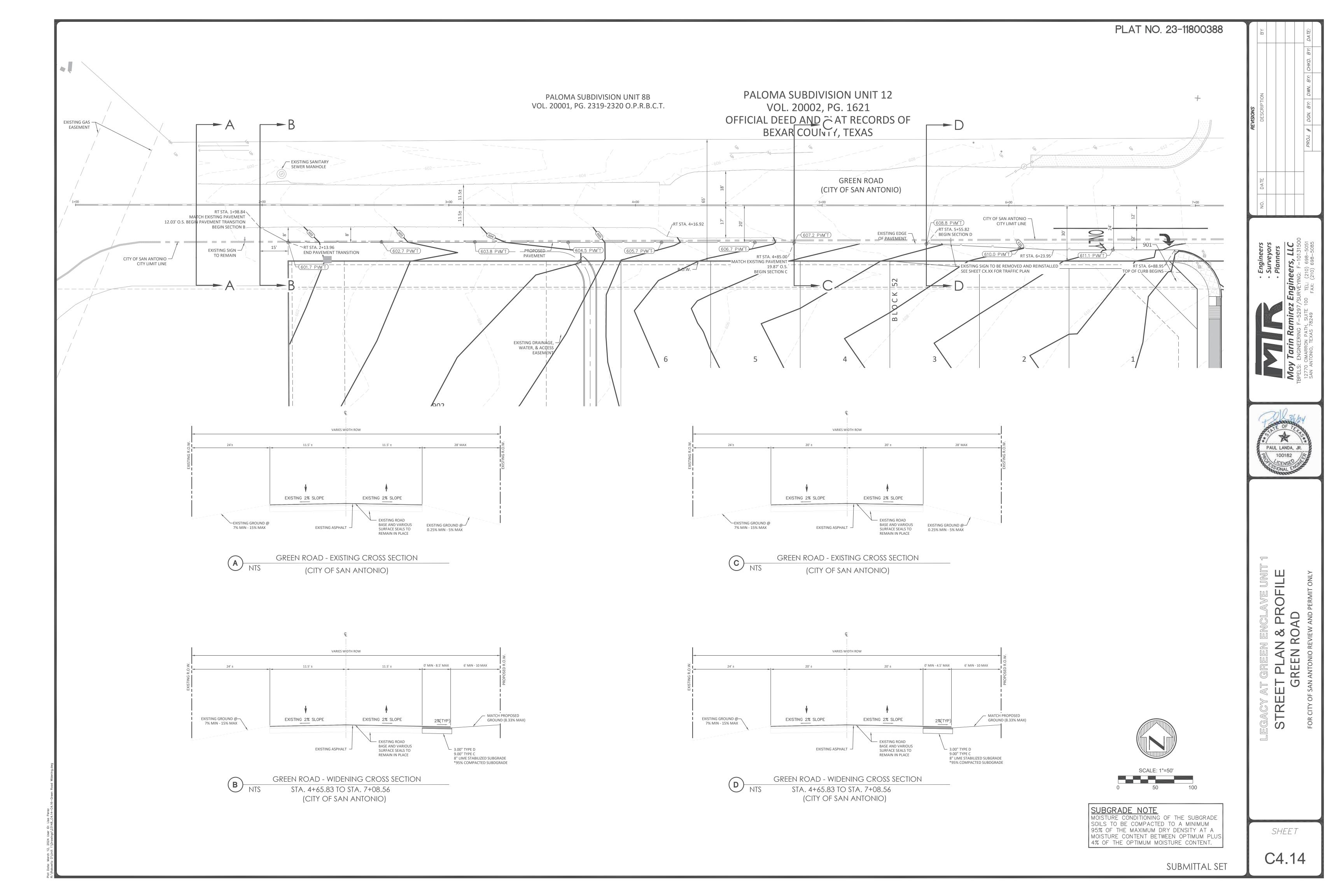
	630
	625
	620
-	615
	610
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	600
	595
	590
	TOP OF CURB LT
	TOP OF CURB RT
	GUTTER LT & RT
9+00	

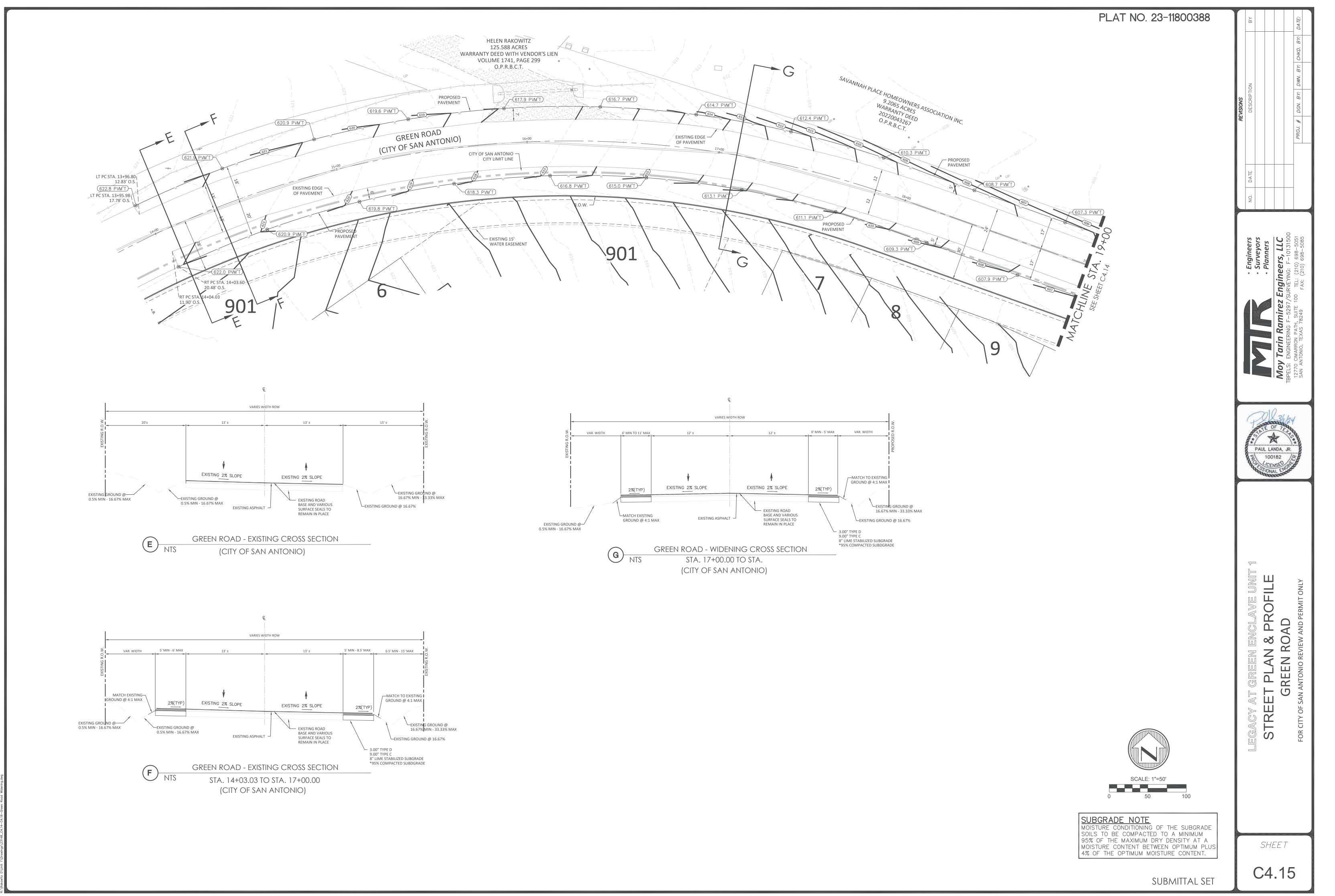
PLAT NO. 23-1180	00388
LEGEND	
CONTRACTOR TO TIE EXISTING AND PROPOSED CURB/SIDEWALK. PRIOR TO CONSTRUCTION CONTRACTOR SHALL VERIFY ELEVATIONS.	1
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (SINGLE)	A
SIDEWALK WHEELCHAIR RAMP - TYPE 10 DIRECTIONAL RAMPS (DUAL)	В
SIDEWALK WHEELCHAIR RAMP - TYPE II (DEVELOPER INSTALLED)	©
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SIDEWALK WHEEL CHAIR RAMP (DEVELOPER INSTALLED)	00000000000000000000000000000000000000
WASH-OUT CROWN	
POSSIBLE DRIVEWAY LOCATION	
PROPERTY LINE	
EXISTING CONTOUR	— — — — — — — — — — — — — — — — — — — —
PROPOSED CONTOUR	1120
PROPOSED CONCRETE CURB	
FLOW ARROW	<b>—</b> ►
COSA CITY LIMIT LINE	

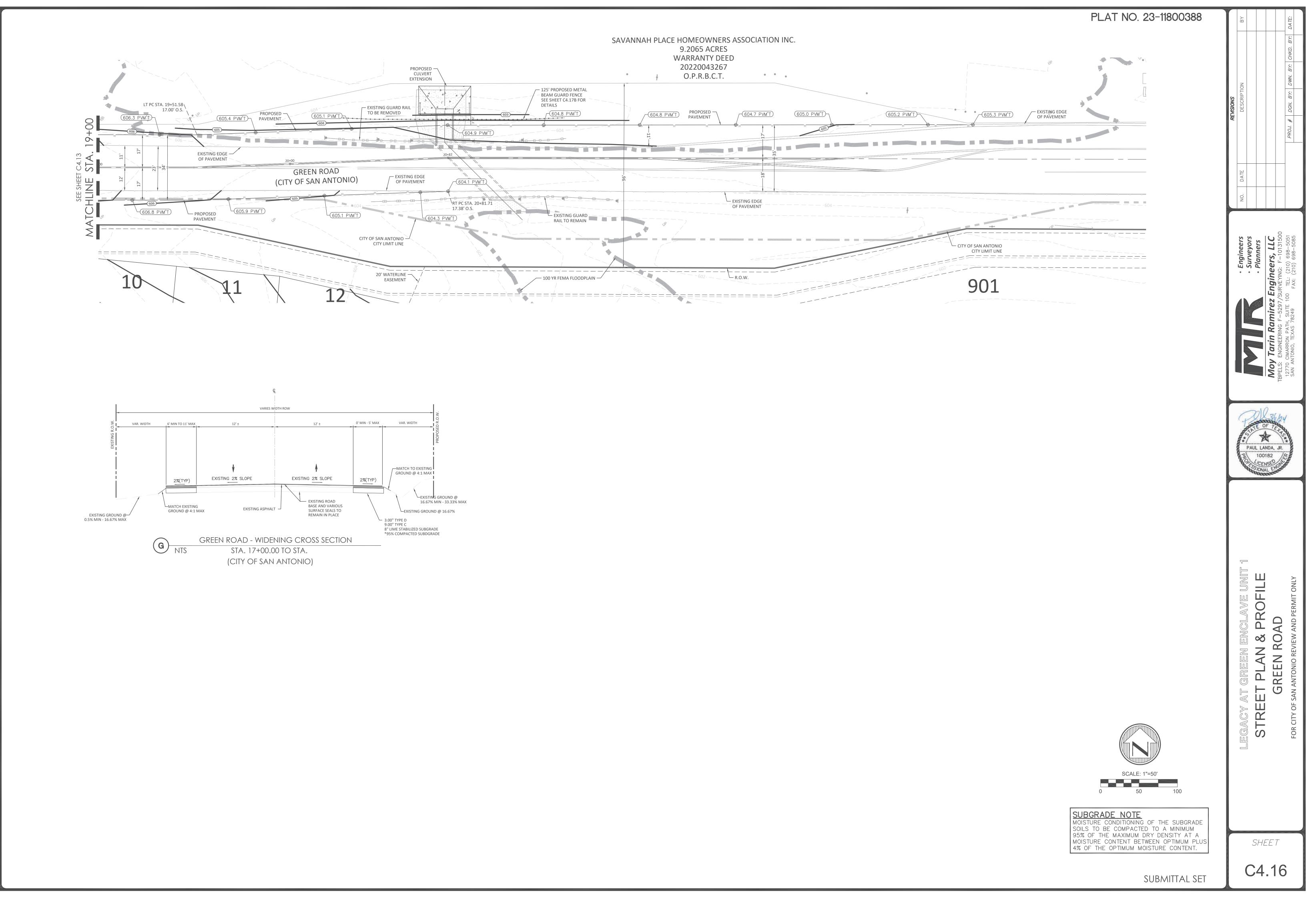


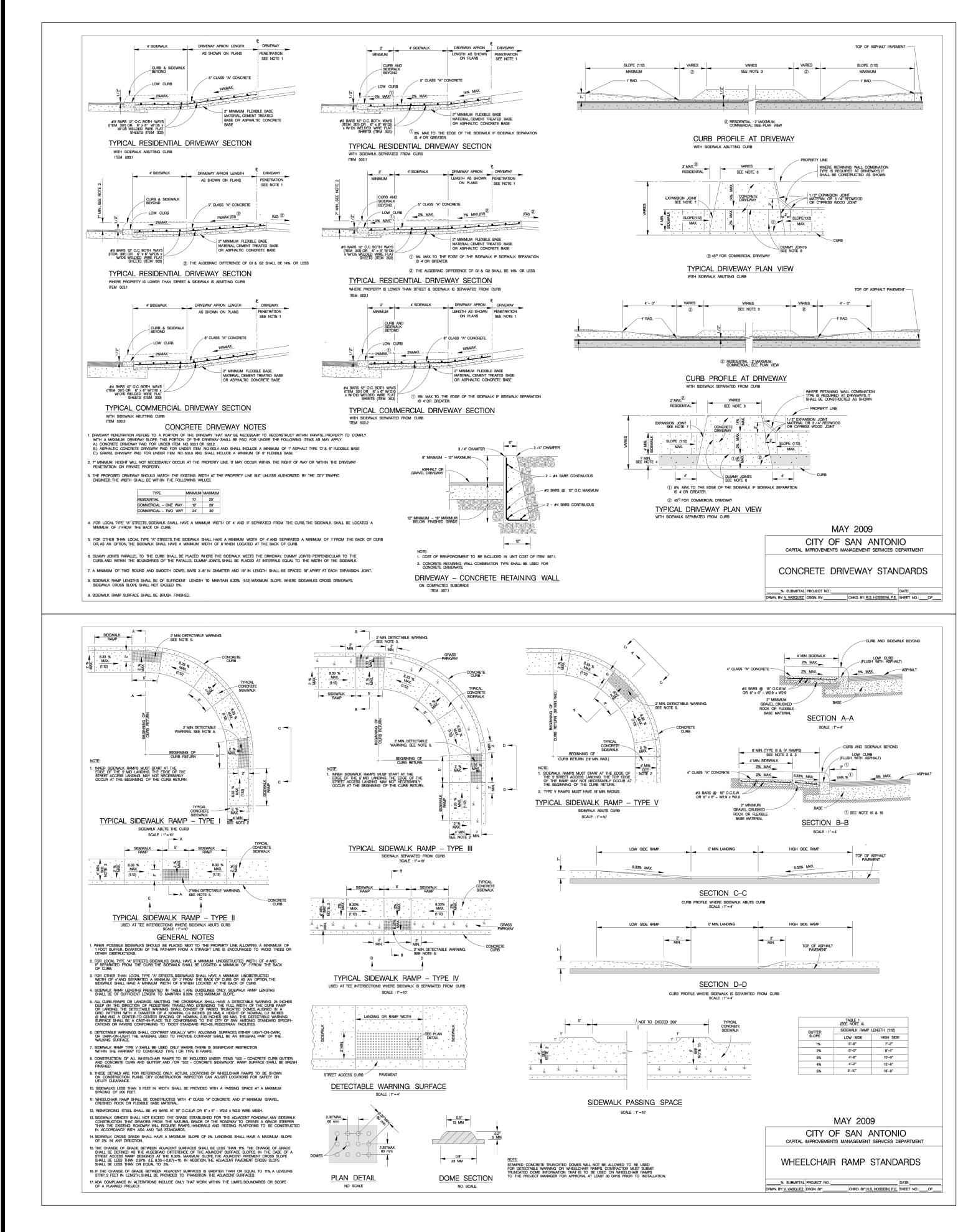
## SUBMITTAL SET

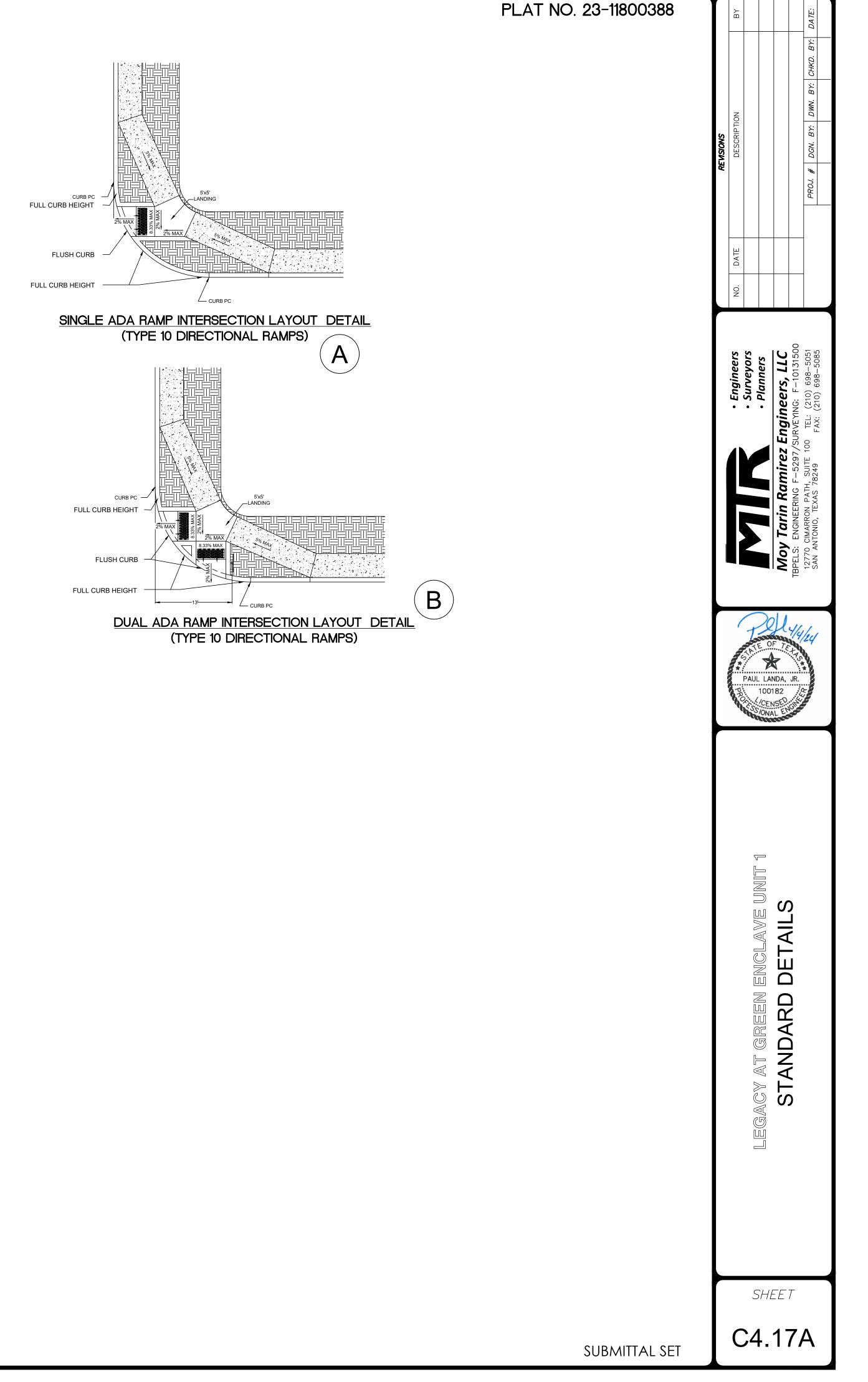
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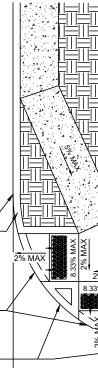


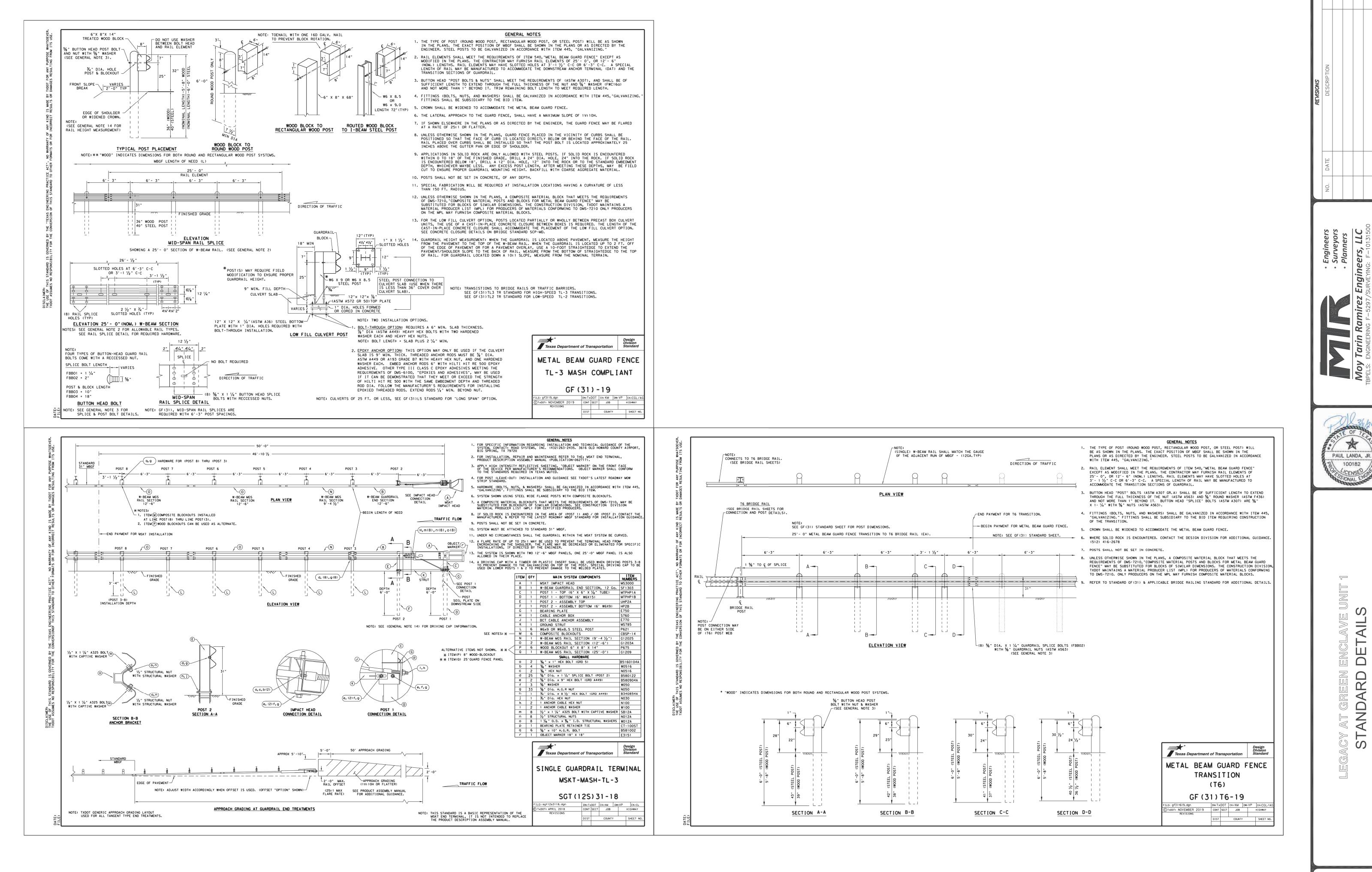












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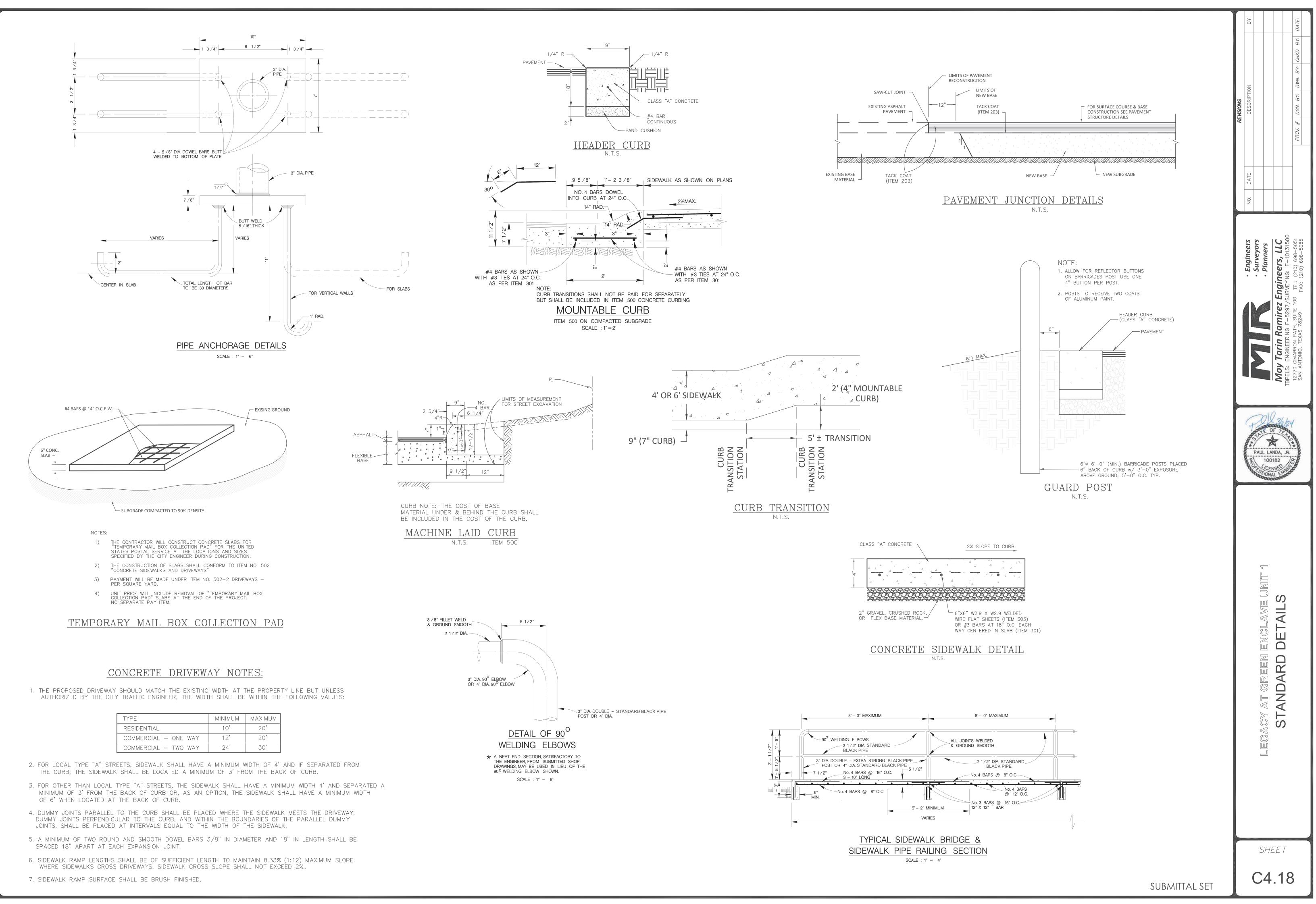
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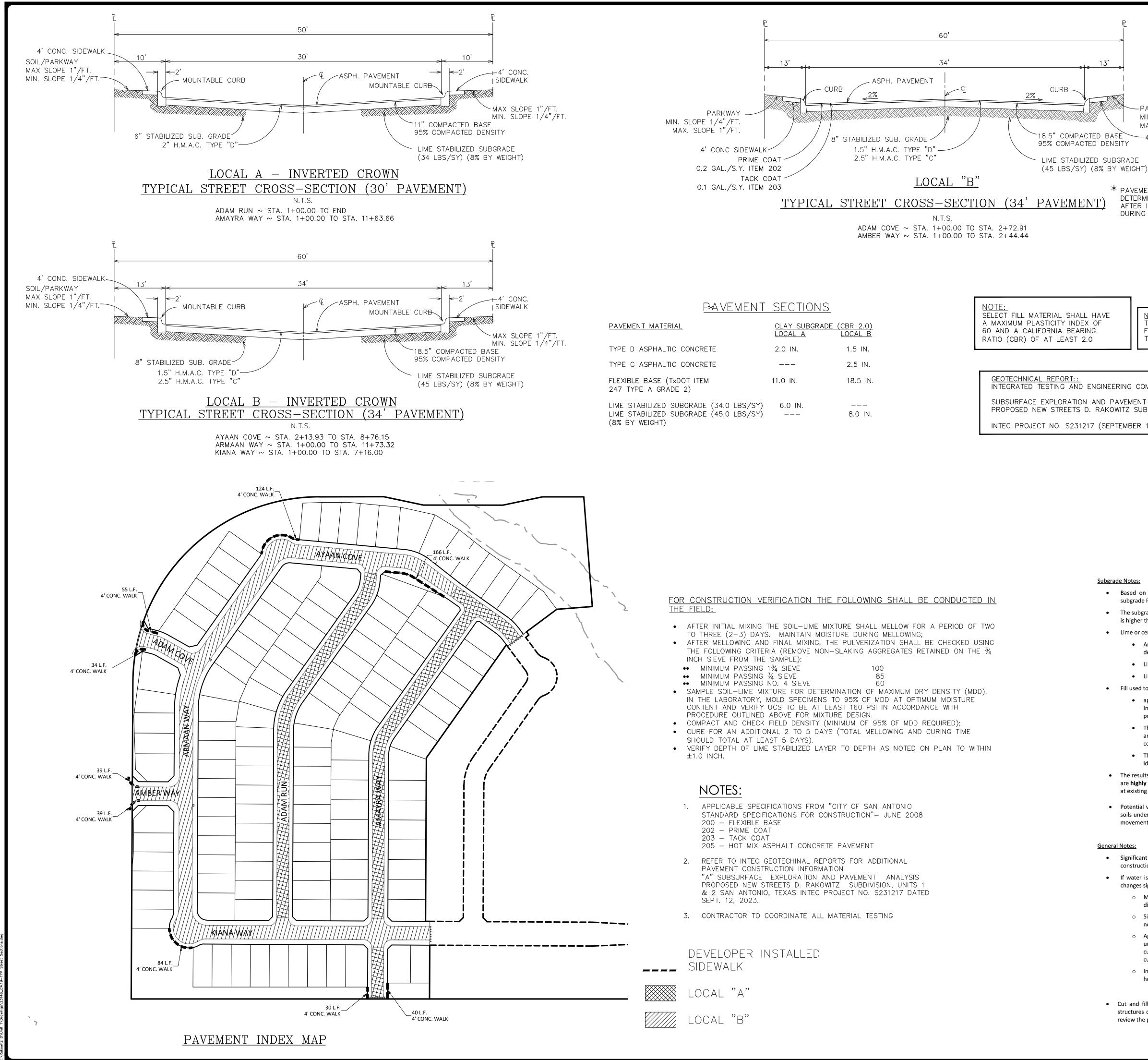
AND,

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SHEET

C4.17B





1.3'

OF

Subgrade Notes:

General Notes:

0.0110				
CONC. EWALK	PAVEMENT	SECTIONS		NOTE: SELECT FILL MATERIAL SHALL F
SLOPE 1"/FT. SLOPE 1/4"/FT.	PAVEMENT MATERIAL	<u>CLAY SUBGRADE</u> LOCAL A	(CBR 2.0) LOCAL B	A MAXIMUM PLASTICITY INDEX 60 AND A CALIFORNIA BEARING RATIO (CBR) OF AT LEAST 2.0
SLOPE 1/4 /FT.	TYPE D ASPHALTIC CONCRETE	2.0 IN.	1.5 IN.	
	TYPE C ASPHALTIC CONCRETE		2.5 IN.	
DE IGHT)	FLEXIBLE BASE (TxDOT ITEM 247 TYPE A GRADE 2)	11.0 IN.	18.5 IN.	GEOTECHNICAL REPORT:: INTEGRATED TESTING AND EN
	LIME STABILIZED SUBGRADE (34.0 LBS/SY) LIME STABILIZED SUBGRADE (45.0 LBS/SY)	6.0 IN.	 8.0 IN.	SUBSURFACE EXPLORATION A PROPOSED NEW STREETS D.
	(8% BY WEIGHT)			INTEC PROJECT NO. S231217

ARKWAY MIN. SLOPE 1/4"/FT. MAX. SLOPE 1"/FT. BASE 4' CONC SIDEWALK SUBGRADE 6 BY WEIGHT) * PAVEMENT SECTION TO BE	<b>REVISIONS</b> DESCRIPTION	PROJ. # DGN. BY: DWN. BY: CHKD.
) DETERMINED BY ENGINEER AFTER INSPECTION OF SUBGRADE DURING CONSTRUCTION.	rs NO. DATE	051 055 055 055 055 055 055 055 055 055
VE NOTE: THE SUBGRADE SOILS SHOULD BE TESTED FOR SOLUBLE SULPHATE CONTENT PRIOR TO INSTALLATION OF LIME OR CEMENT	<ul> <li>Engineers</li> <li>Surveyors</li> </ul>	- 7/SURVEYING 100 TEL: (2 FAX: (21
D PAVEMENT ANALYSIS AKOWITZ SUBDIVISION, UNITS 1 & 2 SEPTEMBER 12, 2023)		Moy Tarin Ramire TBPELS: ENGINEERING F-529 12770 CIMARRON PATH, SUITE SAN ANTONIO, TEXAS 78249
ubgrade Notes:	PA	UL LANDA, JR. 100182
<ul> <li>Based on the thickness of the clays encountered in the borings, we anticipate the final pavement subgrade Plasticity index value to be greater than 20. Subgrade stabilization is recommended.</li> <li>The subgrade soils should be tested for soil sulfate content prior to stabilization. If the soil sulfate content is higher than 3000 ppm an alternate / modified procedure will be needed.</li> <li>Lime or cement may be used to stabilize the subgrade.</li> <li>An application rate of 8 percent lime content. Application rate for cement, if needed, should be determined at the time construction.</li> <li>Lime application rate of 44.0 lbs per sq yard for 6-inch depth of stabilization is recommended.</li> <li>Lime application rate of 45.0 lbs per sq yard for 8-inch depth of stabilization is recommended.</li> <li>Fill used to raise the grade: <ul> <li>approved fill material free should have a minimum CBR value of 2.0 and a maximum Plasticity index value of 60. Lime application rates should be re-evaluated and tested for sulfate content prior to use of the fill material.</li> <li>The fill material should be approved by the geotechnical engineer, free of deleterious material, and the gravel size should not exceed 3 inches in size. The material should be placed and compacted as per applicable (tty / county guidelines.</li> <li>The subgrade, prior to placement of fill, should be proof rolled to identify weak areas. Any identified weak areas should be recompacted.</li> </ul> </li> <li>The results of our laboratory testing and engineering evaluation indicate that the underlying shallow clays are highly plastic in character. Potential vertical movement on the order of 4 ½ to 5 ½ inches is estimated at existing grade elevation.</li> <li>Potential vertical movement on the order of 4 ½ inches is anticipated at the subgrade elevation. If the soils underlying the stabilized is maitcure conditioned to a depth of 18 inches potential vertical movement on the order of 3 inches is anticipated.</li> </ul>	LEGACY AT GREEN ENCLAVE UNIT 1	TYPICAL STREET SECTIONS

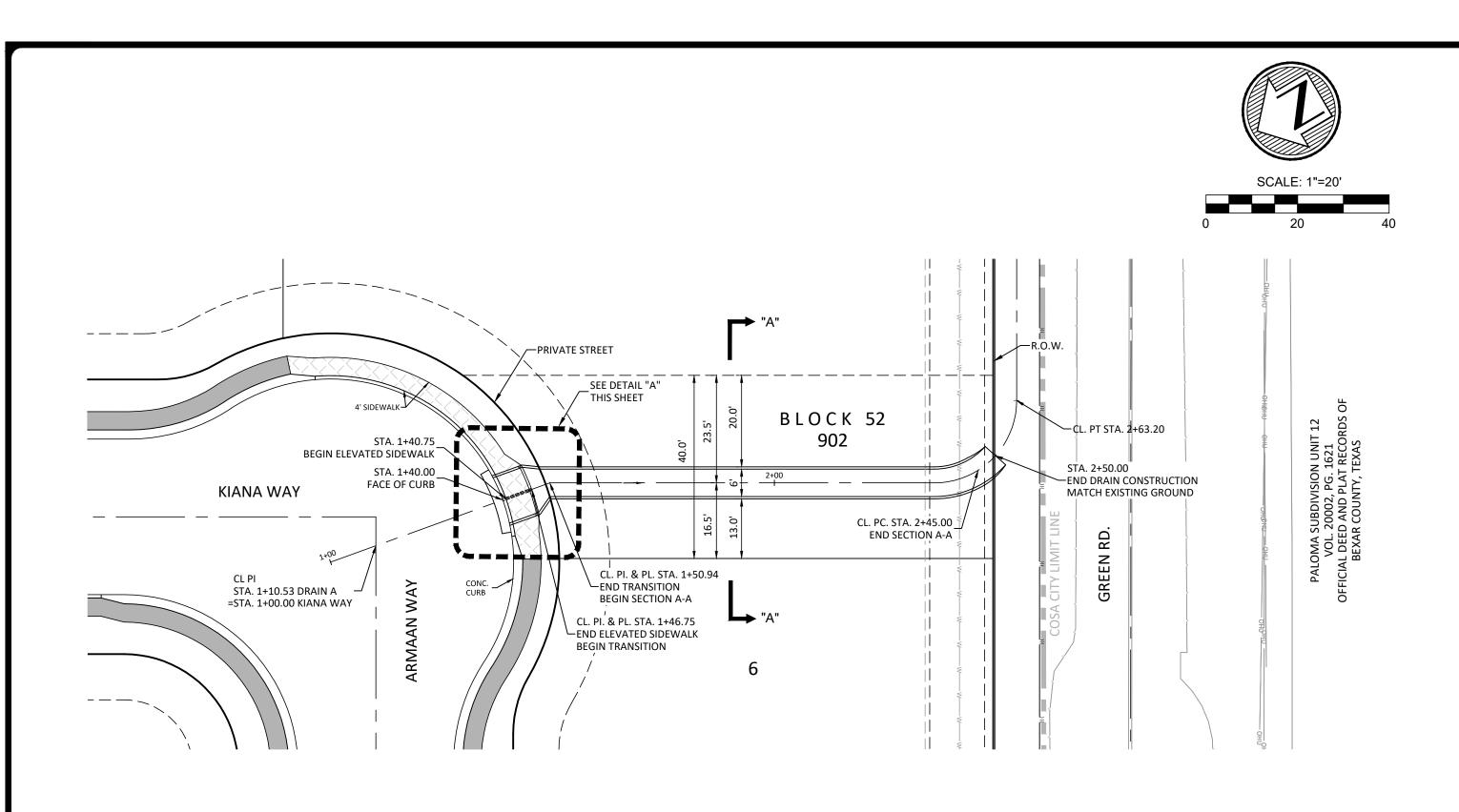
PLAT NO. 23-11800388

- changes sig • Minimizing moisture penetration underneath the asphalt will lower the chances of pavement distress.
- Significant pavement distress, more often caused by water getting underneath the asphalt, is noted during home construction.
- Aggregate base extending beyond the back of the curb increases the likelihood of water getting underneath the asphalt. Moisture penetration may be reduced by using a deeper curb, such as curb extending a minimum of 6 inches into subgrade or compacted clays backfilled against the curbs.
- o In addition, water should not be allowed to get underneath the pavement section at the time of home construction.
- Cut and fill information (street profile) is not available at this time. In addition, information on any structures crossing the street (such as a culvert), is not available at this time. Please contact InTEC to review the proposed street profiles and recommend details for such crossings.

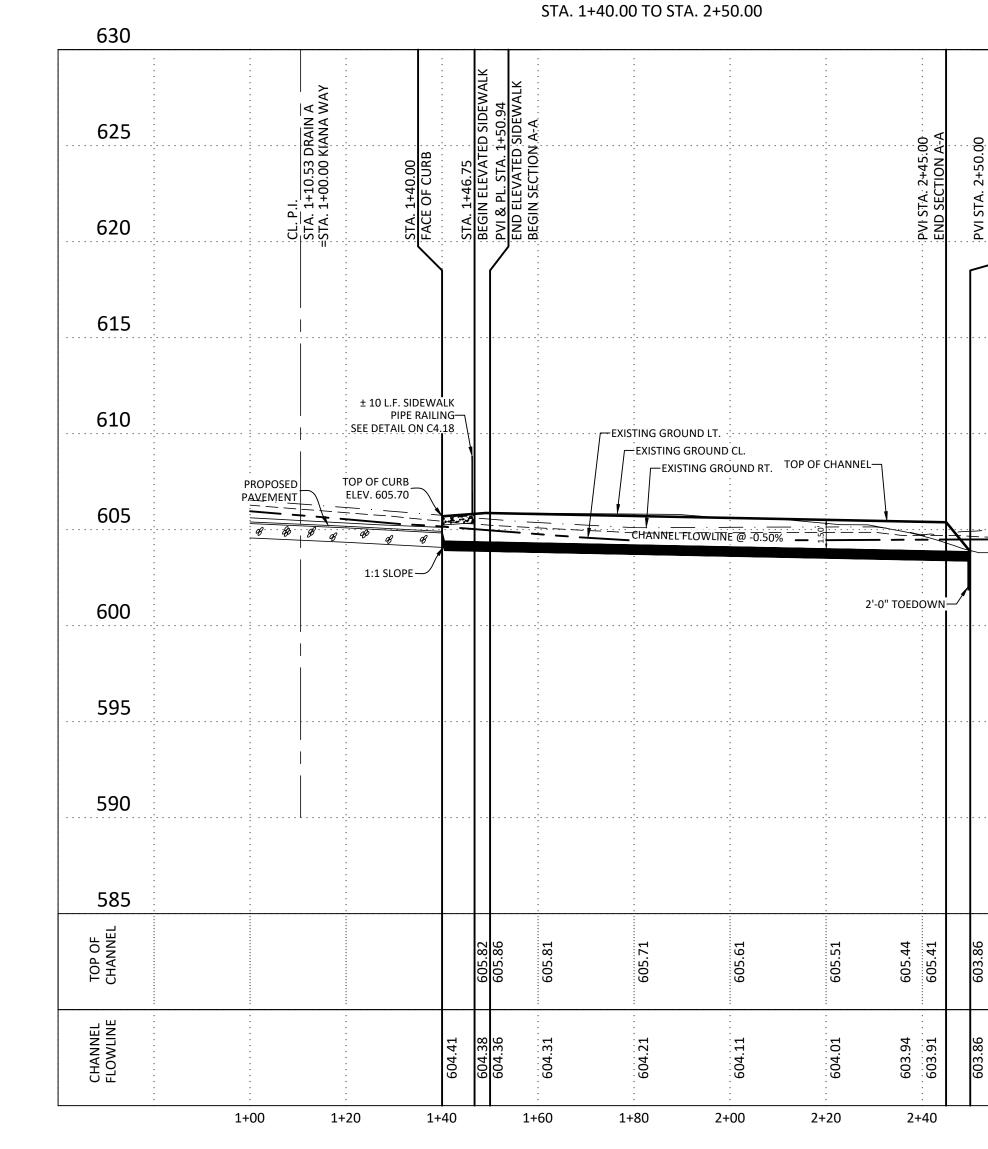
SUBMITTAL SET

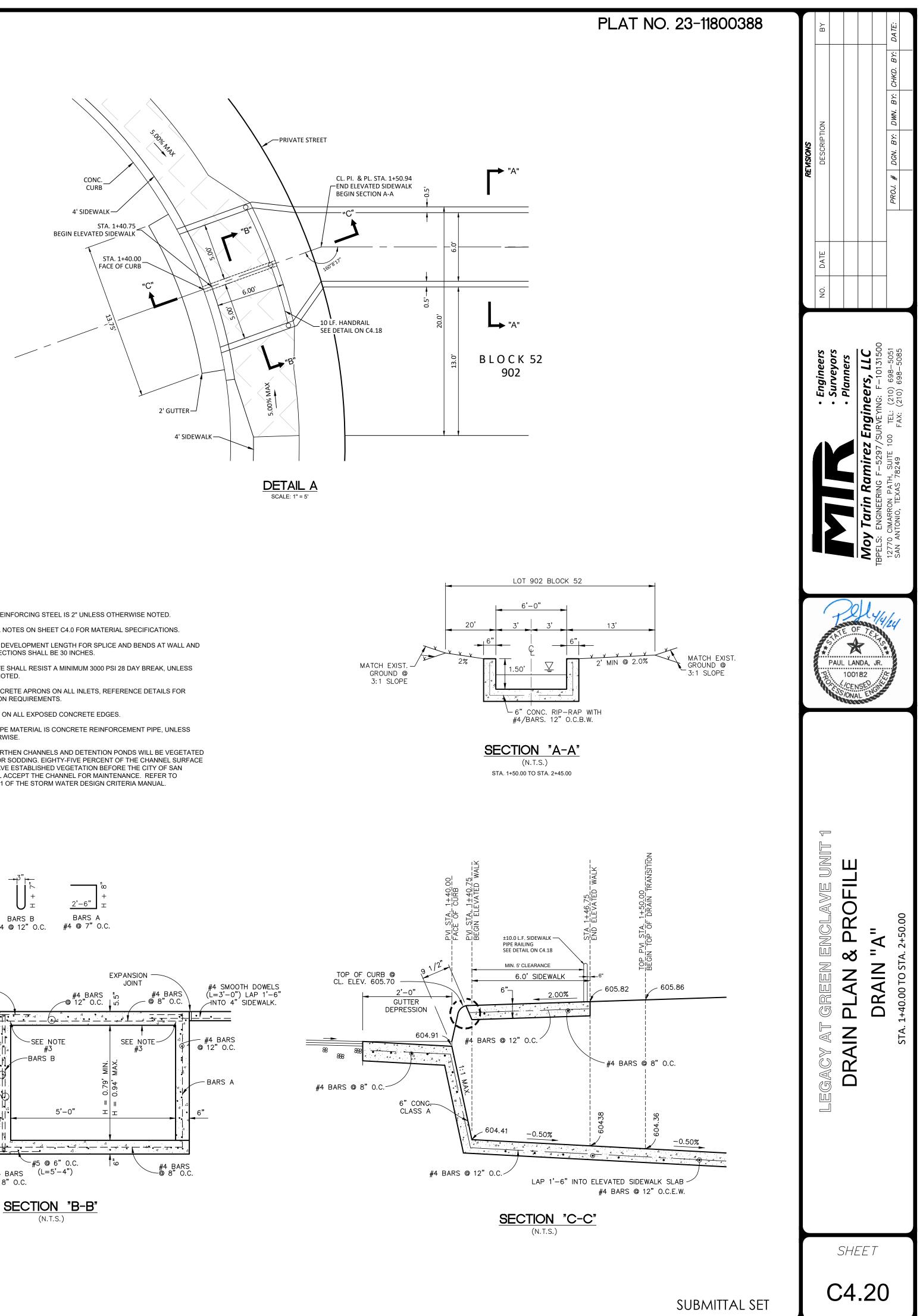
C4.19

SHEET



DRAIN "A"



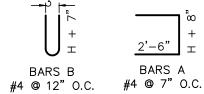


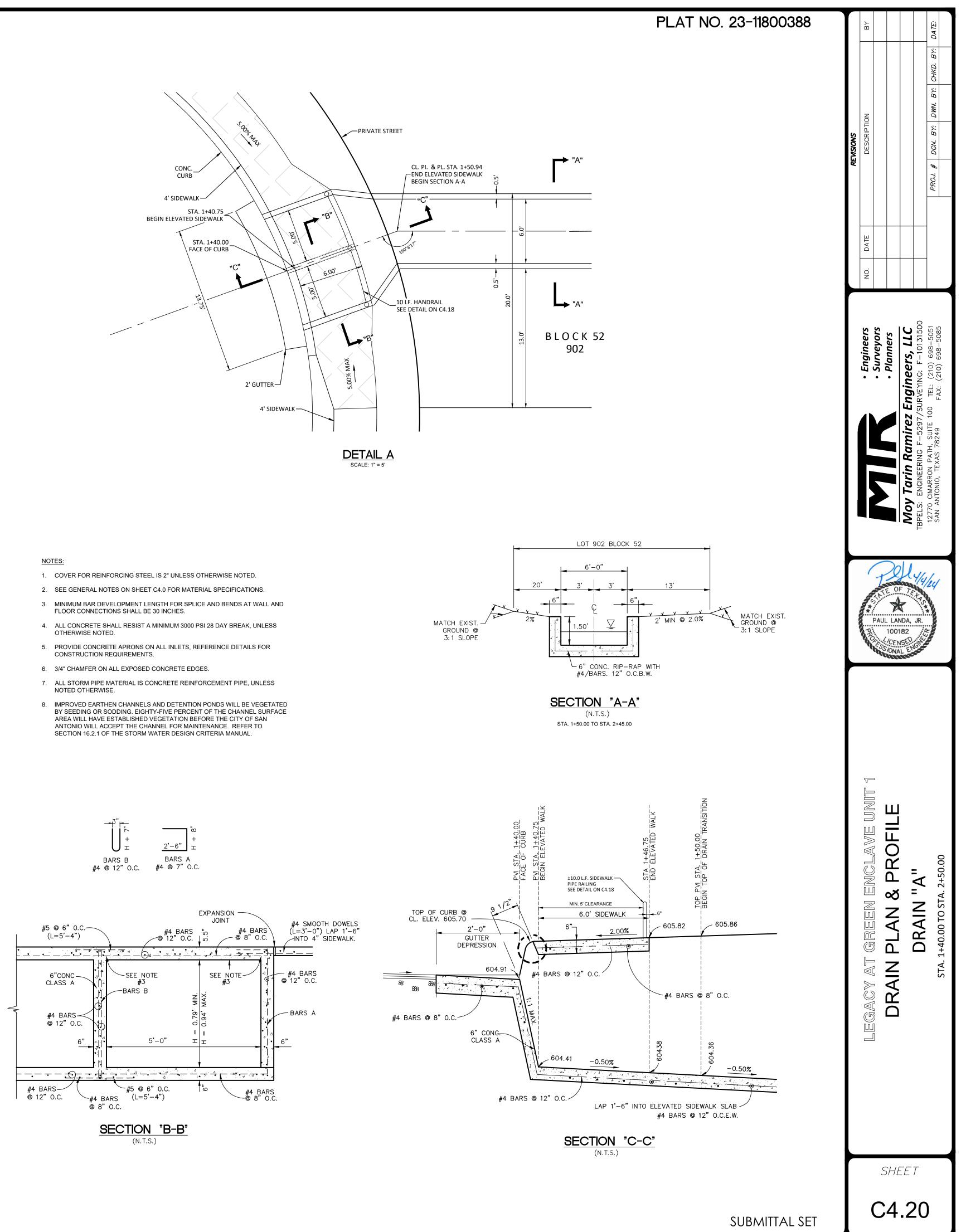
HORIZONTAL SCALE: 1" = 20' VERTICAL SCALE: 1" = 5' 630 625 PVI STA. 2+50.00 END DRAIN CONST MATCH EXISTING ( 620 615 610 PROPOSED CL. 605 \_\_\_\_ 600 595 590 585 TOP OF CHANNE ЧЧ CHANN FLOWL

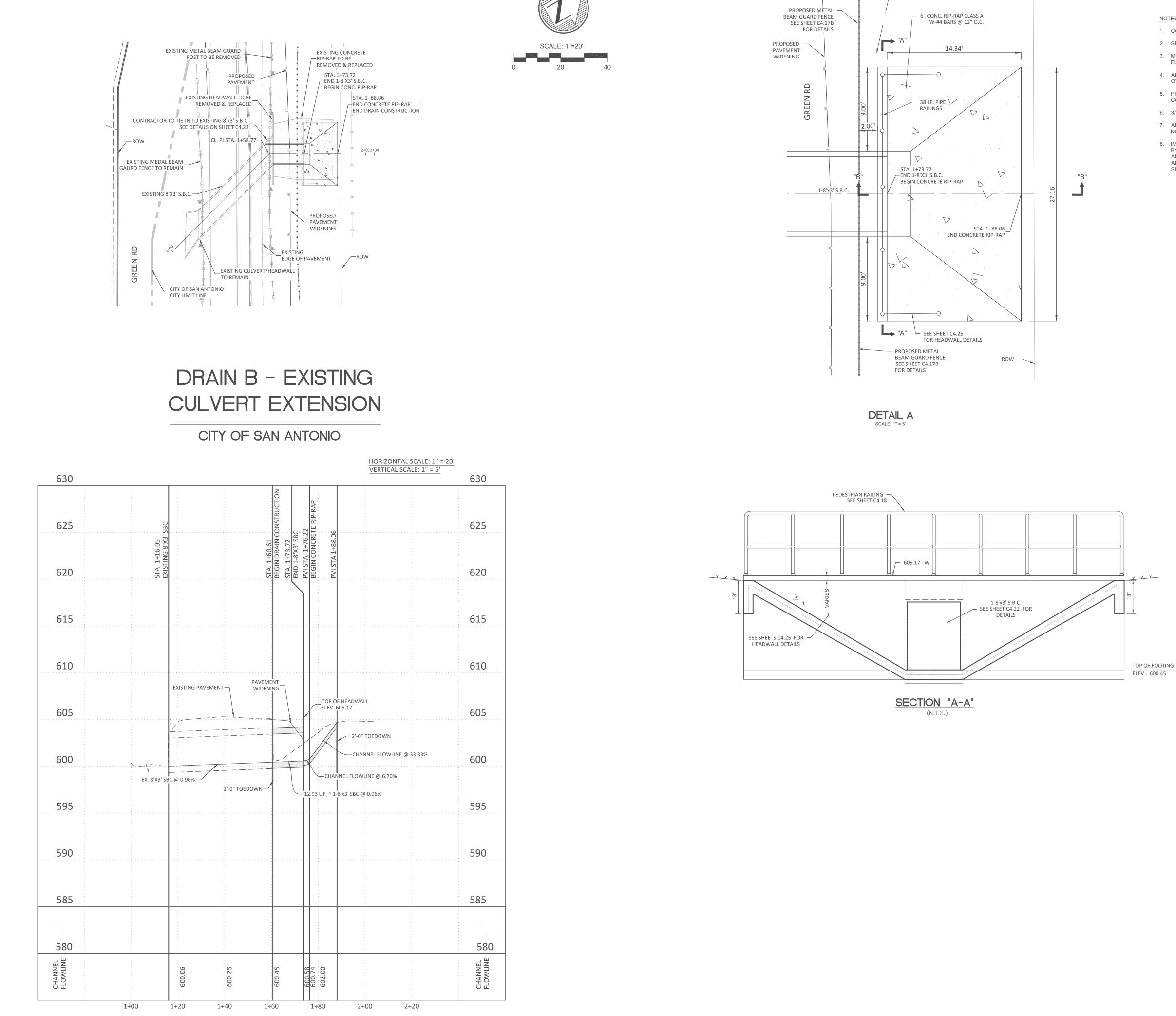
2+60

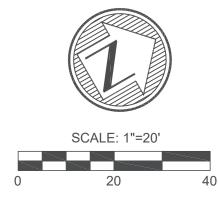
2+80

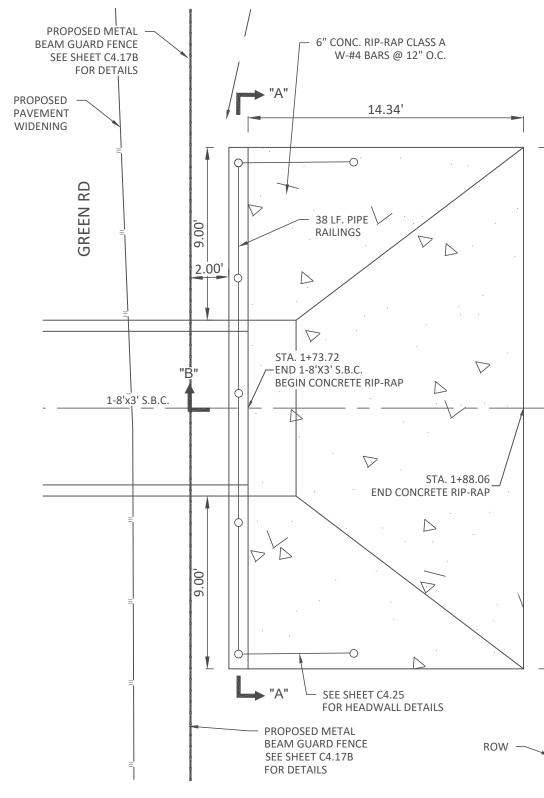
- AREA WILL HAVE ESTABLISHED VEGETATION BEFORE THE CITY OF SAN ANTONIO WILL ACCEPT THE CHANNEL FOR MAINTENANCE. REFER TO









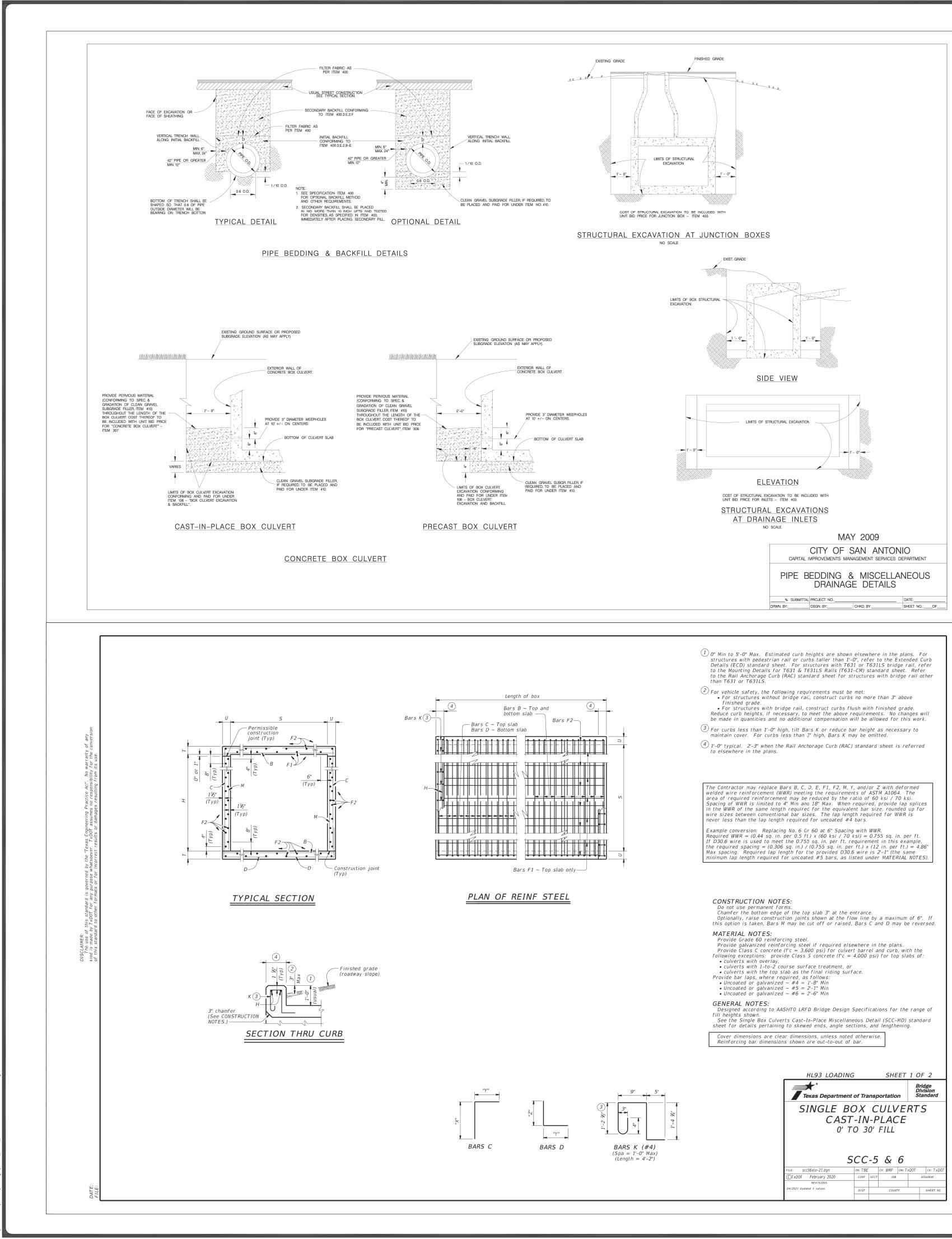






- 1. COVER FOR REINFORCING STEEL IS 2" UNLESS OTHERWISE NOTED.
- 2. SEE GENERAL NOTES ON SHEET C4.0 FOR MATERIAL SPECIFICATIONS. 3. MINIMUM BAR DEVELOPMENT LENGTH FOR SPLICE AND BENDS AT WALL AND FLOOR CONNECTIONS SHALL BE 30 INCHES.
- 4. ALL CONCRETE SHALL RESIST A MINIMUM 3000 PSI 28 DAY BREAK, UNLESS OTHERWISE NOTED.
- 5. PROVIDE CONCRETE APRONS ON ALL INLETS, REFERENCE DETAILS FOR CONSTRUCTION REQUIREMENTS.
- 6. 3/4" CHAMFER ON ALL EXPOSED CONCRETE EDGES.
- 7. ALL STORM PIPE MATERIAL IS CONCRETE REINFORCEMENT PIPE, UNLESS NOTED OTHERWISE.
- 8. IMPROVED EARTHEN CHANNELS AND DETENTION PONDS WILL BE VEGETATED BY SEEDING OR SODDING. EIGHTY-FIVE PERCENT OF THE CHANNEL SURFACE AREA WILL HAVE ESTABLISHED VEGETATION BEFORE THE CITY OF SAN ANTONIO WILL ACCEPT THE CHANNEL FOR MAINTENANCE. REFER TO SECTION 16.2.1 OF THE STORM WATER DESIGN CRITERIA MANUAL.

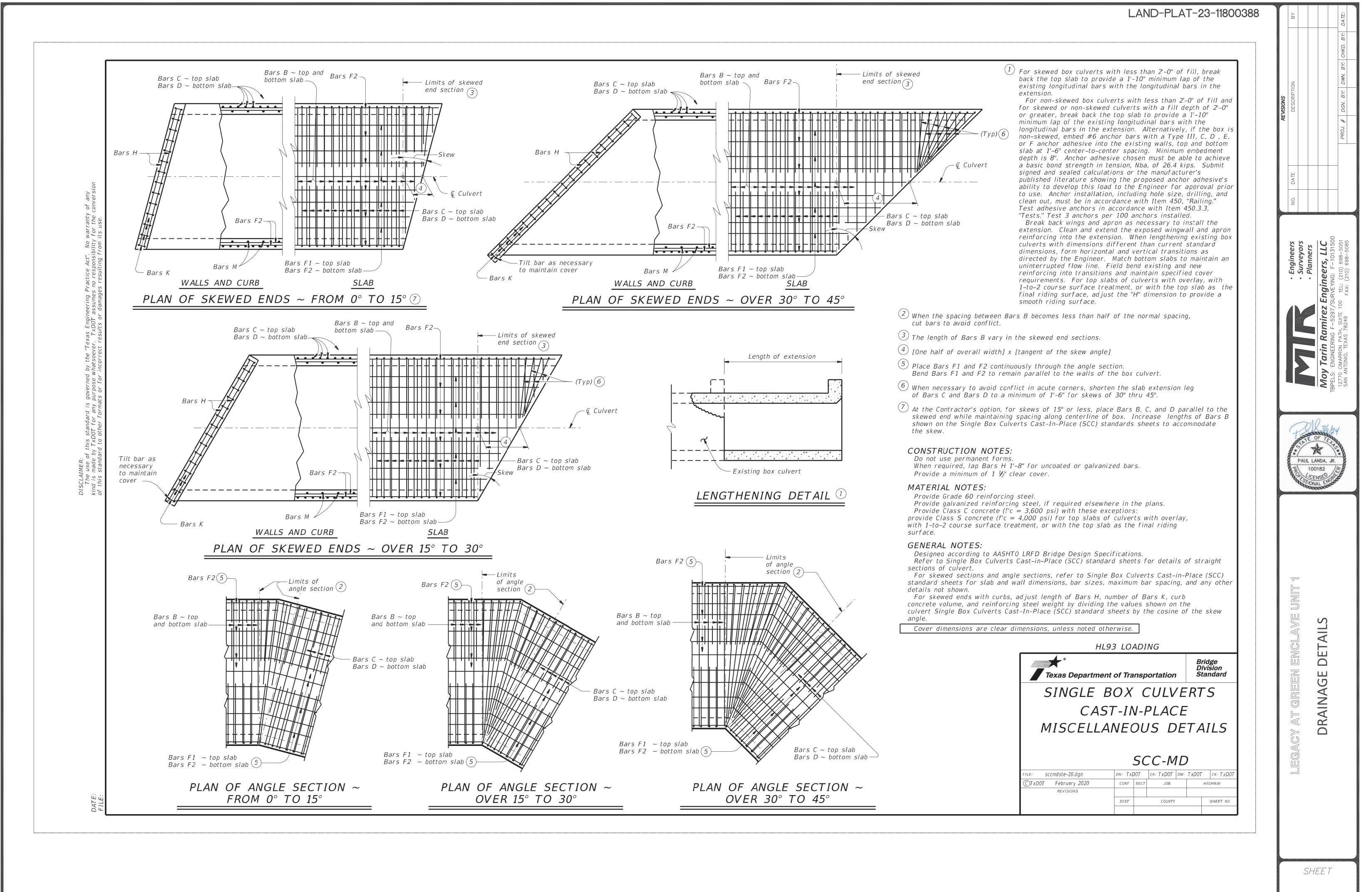
REUSIONS	NO. DATE DESCRIPTION BY			PROJ. # DGN. BY: DWN. BY: CHKD. BY: DATE:
	Engineers     Surveyors	• Planners	Moy Tarin Ramirez Engineers, LLC TBPELS: ENGINEERING F-5297/SURVEYING: F-10131500	12770 CIMARRON PATH, SUITE 100 TEL: (210) 698–5051 San Antonio, Texas 78249 Fax: (210) 698–5085
	Tron Prost			
	LEGACY AT GREEN ENCLAVE UNIT 1	DRAIN PLAN & PROFILE	DRAIN B - EXISTING CULVERT EXTENSION	FOR CITY OF SAN ANTONIO REVIEW AND PERMIT ONLY
	(	<i>SHE</i>	ет .21	



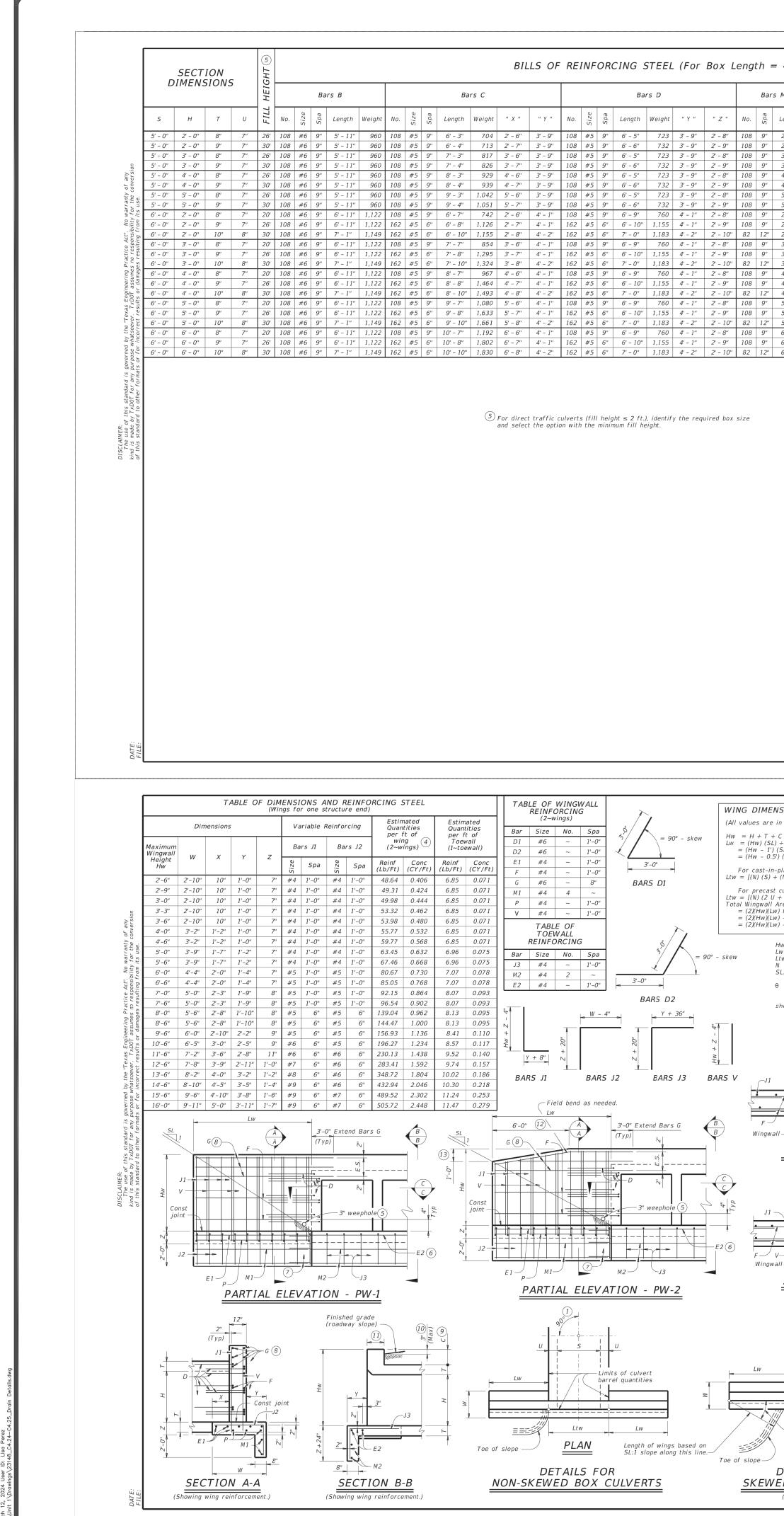
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REVISIONS	NO. DATE DESCRIPTION BY				PROJ. # DGN. BY: DWN. BY: CHKD. BY: DATE:	
	Engineers     Surveyors	Planners	Moy Tarin Ramirez Engineers, LLC	TBPELS: ENGINEERING F-5297/SURVEYING: F-10131500	12770 CIMARRON PATH, SUITE 100 TEL: (210) 698-5051 San Antonio, texas 78249	
	PAU		IDA. 82 SEO	124 TS R LAND		
	LEGACY AT GREEN ENCLAVE UNIT 1		DRAINAGE DETAILS			
		5HE 5 <b>4</b>			)	

# LAND-PLAT-23-11800388



C4.23

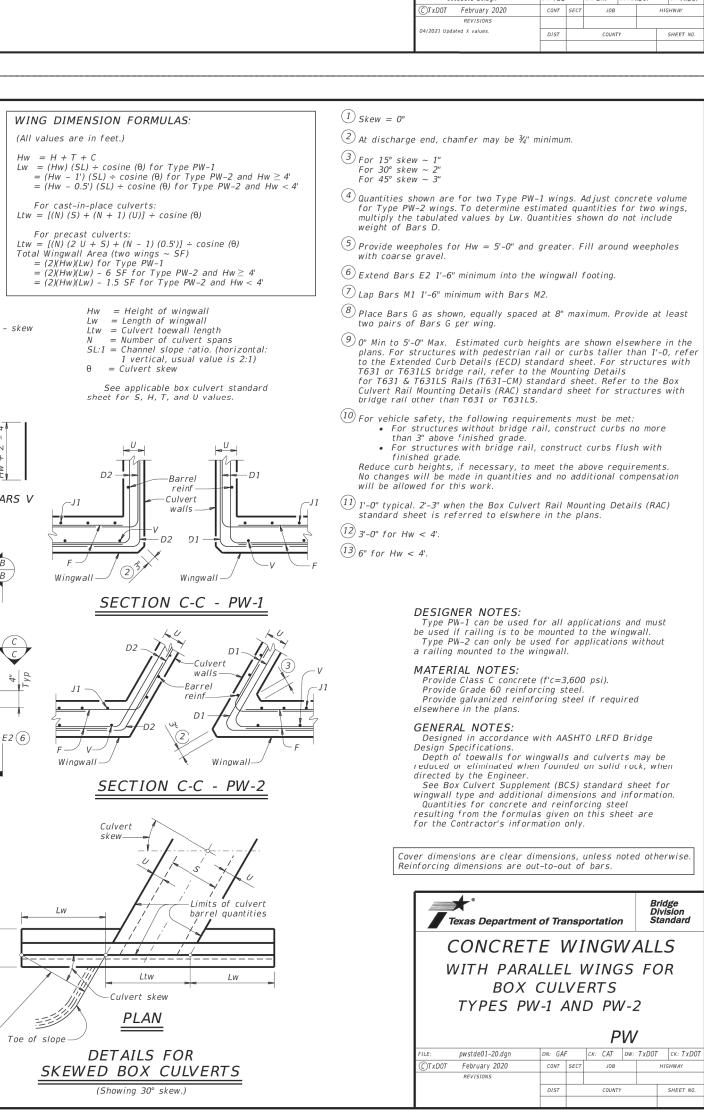


Length       Weight       No.       Length       Weight       Length       Wt       No.       Wt       No.       Wt       (CY)       (Lb)		40 f	eet)												QU	'AN1		-5	
Length         We         No.         Length         We         Length         We         No.         We         (CY)         (Lb)         (CY)         (Lb	rs	M ~ #4	4	Ba	ars F1 ~ at 18" Sp	#4 a	В	ars F2 ~ at 18" S <sub>1</sub>	/ #4 pa			Bar	s K			Cu	rb	То	tal
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$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		2' - 0''	144	4	39' - 9''	106	22	39' - 9''	584	5' - 11''	16	14	39	0.391	80.5	0.5	55	16.1	3,27
3' - 0'' $216$ $4$ $39' - 9''$ $106$ $26$ $39' - 9''$ $690$ $5' - 11''$ $16$ $14$ $39$ $0.472$ $88.3$ $0.5$ $55$ $19.3$ $3.56$ $4' - 0''$ $289$ $4$ $39 - 9''$ $106$ $26$ $39 - 9''$ $690$ $5' - 11''$ $16$ $14$ $39$ $0.477$ $92.4$ $0.5$ $55$ $19.5$ $3.75$ $4' - 0''$ $289$ $4$ $39 - 9''$ $106$ $26$ $39' - 9''$ $690$ $5' - 11''$ $16$ $14$ $39$ $0.515$ $92.9$ $0.5$ $55$ $21.1$ $3.77$ $5' - 0''$ $361$ $4$ $39' - 9''$ $106$ $30$ $39' - 9''$ $797$ $5' - 11''$ $16$ $14$ $39$ $0.521$ $99.7$ $0.5$ $55$ $21.3$ $4,04$ $5' - 0''$ $361$ $4$ $39' - 9''$ $106$ $30$ $39' - 9''$ $797$ $5' - 11''$ $16$ $14$ $39$ $0.521$ $99.7$ $0.5$ $55$ $21.3$ $4,04$ $5' - 0''$ $361$ $4$ $39' - 9''$ $133$ $25$ $39' - 9''$ $664$ $6' - 11''$ $18$ $16$ $45$ $0.440$ $89.1$ $0.5$ $63$ $18.1$ $362$ $2' - 0''$ $1144$ $5$ $39' - 9''$ $133$ $25$ $39' - 9''$ $664$ $7' - 1''$ $19$ $18$ $50$ $0.551$ $10.9$ $0.5$ $63$ $18.1$ $362$ $2' - 0''$ $110$ $5$ $39' - 9''$ <td></td> <td>2' - 0''</td> <td>144</td> <td>4</td> <td>39' - 9''</td> <td>106</td> <td>22</td> <td>39' - 9''</td> <td>584</td> <td>5' - 11''</td> <td>16</td> <td>14</td> <td>39</td> <td>0.429</td> <td>81.0</td> <td>0.5</td> <td>55</td> <td>17.6</td> <td>3,29</td>		2' - 0''	144	4	39' - 9''	106	22	39' - 9''	584	5' - 11''	16	14	39	0.429	81.0	0.5	55	17.6	3,29
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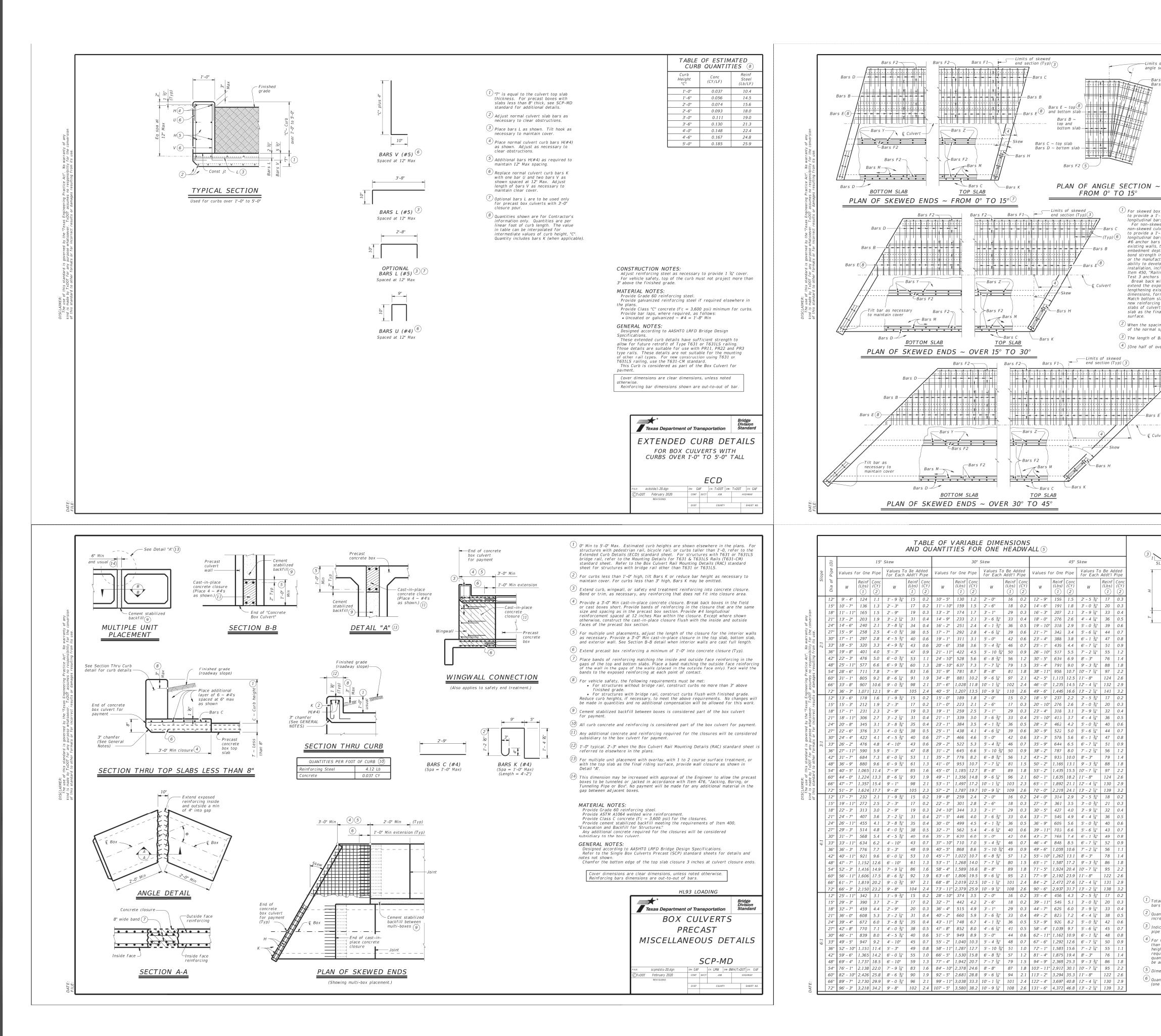
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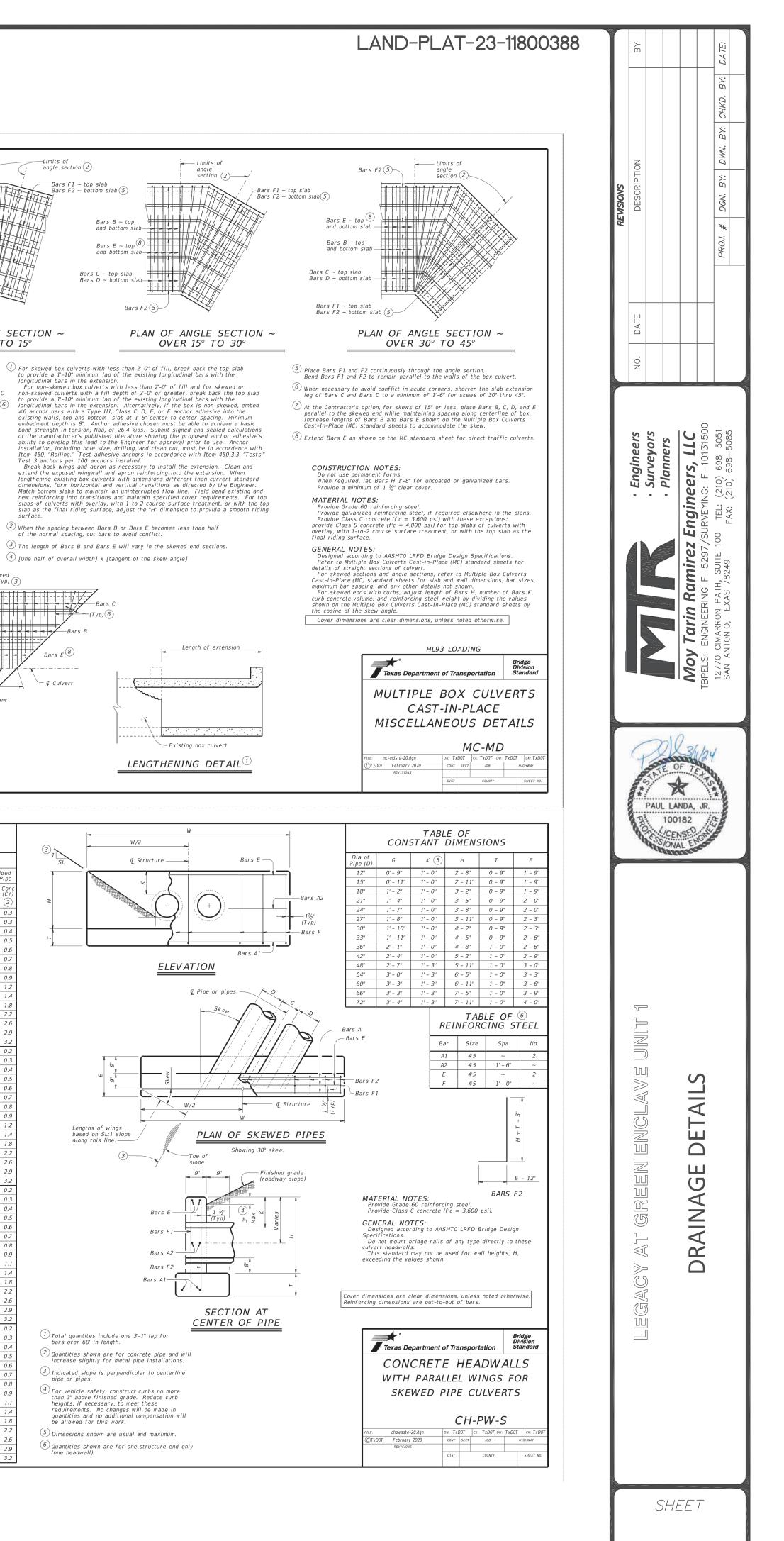


REVISIONS	NO. DATE DESCRIPTION BY		PROJ. # DGN. BY: DWN. BY: CHKD. BY: DATE:
	Engineers     Surveyors	Moy Tarin Ramirez Engineers, LLC TBPELS: ENGINEERING F-5297/SURVEYING: F-10131500	12770 CIMARRON PATH, SUITE 100 TEL: (210) 698–5051 SAN ANTONIO, TEXAS 78249 FAX: (210) 698–5085
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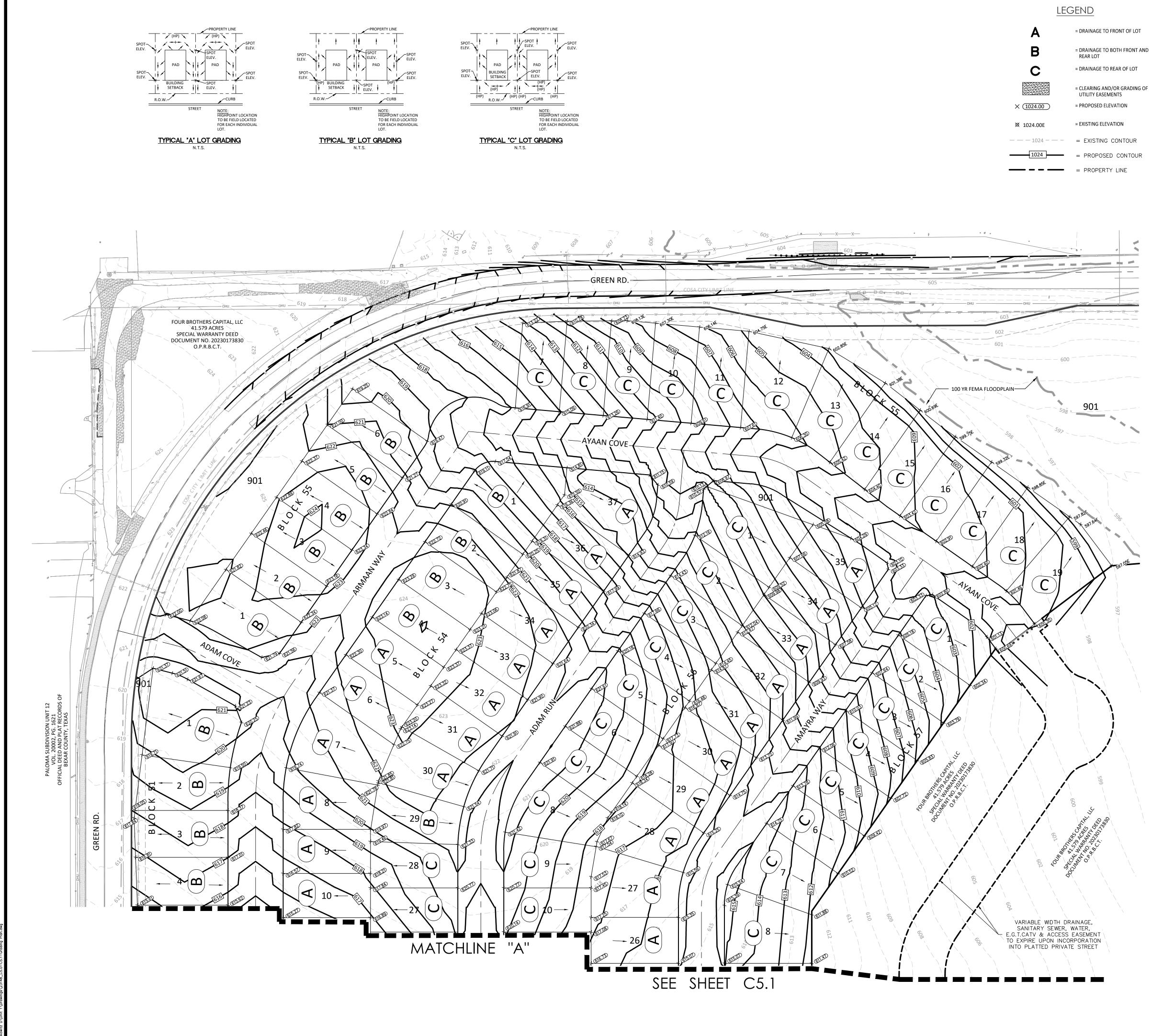
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# LAND-PLAT-23-11800388





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ELEVATIONS IF NECESSARY

# PLAT NO. 23-11800388

# GENERAL SPECIFICATIONS FOR SITE PREPARATION

## 1. GENERAL DESCRIPTION

THIS ITEM SHALL CONSIST OF ALL CLEARING AND GRUBBING, DEMOLITION, PREPARATION OF LAND TO BE FILLED, FILLING OF THE LAND, SPREADING, COMPACTION TESTING AND INSPECTION OF THE FILL, AND ALL SUBSIDIARY WORK NECESSARY TO COMPLETE THE GRADING OF THE CUT AND FILL AREAS TO CONFORM WITH THE LINES, GRADES AND SLOPES AS SHOWN ON THE APPROVED PLANS

ALL LOT GRADING MUST MEET REQUIREMENTS OF FHA/HUD HANDBOOK 4140.3, SPECIFICATIONS FOR LAND DEVELOPMENTS ON CONTROLLED EARTHWORK, DATASHEET 79g. HUD 79g REQUIREMENTS FOR FILL MATERIAL OF 6 INCHES AND MORE WILL BE CONDUCTED. ALL CUT AREAS WILL ALSO MEET THE REQUIREMENTS FOR HUD 79g COMPACTION TESTING. IN ADDITION, ENGINEERS MUST PROVIDED VERIFICATION OF ALL AREAS WHICH DO NOT REQUIRE HUD 79g.

# 2. CLEARING THE AREA TO BE FILLED

ALL TIMBER, LOGS, TREES, BRUSH AND RUBBISH SHALL BE REMOVED FROM THE SITE.

### 3. SCARIFYING THE AREA TO BE FILLED

ALL ORGANIC MATTER SHALL BE REMOVED FROM THE SURFACE UPON WHICH THE FILL IS TO BE PLACED, AND THE SURFACE SHALL THEN BE DISKED OR SCARIFIED TO A MINIMUM DEPTH OF SIX INCHES (6"), ALL SURFACE RUTS OR OTHER UNEVEN FEATURES WILL BE LEVELED PRIOR TO FIELD DENSITY TESTING. WHERE FILLS ARE MADE ON HILLSIDES OR SLOPES, THE SLOPE OF THE ORIGINAL GROUND

UPON WHICH THE FILL IS TO BE PLACED SHALL BE DISKED OR SCARIFIED. WHERE THE SLOPE RATIO OF THE ORIGINAL GROUND IS STEEPER THAN 5 HORIZONTAL TO 1 VERTICAL, THE BANK SHALL BE STEPPED OR BENCHED. GROUND SLOPES WHICH ARE FLATTER THAN 5 TO 1 SHALL BE BENCHED WHEN CONSIDERED NECESSARY BY THE GEOTECHNICAL ENGINEER.

## 4. COMPACTING THE AREA TO BE FILLED

FOLLOWING THE CLEARING AND DISKING OR SCARIFYING OF THE FILL AREA, IT SHALL BE BLADED UNTIL IT IS UNIFORM AND FREE FROM LARGE CLODS. THE AREA SHALL BE BROUGHT TO THE ADEQUATE MOISTURE CONTENT AND COMPACTED (TYPICALLY) TO NOT LESS THAN NINETY PERCENT (90%) OF MAXIMUM DENSITY IN ACCORDANCE WITH THE CURRENT ASTM D 1557 COMPACTION PROCEDURE, OR 95% OF MAXIMUM DENSITY IN ACCORDANCE WITH THE CURRENT THD--TEX--113--E COMPACTION PROCEDURE.

### 5. FILL MATERIALS

THE MATERIALS USED SHALL BE FREE FROM ORGANIC MATTER AND OTHER DELETERIOUS SUBSTANCES, SUCH AS TREES, BRUSH AND RUBBISH, AND SHALL NOT CONTAIN ROCKS OR LUMPS HAVING A DIAMETER OF MORE THAN SIX INCHES (6").

### 6. DEPTH AND MIXING OF FILL LAYERS

THE SELECTED FILL MATERIAL SHALL BE PLACED IN LEVEL, UNIFORM LAYERS WHICH, WHEN COMPACTED, SHALL HAVE A DENSITY CONFORMING TO THAT STIPULATED ABOVE EACH LAYER SHALL BE THOROUGHLY MIXED DURING THE SPREADING TO ENSURE UNIFORMITY OF MATERIAL IN EACH LAYER. COMPACTED LAYER THICKNESS MAY VARY DEPENDING ON THE COMPACTION EQUIPMENT OF DEMONSTRATED CAPABILITY. THE MAXIMUM LOOSE DEPTH FOR ANY MATERIAL SHALL NOT EXCEED TWELVE INCHES (12"). FOR TESTING REQUIREMENTS OF FILL MATERIAL, SEE DENSITY TESTING.

#### 7. ROCK

WHEN FILL MATERIAL INCLUDES ROCK, THE MAXIMUM ROCK SIZE SHALL BE AS APPROVED BY THE GEOTECHNICAL ENGINEER. NO LARGE ROCKS SHALL BE ALLOWED TO NEST AND ALL VOIDS MUST BE FILLED WITH SMALL STONES OR SOIL AND ADEQUATELY COMPACTED. NO LARGE ROCKS WILL BE PERMITTED WITHIN EIGHTEEN INCHES (18") OF THE FINISHED GRADE.

### 8. COMPACTION OF FILL LAYER

COMPACTION EQUIPMENT SHALL BE CAPABLE OF COMPACTING THE FILL TO THE SPECIFIED DENSITY. COMPACTION SHALL BE ACCOMPLISHED WHILE THE FILL MATERIAL IS AT OR NEAR THE APPROPRIATE MOISTURE CONTENT. COMPACTION OF EACH LAYER SHALL BE CONTINUOUS OVER THE ENTIRE STRUCTURAL AREA (BENEATH PROPOSED STRUCTURES).

## 9. COMPACTION OF SLOPES

THE FACES OF FILL SLOPES SHALL BE COMPACTED. COMPACTING OPERATIONS SHALL BE CONTINUED UNTIL THE SLOPE FACES ARE STABLE BUT NOT TOO DENSE FOR PLANTING ON THE SLOPES. COMPACTION OF THE SLOPE FACES MAY BE DONE PROGRESSIVELY IN INCREMENTS OF THREE TO FIVE FEET (3' TO 5') IN FILL HEIGHT AS THIS FILL PROGRESSES OR AFTER THE FILL HAS BEEN BROUGHT TO ITS TOTAL HEIGHT.

### 10. MOISTURE CONTENT

THE FILL MATERIAL SHALL BE COMPACTED AT THE APPROPRIATE MOISTURE CONTENT SPECIFIED FOR THE SOILS BEING USED. APPROPRIATE MOISTURE CONTENT IS DEFINED, TYPICALLY, AS OPTIMUM MOISTURE CONTENT; HOWEVER, FOR EXPANSIVE SOILS IT MAY BE GREATER THAN OPTIMUM MOISTURE CONTENT, AND OTHER MOISTURE CONTENTS MAY BE NECESSARY TO PRODUCE THE DESIRED RESULTS WITH CERTAIN SOILS.

#### 11. DENSITY TESTS

FIELD DENSITY TESTS SHALL BE PERFORMED ON LAYERS OF FILL WHEN THE FILL IS BEING PLACED AS DIRECTED BY THE GEOTECHNICAL ENGINEER. THE MAXIMUM FILL HEIGHT BETWEEN DENSITY TESTING SHALL BE TWELVE INCHES (12") AND AS SPECIFIED BY GEOTECHNICAL ENGINEER. ALL TESTING SHALL BE REQUESTED BY THE CONTRACTOR TO MEET THE CONTRACTOR'S CONSTRUCTION SCHEDULE. NOTIFICATION BY THE CONTRACTOR FOR GEOTECHNICAL ENGINEER TO CONDUCT TESTS SHALL BE AT LEAST THE DAY BEFORE. THIS NOTIFICATION SHALL INCLUDE THE FILL AREA LOCATION (LOT AND BLOCK), THE LIFT OR HEIGHT OF FILL AND APPROXIMATE DESIRED TIME OF TESTING. WHEN THESE TESTS INDICATE THAT THE DENSITY OF ANY LAYER OF FILL OR PORTION THEREOF IS BELOW THE REQUIRED DENSITY, THE PARTICULAR LAYER OR PORTION SHALL BE REWORKED AND RETESTED AT THE EXPENSE OF THE CONTRACTOR UNLESS THE CONTRACTOR CAN SHOW EVIDENCE THAT CIRCUMSTANCES BEYOND HIS CONTROL REQUIRED THE RETESTING. GENERALLY, THE SPECIFIC TESTING WILL BE AS FOLLOWS AND CONDUCTED BY GEOTECHNICAL ENGINEER.

1. THE LAND TO BE FILLED (PREPARED SUBGRADE) SHALL BE PREPARED AND TESTED AT A FREQUENCY AS DETERMINED BY THE GEOTECHNICAL ENGINEER. THE FIRST LIFT OF COMPACTED FILL (GENERALLY 8 TO 12-IN.) SHALL BE

TESTED AS DETERMINED BY THE GEOTECHNICAL ENGINEER. ANY AREAS SUPPORTING THE PROPOSED STRUCTURES REQUIRING FILL SHALL BE TESTED FOR DENSITY COMPLIANCE.

3. FILLS SHALL BE TESTED A MAXIMUM OF EACH TWELVE INCHES (12") AND AS SPECIFIED BY GEOTECHNICAL ENGINEER, OF FILL 4. TEST RESULTS WILL BE PROVIDED BY THE FIELD TECHNICIAN TO THE

CONTRACTOR WHEN POSSIBLE; HOWEVER, ALL TEST RESULTS ARE TO BE REVIEWED BY THE GEOTECHNICAL ENGINEER FOR COMPLIANCE. THE ENGINEER WILL NOTIFY THE CONTRACTOR OF ALL TEST RESULTS.

## 12. CUT/FILL LOTS

AREAS INVOLVING CUT ON ONE PORTION AND FILL ON ANOTHER PORTION OF A SPECIFIC LOT SHALL BE PREPARED TO A MINIMUM DEPTH OF 6-IN. AND WILL BE THE SAME MATERIAL CLASSIFICATION AT THE SAME COMPACTION AND MOISTURE CONTENT A MINIMUM OF TWO (2) FIELD DENSITY TESTS SHALL BE REQUIRED ON EACH CUT/FILL LOT FOR THE PURPOSE OF DETERMINING UNIFORMITY OF THE AREA SUPPORTING THE PROPOSED STRUCTURES.

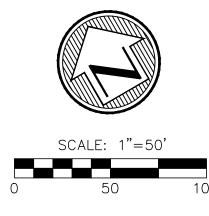
### NOTES:

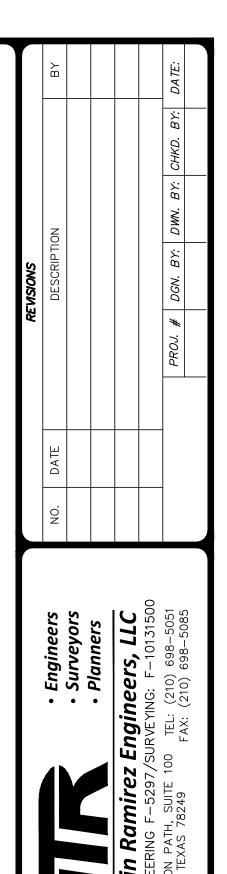
MINIMUM SLAB EXPOSURE IS 1.0'.

ALL ELEVATIONS AT FRONT PROPERTY LINE ARE 0.18' ABOVE CURB ELEVATION.

CONTRACTOR TO VERIFY 1.5% MINIMUM SLOPE ON LOTS AND REGRADE TO MEET MINIMUM PROPOSED

CONTRACTOR TO CLEAR ALL RIGHT OF WAY, EASEMENTS AND PRESERVE ANY TREE 10" AND LARGER OUTSIDE OF THESE AREAS.







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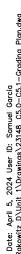


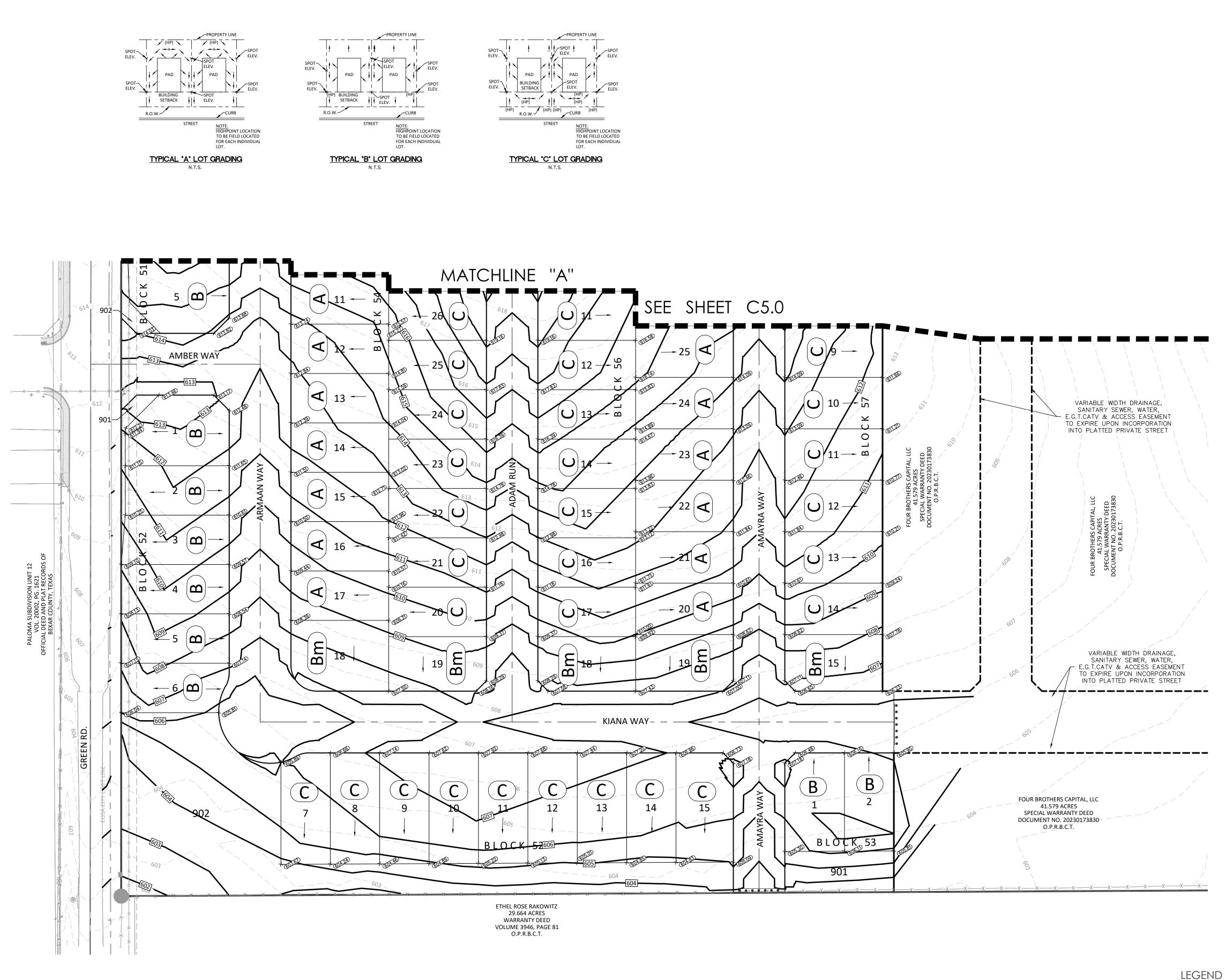
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SUBMITTAL SET





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# PLAT NO. 23-11800388

# GENERAL SPECIFICATIONS FOR SITE PREPARATION

## 1. GENERAL DESCRIPTION

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SPECIFICATIONS FOR LAND DEVELOPMENTS ON CONTROLLED EARTHWORK, DATASHEET 79g. HUD 79g REQUIREMENTS FOR FILL MATERIAL OF 6 INCHES AND MORE WILL BE CONDUCTED. ALL CUT AREAS WILL ALSO MEET THE REQUIREMENTS FOR HUD 79g COMPACTION TESTING. IN ADDITION, ENGINEERS MUST PROVIDED VERIFICATION OF ALL AREAS WHICH DO NOT REQUIRE HUD 79g.

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THE MATERIALS USED SHALL BE FREE FROM ORGANIC MATTER AND OTHER DELETERIOUS SUBSTANCES, SUCH AS TREES, BRUSH AND RUBBISH, AND SHALL NOT CONTAIN ROCKS OR LUMPS HAVING A DIAMETER OF MORE THAN SIX INCHES (6").

#### 6. DEPTH AND MIXING OF FILL LAYERS

THE SELECTED FILL MATERIAL SHALL BE PLACED IN LEVEL, UNIFORM LAYERS WHICH, WHEN COMPACTED, SHALL HAVE A DENSITY CONFORMING TO THAT STIPULATED ABOVE. EACH LAYER SHALL BE THOROUGHLY MIXED DURING THE SPREADING TO ENSURE UNIFORMITY OF MATERIAL IN EACH LAYER. COMPACTED LAYER THICKNESS MAY VARY DEPENDING ON THE COMPACTION EQUIPMENT OF DEMONSTRATED CAPABILITY. THE MAXIMUM LOOSE DEPTH FOR ANY MATERIAL SHALL NOT EXCEED TWELVE INCHES (12"). FOR TESTING REQUIREMENTS OF FILL MATERIAL, SEE DENSITY TESTING.

#### 7. ROCK

WHEN FILL MATERIAL INCLUDES ROCK, THE MAXIMUM ROCK SIZE SHALL BE AS APPROVED BY THE GEOTECHNICAL ENGINEER. NO LARGE ROCKS SHALL BE ALLOWED TO NEST AND ALL VOIDS MUST BE FILLED WITH SMALL STONES OR SOIL AND ADEQUATELY COMPACTED. NO LARGE ROCKS WILL BE PERMITTED WITHIN EIGHTEEN INCHES (18") OF THE FINISHED GRADE.

### 8. COMPACTION OF FILL LAYER

COMPACTION EQUIPMENT SHALL BE CAPABLE OF COMPACTING THE FILL TO THE SPECIFIED DENSITY. COMPACTION SHALL BE ACCOMPLISHED WHILE THE FILL MATERIAL IS AT OR NEAR THE APPROPRIATE MOISTURE CONTENT. COMPACTION OF EACH LAYER SHALL BE CONTINUOUS OVER THE ENTIRE STRUCTURAL AREA (BENEATH PROPOSED STRUCTURES).

# 9. COMPACTION OF SLOPES

THE FACES OF FILL SLOPES SHALL BE COMPACTED. COMPACTING OPERATIONS SHALL BE CONTINUED UNTIL THE SLOPE FACES ARE STABLE BUT NOT TOO DENSE FOR PLANTING ON THE SLOPES. COMPACTION OF THE SLOPE FACES MAY BE DONE PROGRESSIVELY IN INCREMENTS OF THREE TO FIVE FEET (3' TO 5') IN FILL HEIGHT AS THIS FILL PROGRESSES OR AFTER THE FILL HAS BEEN BROUGHT TO ITS TOTAL HEIGHT.

## 10. MOISTURE CONTENT

THE FILL MATERIAL SHALL BE COMPACTED AT THE APPROPRIATE MOISTURE CONTENT SPECIFIED FOR THE SOILS BEING USED. APPROPRIATE MOISTURE CONTENT IS DEFINED, TYPICALLY, AS OPTIMUM MOISTURE CONTENT: HOWEVER, FOR EXPANSIVE SOILS IT MAY BE GREATER THAN OPTIMUM MOISTURE CONTENT, AND OTHER MOISTURE CONTENTS MAY BE NECESSARY TO PRODUCE THE DESIRED RESULTS WITH CERTAIN SOILS.

#### 11. DENSITY TESTS

FIELD DENSITY TESTS SHALL BE PERFORMED ON LAYERS OF FILL WHEN THE FILL IS BEING PLACED AS DIRECTED BY THE GEOTECHNICAL ENGINEER. THE MAXIMUM FILL HEIGHT BETWEEN DENSITY TESTING SHALL BE TWELVE INCHES (12") AND AS SPECIFIED BY GEOTECHNICAL ENGINEER. ALL TESTING SHALL BE REQUESTED BY THE CONTRACTOR TO MEET THE CONTRACTOR'S CONSTRUCTION SCHEDULE. NOTIFICATION BY THE CONTRACTOR FOR GEOTECHNICAL ENGINEER TO CONDUCT TESTS SHALL BE AT LEAST THE DAY BEFORE. THIS NOTIFICATION SHALL INCLUDE THE FILL AREA LOCATION (LOT AND BLOCK), THE LIFT OR HEIGHT OF FILL AND APPROXIMATE DESIRED TIME OF TESTING. WHEN THESE TESTS INDICATE THAT THE DENSITY OF ANY LAYER OF FILL OR PORTION THEREOF IS BELOW THE REQUIRED DENSITY, THE PARTICULAR LAYER OR PORTION SHALL BE REWORKED AND RETESTED AT THE EXPENSE OF THE CONTRACTOR UNLESS THE CONTRACTOR CAN SHOW EVIDENCE THAT CIRCUMSTANCES BEYOND HIS CONTROL REQUIRED THE RETESTING. GENERALLY, THE SPECIFIC TESTING WILL BE AS FOLLOWS AND CONDUCTED BY GEOTECHNICAL ENGINEER.

1. THE LAND TO BE FILLED (PREPARED SUBGRADE) SHALL BE PREPARED AND TESTED AT A FREQUENCY AS DETERMINED BY THE GEOTECHNICAL ENGINEER. THE FIRST LIFT OF COMPACTED FILL (GENERALLY 8 TO 12-IN.) SHALL BE TESTED AS DETERMINED BY THE GEOTECHNICAL ENGINEER. ANY AREAS

SUPPORTING THE PROPOSED STRUCTURES REQUIRING FILL SHALL BE TESTED FOR DENSITY COMPLIANCE. 3. FILLS SHALL BE TESTED A MAXIMUM OF EACH TWELVE INCHES (12") AND AS

SPECIFIED BY GEOTECHNICAL ENGINEER, OF FILL 4. TEST RESULTS WILL BE PROVIDED BY THE FIELD TECHNICIAN TO THE CONTRACTOR WHEN POSSIBLE; HOWEVER, ALL TEST RESULTS ARE TO BE REVIEWED BY THE GEOTECHNICAL ENGINEER FOR COMPLIANCE. THE ENGINEER WILL NOTIFY THE CONTRACTOR OF ALL TEST RESULTS.

### 12. CUT/FILL LOTS

AREAS INVOLVING CUT ON ONE PORTION AND FILL ON ANOTHER PORTION OF A SPECIFIC LOT SHALL BE PREPARED TO A MINIMUM DEPTH OF 6-IN. AND WILL BE THE SAME MATERIAL CLASSIFICATION AT THE SAME COMPACTION AND MOISTURE CONTENT A MINIMUM OF TWO (2) FIELD DENSITY TESTS SHALL BE REQUIRED ON EACH CUT/FILL LOT FOR THE PURPOSE OF DETERMINING UNIFORMITY OF THE AREA SUPPORTING THE TRUCTURES.

B EXPOSURE IS 1.0'.

AT FRONT PROPERTY LINE ARE 0.18' ABOVE CURB ELEVATION.

CONTRACTOR TO VERIFY 1.5% MINIMUM SLOPE ON LOTS AND REGRADE TO MEET MINIMUM PROPOSED ELEVATIONS IF NECESSARY

CONTRACTOR TO CLEAR ALL RIGHT OF WAY, EASEMENTS AND PRESERVE ANY TREE 10" AND LARGER OUTSIDE OF THESE AREAS.

# = DRAINAGE TO REAR OF LOT

= DRAINAGE TO BOTH FRONT AND

= DRAINAGE TO FRONT OF LOT

= CLEARING AND/OR GRADING OF UTILITY EASEMENTS

= PROPOSED ELEVATION

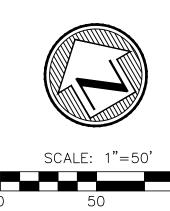
REAR LOT

= EXISTING ELEVATION

---1024 - - - = EXISTING CONTOUR

= PROPOSED CONTOUR

----- = PROPERTY LINE





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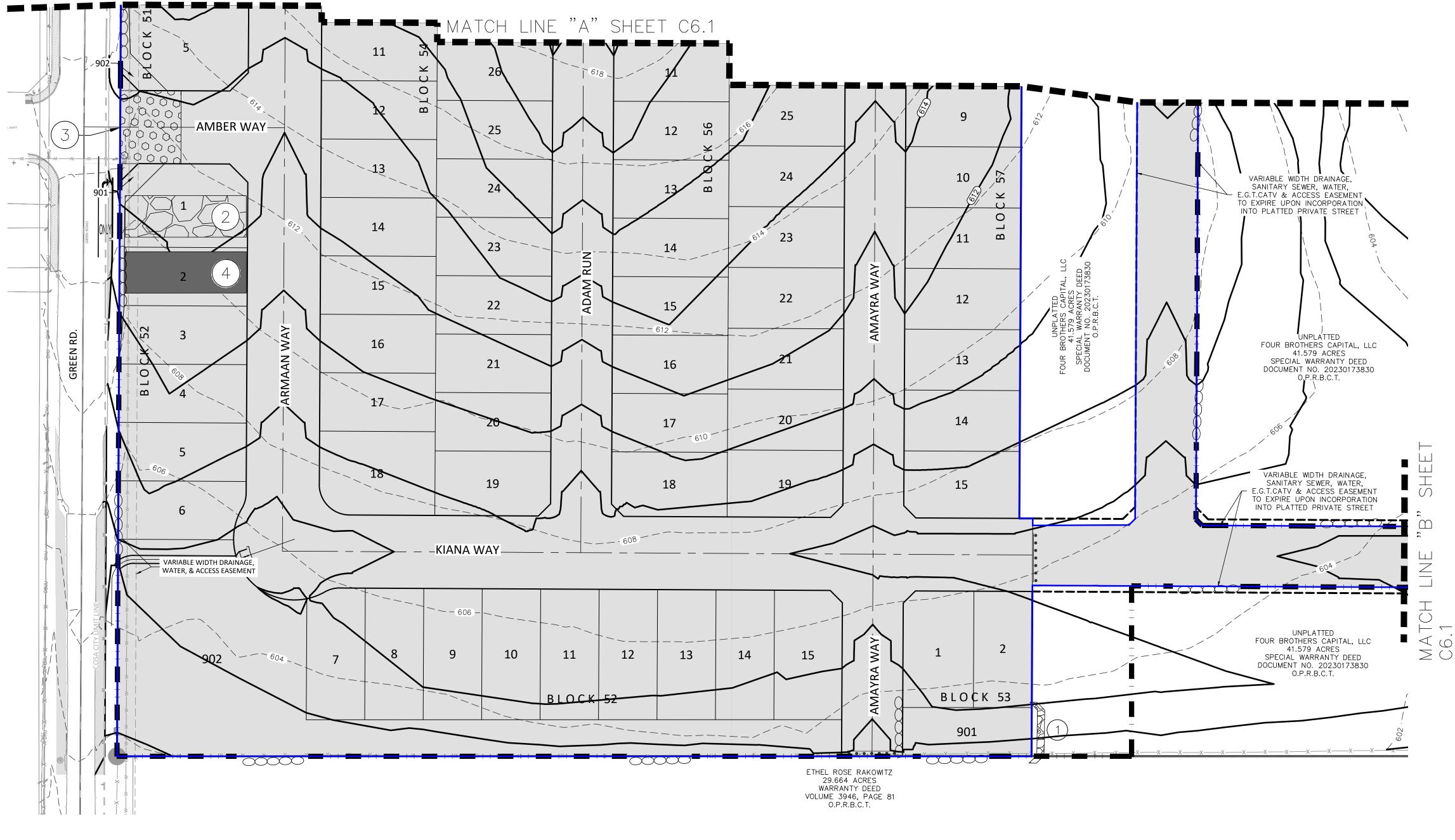
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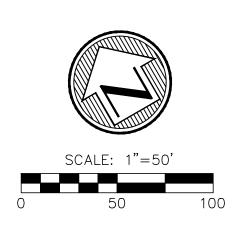
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PROPOSED ST
<u>NOTES:</u>
MINIMUM SLAB
ALL ELEVATIONS



# PLAT NO. 23-11800388



# LEGEND

PROPERTY BOUNDARY		
EXISTING CONTOUR		
PROPOSED CONTOUR		604)
SILT FENCE		
ROCK BERM	$\left(1\right)$	DECT DEC
CONSTRUCTION STAGING AREA	2	DE A DE T
STABILIZED CONSTRUCTION ENTRANCE/EXIT	3	
CONCRETE WASHOUT PIT	4	
BAGGED GRAVEL INLET FILTER		$\infty$
AREA OF DISTURBANCE		

#### **GENERAL NOTES:**

WASHING - WHEN NECESSARY, WHEELS SHALL BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE ONTO PUBLIC ROADWAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE WHICH DRAINS INTO AN APPROVED STRAP OR SEDIMENT BASIN. ALL SEDIMENT SHALL BE PREVENTED FROM ENTERING ANY STORM DRAIN, DITCH, OR WATERCOURSE USING APPROVED METHODS.

MAINTENANCE - THE ENTRANCE/EXIT SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC ROADWAYS. THIS MAY REQUIRE PERIODIC DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND, AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, DROPPED, WASHED, OR TRACKED ONTO PUBLIC ROADWAY MUST BE REMOVED IMMEDIATELY.

DRAINAGE - ENTRANCE/EXIT MUST BE PROPERLY GRADED OR INCORPORATE A DRAINAGE SWALE TO PREVENT RUNOFF FROM LEAVING THE CONSTRUCTION SITE.

ALL CITY PUBLIC SERVICE WORK IS INCLUDED AS PART OF THE CONSTRUCTION OF THE SUBDIVISION AND HAS BEEN CONSIDERED IN THIS STORM WATER POLLUTION PREVENTION PLAN (SWPPP).

CONTRACTOR TO INSTALL AND MAINTAIN THE EROSION AND SEDIMENTATION CONTROLS AS DESIGNED AND SHALL INSPECT THE CONTROLS BI-WEEKLY (14 DAYS) AND AFTER EVERY SIGNIFICANT RAINFALL (0.5 INCHES OR GREATER) TO ENSURE SIGNIFICANT DISTURBANCE HAS NOT OCCURRED. SEDIMENT DEPOSITED AFTER A SIGNIFICANT RAINFALL SHALL BE REMOVED AND PLACED IN A DESIGNATED SOIL DISPOSAL AREA. CONTRACTOR TO ENGAGE A THIRD PARTY FIRM TO PROVIDE A DETAILED STORM WATER POLLUTION PREVENTION PLAN (SWPPP) WHICH INCLUDES INSPECTION AND REPORTING PROCEDURES.

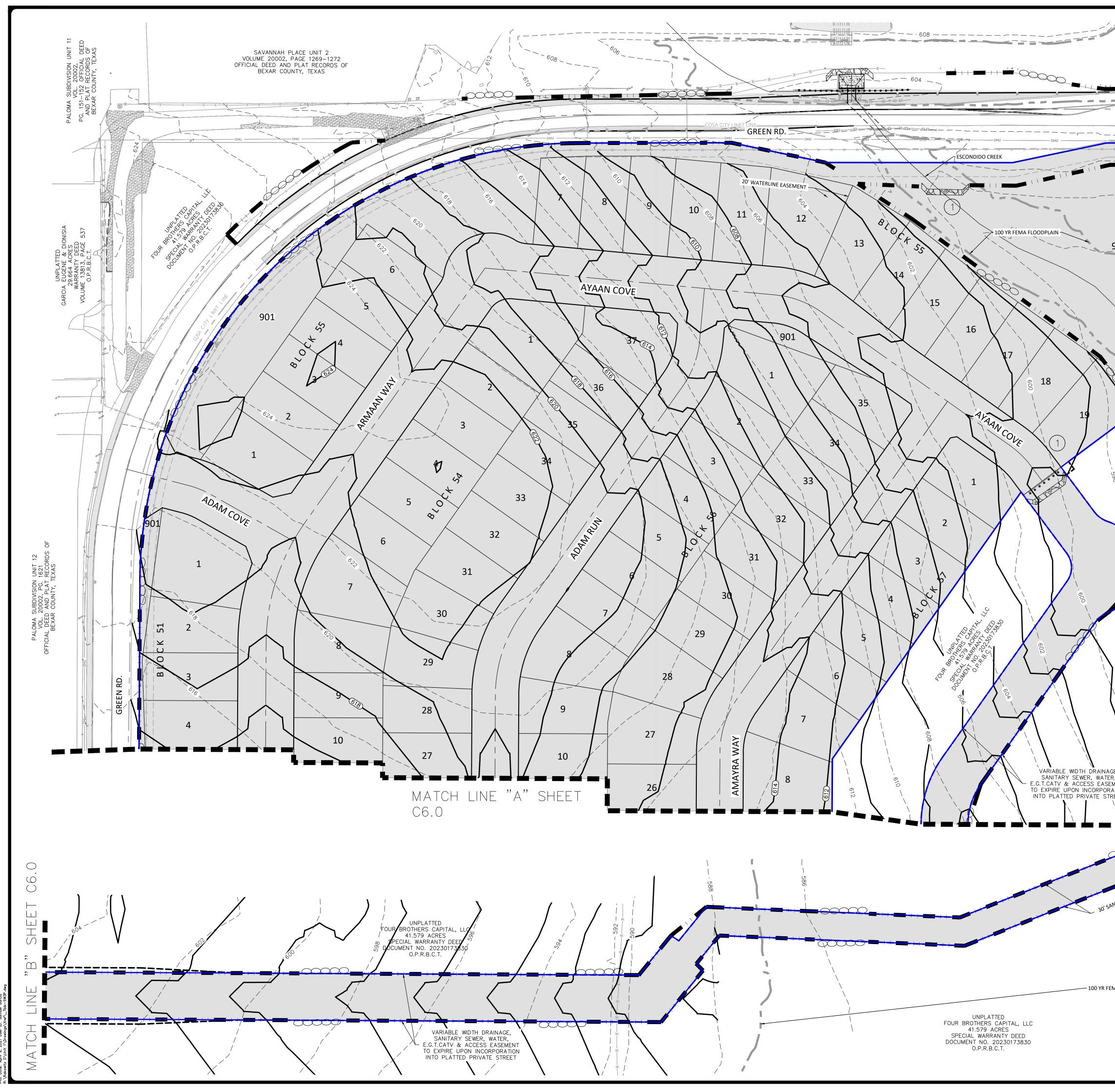
CONTRACTOR WILL INSPECT BMP'S AT LEAST TWICE A WEEK (EVERY 14 DAYS) AS WELL AS AFTER EVERY HALF INCH OR MORE OF RAINFALL. CONTROLS WILL BE REPAIRED, REPLACED, AND/OR REVISED AS NECESSARY.

CONTRACTOR TO PLACE TRENCH EXCAVATION MATERIAL ON THE UPGRADIENT (HIGH) SIDE OF THE TRENCH.

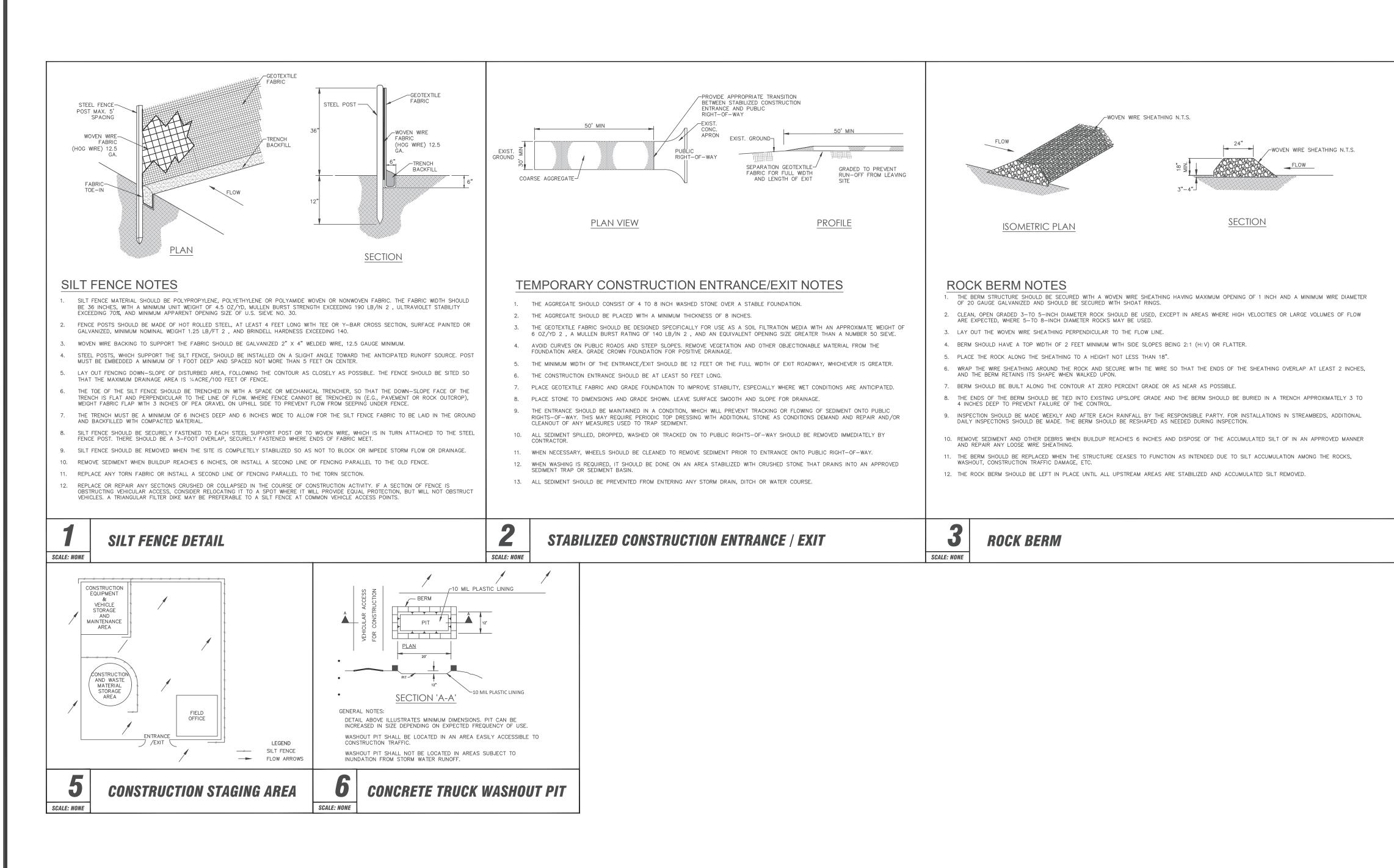
ALL SOIL, SAND, GRAVEL, AND EXCAVATED MATERIALS STOCKPILED ON-SITE WILL HAVE APPROPRIATELY SIZED SILT FENCE PLACED UPGRADIENT AND DOWN GRADIENT.

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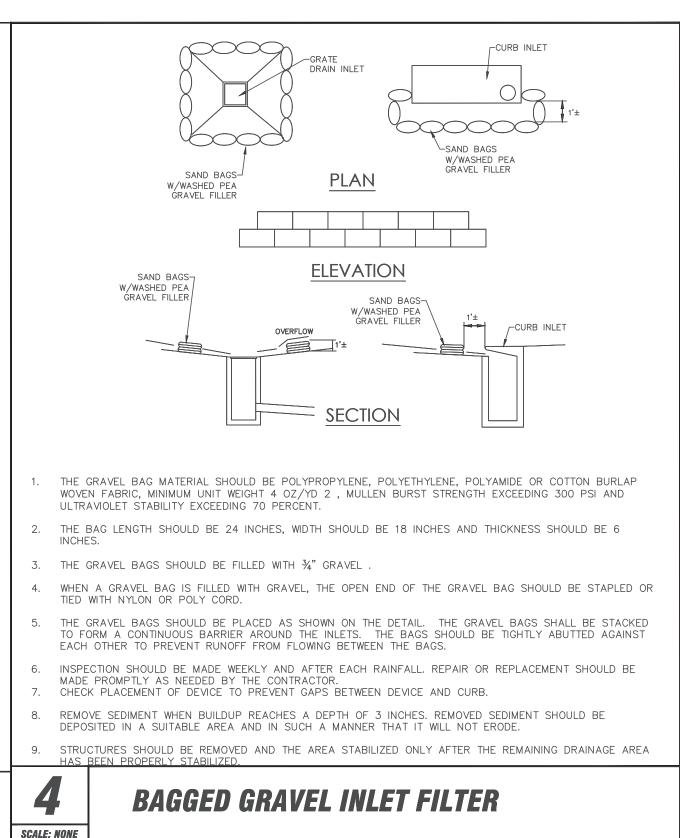
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