



# **SPECIFICATIONS FOR SUNFLOWER RIDGE UNIT 2**

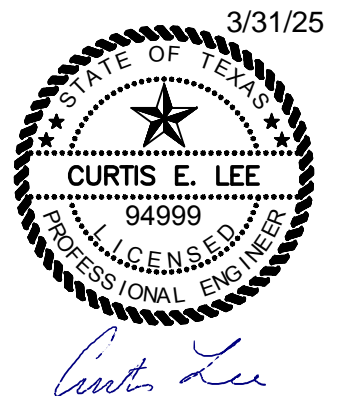


---

Transportation | Water Resources | Land Development | Surveying | Environmental



# SPECIFICATIONS FOR SUNFLOWER RIDGE UNIT 2



**TABLE OF CONTENTS****PROCUREMENT AND CONTRACTING REQUIREMENTS****1.01 Division 00 -- Procurement and Contracting Requirements**

- A. 00 0110 - Table of Contents
- B. 00 1116.10 - Invitation to Bid
- C. 00 1116.20 - Bid Basis
- D. 00 2113 - Instructions to Bidders
- E. 00 4100 - Bid Form
- F. 00 5000 - Contracting Forms and Supplements
- G. 00 5200.10 - Agreement

**SPECIFICATIONS****2.01 Division 01 -- General Requirements**

- A. 01 2000 - Price and Payment Procedures
- B. 01 4216 - Definitions
- C. 01 5000 - Temporary Facilities and Controls
- D. 01 5213 - Field Offices and Sheds
- E. 01 5713 - Temporary Erosion and Sediment Control
- F. 01 5713.10 - Spill Response
- G. 01 5813 - Temporary Project Signage
- H. 01 7123.10 - Field Engineering
- I. 01 7836.10 - Warranties and Bonds

**2.02 Division 02 -- Existing Conditions**

- A. 02 3000 - Subsurface Investigation

**2.03 Division 03 -- Concrete**

- A. 03 0100 - Maintenance of Concrete
- B. 03 1000 - Concrete Forming and Accessories
- C. 03 2000 - Concrete Reinforcing
- D. 03 3000 - Cast-in-Place Concrete
- E. 03 3000.10 - Controlled Low Strength Backfill

**2.04 Division 31 -- Earthwork**

- A. 31 1000 - Site Clearing
- B. 31 1000.10 - Tree Protection
- C. 31 2200 - Grading
- D. 31 2316 - Excavation
- E. 31 2316.13 - Trenching
- F. 31 2316.14 - Trench Excavation Protection
- G. 31 2323 - Fill

- H. 31 3213.19 - Lime Soil Stabilization

**2.05 Division 32 -- Exterior Improvements**

- A. 32 1123 - Aggregate Base Courses
- B. 32 1216 - Asphalt Paving
- C. 32 1250 - Site Pavement
- D. 32 1313.10 - Concrete Curbs, Gutters and Sidewalks
- E. 32 3223 - Segmental Retaining Walls
- F. 32 9219 - Seeding
- G. 32 9223 - Sodding

**2.06 Division 33 -- Utilities**

- A. 33 0110.58 - Disinfection of Water Utility Piping Systems
- B. 33 0273 - Site Concrete Encasement, Cradles, Saddles and Collars
- C. 33 0561 - Concrete Manholes
- D. 33 0513.10 - Site Castings
- E. 33 1416 - Site Water Utility Distribution Piping
- F. 33 3113 - Site Sanitary Sewerage Gravity Piping
- G. 33 4100 - Subdrainage
- H. 33 4211 - Stormwater Gravity Piping
- I. 33 4213 - Stormwater Culverts

**END OF SECTION**

**INVITATION TO BID****PART 1 GENERAL****1.01 PROJECT IDENTIFICATION**

- A. Project Name: Sunflower Ridge Unit 2, located at intersection of Orion Drive and Goodwin Lane.
- B. Project Number: 13348-00.
- C. The Owner, hereinafter referred to as Owner: AG Sunflower Ridge.
- D. Owner's Representative: Curtis Lee, P.E.
  - 1. Corporate Name: AG Sunflower Ridge, LLC.
  - 2. Address: 5910 N. Central Expressway, Suite 1600.
  - 3. City, State, Zip: Dallas, Tx 75206.
  - 4. Phone/Fax: (210) 510-1118.
  - 5. E-mail: rbyrd@aspengroverrealty.com.

**1.02 BID OPENING**

- A. Bid Place
  - 1. Sealed bids will be received in the office of the Engineer:  
Project Name: Sunflower Ridge Unit 2  
ATTENTION: Curtis Lee, P.E.  
c/o Pape-Dawson Engineers  
2000 NW Loop 410  
San Antonio, TX 78213  
(210) 375-9000
- B. Bid Due Date: April 17, 2025 by 3 PM local time.
- C. Bids will be opened in private.

**1.03 REJECTION**

- A. The Owner reserves the right to reject any and all Bids, and to waive any irregularities or information.

**1.04 DELIVERY OF PROPOSALS**

- A. It is the Bidder's responsibility to deliver the proposal at the proper time to the proper place. The mere fact that a proposal was dispatched will not be considered. The Bidder must have the proposal delivered as specified in 1.02.

**1.05 TIME OF COMPLETION**

- A. Each Bidder shall indicate on his bid the number of calendar days he will require to complete the entire work under the Contract with all possible diligence within the time limit as stipulated in the bid proposal. The Owner considers it imperative that the work be completed at the earliest possible date and consideration will be given to the proposed completion date in determining the Bidders to whom the Contract will be awarded.

**END OF SECTION**

**BID BASIS****PART 1 GENERAL****1.01 PRICING DOCUMENTS**

- A. The pricing documents are not final drawings and specifications. These documents, together with the Contractor's knowledge of and anticipation of the final requirements, will be the basis for all pricing. It is the Owner's intention to have the Engineer develop final construction documents with the Contractor's review and assistance. The Engineer, however, will make the final decisions during the development of the final documents.
- B. The final documents to be developed will be a further refinement and clarification of the existing documents; however, any material or labor required to provide a complete and operable project that is not indicated in these initial documents should be included in this price submittal. Change Orders increasing the Contract amount will be issued only in the event the scope is changed.
- C. The Contractor undertakes and accepts that the contract documents are meant to include or imply all items required for the proper execution of the work. Any items mentioned in the specifications are not shown in the drawings, or the reverse, shall be provided as if shown or mentioned in both.

**END OF SECTION**

**INSTRUCTIONS TO BIDDERS****PART 1****1.01 BID SUBMISSION**

- A. Bids signed and under seal, executed, and dated will be received at the office of PD at 2000 NW Loop 410, San Antonio, Texas 78213 before 3 p.m. local standard time on 04-17-2025.
- B. Offers submitted after the above time will be returned to the bidder unopened.
- C. Submit required Supplements To Bid Forms with the bid documents.
- D. Amendments to the submitted offer will be permitted if received in writing prior to bid closing and if endorsed by the same party or parties who signed and sealed the offer.

**1.02 INTENT**

- A. The intent of this Bid request is to obtain an offer to perform work to complete a single-family residential subdivision located at the intersection of Orion Drive and Goodwin Lane in New Braunfels, Texas for a Stipulated Sum contract, in accordance with Contract Documents.

**1.03 WORK IDENTIFIED IN THE CONTRACT DOCUMENTS**

- A. Work of this proposed Contract comprises building construction, including general construction Work.
- B. Project Location:

**1.04 CONTRACT TIME**

- A. Identify Contract Time in the Bid Form. The completion date in the Agreement shall be the Contract Time added to the commencement date.
- B. Owner requires that under the work of this contract be completed as quickly as possible and consideration will be given to time of completion when reviewing the submitted bids.

**PART 2****2.01 DEFINITIONS**

- A. Bid Documents: Contract Documents supplemented with Invitation To Bid, Instructions to Bidders, Information Available to Bidders, Bid Form Supplements To Bid Forms and Appendices identified.
- B. Bid, Offer, or Bidding: Act of submitting an offer under seal.
- C. Bid Amount: Monetary sum identified by the Bidder in the Bid Form.

**2.02 CONTRACT DOCUMENTS IDENTIFICATION**

- A. The Contract Documents are identified as Project Number 13348-00, as prepared by Pape-Dawson Engineers who is located at 2000 NW Loop 410, San Antonio, Texas 78213, and with contents as identified in the Table of Contents.

**2.03 AVAILABILITY**

- A. Complete sets of Bidding Documents shall be used in preparing bids.
- B. Neither the Owner nor the Engineer assumes any responsibility for errors or misinterpretations resulting from the use of incomplete sets of Bidding Documents.
- C. Bid documents may be obtained electronically from Pape-Dawson Engineers, 2000 NW Loop 410, San Antonio, Texas 78213, (210) 375-9000.

- D. Subcontractors requiring Drawings and Project Manuals must obtain them from General Contractors or purchase sets by paying the reproduction cost, which will not be refunded.
- E. Bid Documents are made available only for the purpose of obtaining offers for this project. Their use does not grant a license for other purposes.

**2.04 EXAMINATION**

- A. Bidders shall carefully examine the Bidding Documents and the Construction Site to familiarize themselves with existing local conditions under which the work is to be performed.
- B. Bidders shall carefully examine the Bidding Documents to verify that they agree with the Table of Contents in the Project Manual, the Index of Drawings Sheet on the Drawings, and the Cover Page of all Addenda. Bidders shall be responsible for obtaining any pages or sheets which may have been inadvertently left out during the printing process.
- C. All Bidders and Sub-bidders acknowledge and agree that any information that they may have obtained from Owner or its Engineer relating to site conditions (including surface, sub-surfaces and existing structures, if any), availability of materials or labor, applicable statutes, ordinances, or regulations, and any other information not specifically provided for otherwise in the proposed Contract Documents, including Drawings and Specifications, shall be for general information purposes only, and Owner does not warrant or represent the accuracy or completeness thereof. All Bidders and Sub-bidders agree that they shall and, by submission of a bid, do warrant and represent that they have made their own independent investigation of such matters, have reached their own conclusion with respect thereto, and have relied completely on their own such investigations in connection with the preparation of their bid.
- D. Upon receipt of Bid Documents verify that documents are complete. Notify Engineer should the documents be incomplete.
- E. Immediately notify Engineer upon finding discrepancies or omissions in the Bid Documents.

**2.05 INQUIRIES/ADDENDA**

- A. Bidders shall promptly notify the Engineer of any ambiguity, inconsistency or error which they may discover upon examination of the Bidding Documents or of the site and local conditions. Conflict in documents not brought to the attention of the Engineer during bidding shall be deemed to be provided at no additional cost.
- B. Submit all questions regarding clarification or interpretation of Bidding Documents to the office of the Engineer, Pape-Dawson Engineers, 2000 NW Loop 410, San Antonio, Texas 78213, Attn: Justin Groff, P.E. , (210) 375-9000.
- C. Submit all questions in writing. In the interest of time, requests may be made by telephone, but they must be confirmed in writing the same day. Replies to questions will be issued to all General Contractors in the form of an Addendum.
- D. Addenda may be issued during the bidding period. All Addenda become part of Contract Documents. Include resultant costs in the Bid Amount.
- E. Verbal answers are not binding on any party.
- F. Clarifications requested by bidders must be in writing not less than 7 days before date set for receipt of bids. The reply will be in the form of an Addendum, a copy of which will be forwarded to known recipients.



**2.06 PRODUCT/ASSEMBLY/SYSTEM SUBSTITUTIONS**

- A. General Requirements for Substitution Requests:
- B. Substitution Request Form:
- C. Review and Acceptance of Request:

**PART 3****3.01 SITE EXAMINATION**

- A. Examine the project site before submitting a bid.
- B. The bidder is required to contact Engineer at the following address and phone number in order to arrange a date and time to visit the project site: Pape-Dawson Engineers, 2000 NW Loop 410, San Antonio, Texas 78213, Attn: Justin Groff, P.E., (210) 375-9000.
- C. Extra payments will not be authorized for work that could have been foreseen by careful examination of the Site. Submission of a Bid shall constitute acceptance, by the Bidder, of existing Site conditions as a part of the requirements for this work.
- D. The Contractor shall have access to the premises during the bidding period for the purposes of acquainting himself with the conditions.
- E. The Contractor shall not enter or have access to the site in order to perform the work without first having given timely notice to the Owner so that the necessary arrangements may be made to enter or have access.

**3.02 PREBID CONFERENCE**

- A. A bidders conference has been scheduled for 9 a.m. on the 4th day of April online via Microsoft Teams
- B. All general contract bidders and suppliers are invited.
- C. Representatives of Engineer will be in attendance.
- D. Information relevant to the Bid Documents will be recorded in an Addendum, issued to Bid Document recipients.

**3.03 BASIS OF BIDS**

- A. Bids shall be on a lump sum price basis and shall include all costs for this Project as described and shown by the Contract Documents. Basis for Bidding shall include brands, materials, processes, products, persons or organizations, etc., indicated in the Contract Documents.
- B. Bids shall include all Unit Price costs as indicated by the Contract Documents and Bid Form. The bid price submitted by the Contractor shall be a lump sum price. Unit prices are requested for the purposes of establishing costs for additional work and to evaluate the bids. The Contractor shall guarantee himself of the accuracy of the quantities shown in the bid form. Any quantities shown are estimates only and indicate only the magnitude of the project and a basis for bid comparison. Any discrepancies in quantity or work necessary to fulfill the intent of the plans shall be included, whether a bid item is included or not. Any work required for which a bid item is not shown shall be considered subsidiary to other work items.
- C. Contractor is to perform an independent quantity take-off prior to submitting the bid, to verify that any quantities given in the bid proposal are within five percent (5%) of the actual quantities required to complete the construction represented by the plans and specifications.

If any quantity is found to be in error of more than five percent (5%), the Contractor shall notify the Engineer five days prior to the bid due date.

- D. Bids shall include all Alternate costs as indicated by the Contract Documents and Bid Form.

### **3.04 ALTERNATES**

- A. The Owner may, at his option, elect to proceed with any or all Alternates as set forth in the Bidding Requirements.
- B. Amount shown on Bid Form for each Alternate shall include profit, insurance, contingencies and other costs incidental to performance under such Alternative.
- C. Amount shown on Bid Form for Each Alternate shall include the making of all changes and the installation of all materials and equipment necessary to the accomplishment of the Alternate requirements.
- D. Refer to the respective Section for complete Specifications of each Alternate.
- E. If an Alternate is accepted, it shall be included as part of the Contract Document.

### **3.05 VOLUNTARY ALTERNATES**

- A. The Contractor shall submit a list of voluntary cost-saving alternates for consideration. The list shall be submitted along with the bid.
  - 1. Amount shown in Bid for each Alternate shall include profit, insurance contingencies and other costs incidental to performance under such Alternative.
  - 2. Amount shown in Bid for each Alternate shall include the making of all changes and the installation of all materials and equipment necessary to the accomplishment of the Alternate requirements.

### **3.06 UNIT PRICES**

- A. Authorized work, done in addition to that indicated by the Contract Documents, will be paid for as an extra according to the Unit Price Schedule. Costs of authorized omissions of work from that indicated by the Contract Documents will be deducted from the Contract amount according to the Unit Price Schedule.
- B. Amounts shown on the Unit Price Schedule shall be total compensation; labor, materials, fees, taxes, profit, overhead and insurance and other expenses to be added or deducted from the Contract amount.
- C. Unit Price Schedule form is included with the Bid Form and shall be a part of the Agreement.

### **3.07 BIDS**

- A. Bids shall be made on unaltered Bid Forms furnished by the Engineer. No oral, faxed, voice mailed, or personal Proposals will be considered. All blank spaces shall be properly filled in by typewriter or manually in ink.
- B. Where so indicated by the make up of the Bid Form, sums shall be expressed in both words and figures, and in case of discrepancy between the two, the written amount shall govern.
- C. Any alteration or erasure to information entered in the blank spaces must be initialed by the signer of the Bid. Bidder shall make no additional stipulations on the bid form nor qualify his bid in any other manner.
- D. Original typed sheets shall be submitted, signed in longhand below the typed name of the person authorized to bind the Bidder to a Contract.

- E. Where Bidder is a corporation, Bid must be signed with the legal name of the corporation followed by the name of the State of Incorporation and the legal signature of a person authorized to bind the corporation to a Contract.
- F. The bid must be accompanied by a sealed envelope marked "List of Subcontractors" containing a list of the names and addresses of all proposed subcontractors on the project, a general description of the work to be performed by each and an estimated dollar value of each subcontract.
- G. Upon submittal of his bid, each Bidder shall sign a statement declaring that he has received and understands each Addendum which might be prepared during the course of the bid process.
- H. Failure to submit a Bid in the form requested, or the inclusion of conditions, limitations or provisions distorting the intent of the Bid Documents, will render the Bid irregular and subject to rejection.

**PART 4****4.01 EVIDENCE OF QUALIFICATIONS**

- A. To demonstrate qualification for performing the Work of this Contract, bidders may be requested to submit written evidence of financial position, previous experience, and current commitments, license to perform work in the City.

**4.02 SUBCONTRACTORS/SUPPLIERS/OTHERS**

- A. Owner reserves the right to reject a proposed subcontractor for reasonable cause.
- B. Refer to General Conditions.

**PART 5****5.01 SUBMISSION PROCEDURE**

- A. Bidders shall be solely responsible for the delivery of their bids in the manner and time prescribed.
- B. Submit one copy of the executed offer on the Bid Forms provided, signed and sealed with the required security in a closed opaque envelope, clearly identified with bidder's name, project name and Owner's name on the outside.
- C. If the Bid is sent by mail the sealed envelope shall be enclosed in a separate mailing envelope with the notation "BID ENCLOSED", ATTN: (Owner's Name; Project Name) c/o: Pape-Dawson Engineers, 2000 NW Loop 410, San Antonio, Texas 78213.
- D. Improperly completed information, irregularities in security deposit, may be cause not to open the Bid Form envelope and declare the bid invalid or informal.

**5.02 MODIFICATION OR WITHDRAWAL OF BID**

- A. Bid may not be modified, withdrawn or canceled by the Bidder during the stipulated time period following the time and date designated for the receipt of Bids, unless the award of Contract has been delayed more than ninety (90) days.
- B. Prior to the time and date designated for receipt of Bids, Bids submitted early may be modified or withdrawn only by notice to the party receiving Bids at the place and prior to the time designated for receipt of Bids.
- C. Modification of Bids shall be in writing over the signature of the Bidder. Written confirmation over the signature of Bidder must have been mailed and postmarked on or before the date

and time set for receipt of Bids; it shall be so worded as not to reveal the amount of the original Bid.

- D. Withdrawn Bids may be resubmitted up to the time designated for the receipt of Bids provided that they are then fully in conformance with these instructions to Bidders.

### 5.03 BID INELIGIBILITY

- A. Bids that are unsigned, improperly signed or sealed, conditional, illegible, obscure, contain arithmetical errors, erasures, alterations, or irregularities of any kind, may at the discretion of the Owner, be declared unacceptable.
- B. Bid Forms, Appendices, and enclosures that are improperly prepared may, at the discretion of Owner, be declared unacceptable.
- C. Failure to provide security deposit, bonding or insurance requirements may, at the discretion of Owner, invalidate the bid.
- D. Bids are by invitation, only from selected bidders. Bids from unsolicited bidders may be returned.

### 5.04 CONSIDERATION OF BIDS

- A. Properly identified Bids received on time will be opened in private.
- B. Owner reserves the right to reject any or all bids submitted, to enter into negotiations with any bidder hereunder; to alter the Contract by agreement in writing with the successful bidder; or to take such other steps as may insure its complete freedom of action in selecting the successful bidder, all without any obligation or liability whatsoever to any bidder hereunder.
- C. All bidders shall be completely responsive to these specifications; however, should the bidder, through his own experience, wish to make alternate suggestions regarding any phase of the scope, terms, procedures or frequencies, he may do so as an alternate and such suggestions must be in a separate letter enclosed with this bid.
- D. The Owner shall have the right to reject any or all Bids and in particular to reject a Bid not accompanied by any required bid security or data required by the Bidding Documents or a Bid in any way incomplete or irregular.
- E. The Owner shall have the right to waive any informality or irregularity in any Bid received.
- F. If the Owner accepts any Alternatives, he shall have the right to accept them in any order or combination.
- G. Award of Contract may include full consideration of completion time, unit prices and Alternates. Owner may accept or reject any or all Alternates.
- H. Owner shall have the right to make such inquiries as it seems appropriate to determine the ability of the Bidder to perform the work and the Bidder shall furnish Owner such information for this purpose as Owner may request. The right is reserved to reject any proposal where information submitted by such Bidder does not satisfy Owner that the Bidder is qualified to carry out properly the terms of the contract.
- I. The Owner reserves the right to let other contracts in connection with this work. The Contractor shall afford other Contractors reasonable opportunity for the introduction and storage of their materials and the execution of their work and shall properly connect and coordinate his work with theirs.

- J. The successful Bidder to whom the contract is awarded shall, within ten (10) days, execute the necessary bonds and sign the Agreement. Failure to do so will be considered abandonment by the Owner.
- K. Prior to any work beginning Contractor must adhere to the following requirements:
  - 1. A fully executed Contract between Owner and Contractor.
  - 2. A Certificate of Insurance showing that the insurance requirements outlined herein have been met.

**PART 6****6.01 SECURITY DEPOSIT**

- A. Bids shall be accompanied by a security deposit as follows:
  - 1. Bid Bond of a sum no less than 5 percent of the Bid Amount in the form of a certified check or bank check or a bid bond issued by a surety acceptable to Owner.
- B. Endorse the Bid Bond in the name of the Owner as obligee, signed and sealed by the principal (Contractor) and surety.
- C. The security deposit will be returned after delivery to the Owner of the required Performance and Payment Bond(s) by the accepted bidder.
- D. Include the cost of bid security in the Bid Amount.
- E. After a bid has been accepted, all securities will be returned to the respective bidders .
- F. If no contract is awarded, all security deposits will be returned.

**6.02 PERFORMANCE ASSURANCE**

- A. Owner shall have the right to require Contractor to furnish a performance and payment bond in the full amount of the contract price on a form and by a surety approved by Owner. Bidders shall state the premium cost for the bond and name of surety in the blank spaces provided in the proposal form. If Owner requires such bond, the premium cost given in proposal will be paid by Owner. If Owner required a bond and a surety approved by Owner is unwilling to execute such bond with Contractor as principal, Owner may, at its election, reject the proposal or terminate the contract.

**6.03 INSURANCE**

- A. Contractor will carry insurance during performance of the Contract. Bidders shall state the limits of such insurances which they will carry and the names of the insurance carriers in the blank spaces provided in the proposal form.
- B. Contractor must provide Owner with evidence of the following minimum insurance requirements. In no way do these minimum requirements limit the liability assumed elsewhere in this Agreement.
  - 1. Workers' Compensation and Employer's Liability
    - a. Statutory requirements in Texas to include all areas involved in operations covered under the contract.
    - b. Coverage "B" - Employer's Liability, Limit - \$500,000.
    - c. Must include Waiver of Subrogation rights against Owner.
  - 2. Commercial General Liability
    - a. Commercial General Liability form, including Premises/ Operations, Independent Contractors, Products - Completed Operations, Broad Form Property Damage (including Completed Operations).

- b. Contractual Liability, Blanket basis insuring the liability assumed under this contract.
  - c. Limits of Liability: Bodily Injury, \$1,000,000 each occurrence, \$2,000,000 aggregate; Products - Completed Operations, \$2,000,000 aggregate; Personal Injury, \$1,000,000 each occurrence, \$1,000,000 aggregate; and Property Damage, \$1,000,000 each occurrence, \$1,000,000 aggregate.
- 3. Commercial Automobile Liability
  - a. Commercial Automobile Liability form, including all Owned, Non-Owned and Hired Vehicles.
  - b. Limits of Liability: Bodily Injury, \$1,000,000 each person, \$1,000,000 each occurrence, Property Damage, \$1,000,000 each occurrence.
- 4. Umbrella Liability
  - a. Such insurance shall provide coverage with limits of not less than \$1,000,000 per occurrence, \$1,000,000 aggregate, in excess of the underlying coverages listed above.
- 5. Additional Requirements
  - a. Owner shall be included as an Additional Insured on all coverages listed above.
  - b. Contractor shall require the same minimum insurance requirements, as listed above, from its subcontractors and suppliers and they shall also comply with the additional requirements listed herein.
  - c. All insurance coverages required as herein set forth shall be at the sole cost and expense of contractor, subcontractor, or suppliers and all deductibles shall be assumed by, for the account of, and at the sole risk of said contractor, subcontractor or suppliers.
  - d. Except where prohibited by law, all insurance policies shall contain provisions that the insurance companies waive the rights of recovery of subrogation against Owner, its agents, servants, invitees, employees, co-lessees, co-venturers, affiliated companies and their insurers.
  - e. A Certificate of Insurance evidencing all the above must be presented to Owner prior to commencement of the Work.
  - f. The cancellation provision of such Certificate of Insurance shall provide as follows:
    - 1) "To be effective as to certificate holder, the issuing companies must provide to the below named certificate holder ten days' written notice prior to any cancellation or material modification of the above referenced policies before the expiration dates thereof."

#### **6.04 BID FORM REQUIREMENTS**

- A. Complete all requested information in the Bid Form and Appendices.

#### **6.05 FEES FOR CHANGES IN THE WORK**

- A. Include in the Bid Form, the overhead and profit fees on own Work and Work by subcontractors, applicable for Changes in the Work, whether additions to or deductions from the Work on which the Bid Amount is based.
- B. Include in the Bid Form, the fees proposed for subcontract work for changes (both additions and deductions) in the Work. Contractor shall apply fees as noted, to the subcontractor's gross (net plus fee) costs on additional work.

**6.06 SELECTION AND AWARD OF Alternates**

- A. Indicate variation of bid price for Alternates listed on the Bid Form. Unless otherwise indicated, indicate Alternates as a difference in bid price by adding to or deducting from the base bid price.
- B. Bids will be evaluated on the total of the base bid price and all of the Alternates. After determination of the successful bidder, consideration will be given to which Alternates will be included in the Work.

**PART 7****7.01 DURATION OF OFFER**

- A. Bids shall remain open to acceptance and shall be irrevocable for a period of sixty (60) days after the bid closing date.

**7.02 ACCEPTANCE OF OFFER**

- A. Owner reserves the right to reject any or all bids submitted, to enter into negotiations with any bidder hereunder; to alter the Contract by agreement in writing with the successful bidder; or to take such other steps as may insure its complete freedom of action in selecting the successful bidder, all without any obligation or liability whatsoever to any bidder hereunder.
- B. After acceptance by Owner, Owner will issue to the successful bidder, a written Notice To Proceed.

**END OF SECTION**

**BID FORM****THE PROJECT AND THE PARTIES****1.01 TO:**

A. AG Sunflower Ridge, LLC (Owner)

**1.02 FOR:**

A. Sunflower Ridge Unit 2

**1.03 DATE: \_\_\_\_\_ (Bidder to enter date)****1.04 SUBMITTED BY: (Bidder to enter name and address)**

- A. Bidder's Full Name: \_\_\_\_\_
1. Address: \_\_\_\_\_
  2. City, State, Zip: \_\_\_\_\_
  3. The Undersigned proposes to furnish all labor, services, materials, tools and necessary equipment for the construction of various improvements and to perform the work required at the locations set out by the Plans and Specifications, in strict accordance with the Contract Documents.
  4. In submitting this Bid, it is understood that this Bid may not be altered or withdrawn for a minimum of 90 calendar days, and that the Owner has reserved the right to reject any and all Bids.
  5. The Undersigned certifies that this Bid is made in good faith, without collusion or connection with any other person, persons, partnership, company, firm, association, or corporation offering Bids on this work, for the following sum or prices to wit:

**1.05 DECLARATION**

- A. The undersigned hereby declares that he has visited the site and has carefully examined the Drawings, Specifications, Contract Documents and Bidding Documents related to the work covered by his Bid.

**1.06 OFFER**

- A. Having examined the Place of The Work and all matters referred to in the Instructions to Bidders and the Contract Documents for the above mentioned project, we, the undersigned, hereby offer to enter into a Contract to perform the Work for the Unit Prices listed in this bid form of:
- B. \_\_\_\_\_ dollars  
( \$ \_\_\_\_\_ ), in lawful money of the United States of America.
- C. We have included the required security Bid Bond as required by the Instruction to Bidders.
- D. All applicable federal taxes are included and State of Texas taxes are included in the Bid Price.
- E. All Cash and Contingency Allowances described in Section 01 2100 - Allowances are included in the Bid Sum.



**1.07 ACCEPTANCE**

- A. This offer shall be open to acceptance and is irrevocable for ninety days from the bid closing date.
- B. If this bid is accepted by Owner within the time period stated above, we will:
1. Execute the Agreement within seven days of receipt of Notice of Award or acceptance of this bid.
  2. Furnish the required bonds within seven days of receipt of Notice of Award or acceptance of this bid.
  3. Commence work within \_\_\_\_\_ days after written Notice to Proceed or acceptance of this bid. Completion time includes \_\_\_\_\_ days for adverse weather conditions.
- C. If this bid is accepted within the time stated, and we fail to commence the Work or we fail to provide the required Bond(s), the security deposit shall be forfeited as damages to Owner by reason of our failure, limited in amount to the lesser of the face value of the security deposit or the difference between this bid and the bid upon which a Contract is signed.
- D. In the event our bid is not accepted within the time stated above, the required security deposit shall be returned to the undersigned, in accordance with the provisions of the Instructions to Bidders; unless a mutually satisfactory arrangement is made for its retention and validity for an extended period of time.

**1.08 CONTRACT TIME**

- A. If this Bid is accepted, we will complete the Work in [\_\_\_\_\_] calendar days from Notice to Proceed or acceptance of this bid.

**1.09 UNIT PRICES**

- A. The following are Unit Prices for specific portions of the Work as listed. The following is the list of Unit Prices:

**B. ITEM DESCRIPTION - UNIT - QUANTITY x UNIT PRICE = ITEM VALUE**

C.	_____	-	_____	-	_____	x \$	_____	= \$	_____
D.	_____	-	_____	-	_____	x \$	_____	= \$	_____
E.	_____	-	_____	-	_____	x \$	_____	= \$	_____
F.	_____	-	_____	-	_____	x \$	_____	= \$	_____
G.	_____	-	_____	-	_____	x \$	_____	= \$	_____
H.	_____	-	_____	-	_____	x \$	_____	= \$	_____
I.	_____	-	_____	-	_____	x \$	_____	= \$	_____
J.	_____	-	_____	-	_____	x \$	_____	= \$	_____
K.	_____	-	_____	-	_____	x \$	_____	= \$	_____
L.	_____	-	_____	-	_____	x \$	_____	= \$	_____
M.	_____	-	_____	-	_____	x \$	_____	= \$	_____
N.	_____	-	_____	-	_____	x \$	_____	= \$	_____
O.	_____	-	_____	-	_____	x \$	_____	= \$	_____
P.	_____	-	_____	-	_____	x \$	_____	= \$	_____
Q.	_____	-	_____	-	_____	x \$	_____	= \$	_____

R. \_\_\_\_\_ - \_\_\_\_\_ - \_\_\_\_\_ x \$ \_\_\_\_\_ = \$ \_\_\_\_\_  
 S. \_\_\_\_\_ - \_\_\_\_\_ - \_\_\_\_\_ x \$ \_\_\_\_\_ = \$ \_\_\_\_\_  
 T. \_\_\_\_\_ - \_\_\_\_\_ - \_\_\_\_\_ x \$ \_\_\_\_\_ = \$ \_\_\_\_\_  
 U. \_\_\_\_\_ - \_\_\_\_\_ - \_\_\_\_\_ x \$ \_\_\_\_\_ = \$ \_\_\_\_\_

**1.10 CHANGES TO THE WORK**

- A. When Engineer or Owner establishes that the method of valuation for Changes in the Work will be net cost plus a percentage fee in accordance with General Conditions, our percentage fee will be:
1. \_\_\_\_\_ percent overhead and profit on the net cost of our own Work;
  2. \_\_\_\_\_ percent on the cost of work done by any Subcontractor.
- B. On work deleted from the Contract, our credit to Owner shall be Engineer-approved net cost plus \_\_\_\_\_ of the overhead and profit percentage noted above.

**1.11 ADDENDA**

- A. The following Addenda have been received. The modifications to the Bid Documents noted below have been considered and all costs are included in the Bid Price.
1. Addendum # \_\_\_\_\_ Dated \_\_\_\_\_.
  2. Addendum # \_\_\_\_\_ Dated \_\_\_\_\_.
  3. Addendum # \_\_\_\_\_ Dated \_\_\_\_\_.
  4. Addendum # \_\_\_\_\_ Dated \_\_\_\_\_.
  5. Addendum # \_\_\_\_\_ Dated \_\_\_\_\_.
  6. Addendum # \_\_\_\_\_ Dated \_\_\_\_\_.
  7. Addendum # \_\_\_\_\_ Dated \_\_\_\_\_.

**1.12 BID FORM SUPPLEMENTS**

- A. The following information is included with Bid submission:
1. Subcontractors: \_\_\_\_\_, \_\_\_\_\_, \_\_\_\_\_.
  2. Unit Prices: \_\_\_\_\_, \_\_\_\_\_, \_\_\_\_\_.
  3. Alternates: \_\_\_\_\_, \_\_\_\_\_, \_\_\_\_\_.
- B. The following Supplements are attached to this Bid Form and are considered an integral part of this Bid Form:
1. Document 00 4322 - Unit Prices Form: Include a listing of unit prices specifically requested by Contract Documents.
  2. Document 00 4323 - Alternatives: Include the cost variations to the Bid Price applicable to the Work.
  3. Document 00 4327 - Separate Prices Break-Out Form: Include a listing of separate prices as specifically requested in Contract Documents.
  4. Document 00 4336 - Proposed Subcontractors Form: Include the names of Subcontractors and the portions of the Work they will perform.
  5. Document 00 4373 - Proposed Schedule of Values Form identifies the Bid Sum segmented into portions as requested.
- C. Bonds:
1. Payment and performance bonds running to the Owner pursuant to Article 1.02, INSTRUCTION FOR BIDDERS.
    - a. Premium: \_\_\_\_\_

b. Name and Address of Surety:

1) \_\_\_\_\_

2) \_\_\_\_\_

D. Insurance

1. Insurance pursuant to Article 1.16, INSTRUCTION FOR BIDDERS (show normal coverage).

a. Workmen's Compensation Insurance complying with the laws of the state:

1) Carrier: \_\_\_\_\_

b. Employer's Liability Insurance:

1) Carrier: \_\_\_\_\_

2) \$ \_\_\_\_\_

c. Comprehensive General Liability and Property Damage Insurance:

1) Carrier: \_\_\_\_\_

2) \$ \_\_\_\_\_

d. Comprehensive Automobile Liability and Property Damage Insurance:

1) Carrier: \_\_\_\_\_

2) \$ \_\_\_\_\_

### 1.13 BID FORM SIGNATURE(S)

A. The Corporate Seal of

1. \_\_\_\_\_

2. (Bidder - print the full name of your firm)

B. was hereunto affixed in the presence of:

1. \_\_\_\_\_

2. (Authorized signing officer, Title)

3. (Seal)

4. \_\_\_\_\_

5. (Authorized signing officer, Title)

(Seal)

**END OF SECTION**

**CONTRACTING FORMS AND SUPPLEMENTS**

**PART 1 GENERAL**

**1.01 AGREEMENT AND CONDITIONS OF THE CONTRACT**

**1.02 FORMS**

- A. Use the following forms for the specified purposes unless otherwise indicated elsewhere in Contract Documents.

**PART 2 PRODUCTS - NOT USED**

**PART 3 EXECUTION - NOT USED**

**END OF SECTION**

**AGREEMENT****PART 1 GENERAL****1.01 AGREEMENT FORM**

- A. The "Standard Form of Agreement Between Owner and Contractor on the Basis of a Stipulated Price" where the Basis of Payment is a Stipulated Sum, prepared by Engineers Joint Contract Documents Committee (EJCDC No. C-520 - 2018 Edition), will be the form used as a Contract for this Project.
- B. A copy of the Document is available for review at the office of the Engineer. It shall be construed that this Document is included in this Project Manual and shall apply to every Section as if written in full therein.

**END OF SECTION**

**PRICE AND PAYMENT PROCEDURES****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Procedures for preparation and submittal of applications for progress payments.
- B. Documentation of changes in Contract Sum and Contract Time.
- C. Change procedures.
- D. Correlation of Contractor submittals based on changes.
- E. Procedures for preparation and submittal of application for final payment.

**1.02 RELATED REQUIREMENTS**

- A. Section 00 5000 - Contracting Forms and Supplements.
- B. Document 00 5200 - Agreement Form .
- C. Document 00 7200 - General Conditions and Document 00 7300 - Supplementary Conditions.
- D. Document 00 7300 - Supplementary Conditions.
- E. Section 01 2100 - Allowances.
- F. Section 01 2200 - Unit Prices.
- G. Section 01 7000 - Execution and Closeout Requirements.

**1.03 SCHEDULE OF VALUES**

- A. Electronic media printout including equivalent information will be considered in lieu of standard form specified; submit draft to Engineer for approval.
- B. Submit Schedule of Values in duplicate within 15 days after date of Owner-Contractor Agreement.
- C. Format: Utilize the Table of Contents of this Project Manual. Identify each line item with number and title of the specification section. Identify site mobilization.
- D. Include in each line item, the amount of Allowances specified in this section. For unit cost Allowances, identify quantities taken from Contract Documents multiplied by the unit cost to achieve the total for the item.
- E. Include within each line item, a direct proportional amount of Contractor's overhead and profit.
- F. Revise schedule to list approved Change Orders, with each Application For Payment.

**1.04 APPLICATIONS FOR PROGRESS PAYMENTS**

- A. Payment Period: Submit at intervals stipulated in the Agreement.
- B. Electronic media printout including equivalent information will be considered in lieu of standard form specified; submit sample to Engineer for approval.
- C. Forms filled out by hand will not be accepted.
- D. For each item, provide a column for listing each of the following:
  - 1. Item Number.
  - 2. Description of work.
  - 3. Scheduled Values.
  - 4. Previous Applications.

5. Work in Place and Stored Materials under this Application.
  6. Authorized Change Orders.
  7. Total Completed and Stored to Date of Application.
  8. Percentage of Completion.
  9. Balance to Finish.
  10. Retainage.
- E. Execute certification by signature of authorized officer.
- F. Use data from approved Schedule of Values. Provide dollar value in each column for each line item for portion of work performed and for stored products.
- G. List each authorized Change Order as a separate line item, listing Change Order number and dollar amount as for an original item of work.
- H. Submit a minimum of five copies of each Application for Payment.
- I. Include the following with the application:
1. Transmittal letter.
  2. Construction progress schedule, revised and current.
  3. Current construction photographs, if required.
  4. Partial release of liens from major subcontractors and vendors.
  5. Any waivers.
  6. Project record documents, for review by Owner which will be returned to the Contractor.
  7. Affidavits attesting to off-site stored products.
- J. When Engineer requires substantiating information, submit data justifying dollar amounts in question. Provide one copy of data with cover letter for each copy of submittal. Show application number and date, and line item by number and description.

#### **1.05 MODIFICATION PROCEDURES**

- A. Submit name of the individual authorized to receive change documents and who will be responsible for informing others in Contractor's employ or subcontractors of changes to Contract Documents.
- B. For minor changes not involving an adjustment to the Contract Sum or Contract Time, Engineer will issue instructions directly to Contractor.
- C. For other required changes, Engineer will issue a document signed by Owner instructing Contractor to proceed with the change, for subsequent inclusion in a Change Order.
1. The document will describe the required changes and will designate method of determining any change in Contract Sum or Contract Time.
  2. Promptly execute the change.
- D. For changes for which advance pricing is desired, Engineer will issue a document that includes a detailed description of a proposed change with supplementary or revised drawings and specifications and the period of time during which the requested price will be considered valid. Contractor shall prepare and submit a fixed price quotation within five days.
- E. Contractor may propose a change by submitting a request for change to Engineer, describing the proposed change and its full effect on the work, with a statement describing the reason for the change, and the effect on the Contract Sum and Contract Time with full documentation. Document any requested substitutions in accordance with Section 01 6000.

- F. Computation of Change in Contract Amount: As specified in the Agreement and Conditions of the Contract.
  - 1. For change requested by Engineer for work falling under a fixed price contract, the amount will be based on Contractor's price quotation.
  - 2. For change requested by Contractor, the amount will be based on the Contractor's request for a Change Order as approved by Engineer.
  - 3. For pre-determined unit prices and quantities, the amount will be based on the fixed unit prices.
  - 4. For change ordered by Engineer without a quotation from Contractor, the amount will be determined by Engineer based on the Contractor's substantiation of costs as specified for Time and Material work.
- G. Substantiation of Costs: Provide full information required for evaluation.
  - 1. On request, provide the following data:
    - a. Quantities of products, labor, and equipment.
    - b. Taxes, insurance, and bonds.
    - c. Overhead and profit.
    - d. Justification for any change in Contract Time.
    - e. Credit for deletions from Contract, similarly documented.
  - 2. Support each claim for additional costs with additional information:
    - a. Origin and date of claim.
    - b. Dates and times work was performed, and by whom.
    - c. Time records and wage rates paid.
    - d. Invoices and receipts for products, equipment, and subcontracts, similarly documented.
  - 3. For Time and Material work, submit itemized account and supporting data after completion of change, within time limits indicated in the Conditions of the Contract.
- H. Execution of Change Orders: Engineer will issue Change Orders for signatures of parties as provided in the Conditions of the Contract.
- I. After execution of Change Order, promptly revise Schedule of Values and Application for Payment forms to record each authorized Change Order as a separate line item and adjust the Contract Sum.
- J. Promptly revise progress schedules to reflect any change in Contract Time, revise sub-schedules to adjust times for other items of work affected by the change, and resubmit.
- K. Promptly enter changes in Project Record Documents.

#### **1.06 APPLICATION FOR FINAL PAYMENT**

- A. Prepare Application for Final Payment as specified for progress payments, identifying total adjusted Contract Sum, previous payments, and sum remaining due.
- B. Application for Final Payment will not be considered until the following have been accomplished:
  - 1. All closeout procedures specified in Section 01 7000.
  - 2. All plan of record drawings are provided.
  - 3. All warranty documents are provided.
  - 4. All owner's manuals are provided.
  - 5. All as-built drawings for public improvements are provided.



6. All testing results are provided.
7. All observation records are provided.
8. The owner's punch list has been completed.

**PART 2 PRODUCTS - NOT USED**

**PART 3 EXECUTION - NOT USED**

**END OF SECTION**

**DEFINITIONS****PART 1 - GENERAL****1.01 SUMMARY**

- A. References to standards, codes, Specifications, recommendations and regulations refer to the latest edition or printing prior to the date of issue of the Contract Documents.
- B. Applicable portions of standards listed that are not in conflict with Contract Documents are hereby made a part of the Specifications.
- C. Modifications or exceptions to Standards shall be considered as amendments and unmodified portions shall remain in full effect. In cases of discrepancies between standards, the more stringent requirements shall govern.

**1.02 DEFINITIONS**

- A. Furnish: To supply, deliver, unload, and inspect for damage.
- B. Install: To unpack, assemble, erect, apply, place, finish, cure, protect, clean, start up, and make ready for use.
- C. Product: Material, machinery, components, equipment, fixtures, and systems forming the work result. Not materials or equipment used for preparation, fabrication, conveying, or erection and not incorporated into the work result. Products may be new, never before used, or re-used materials or equipment.
- D. Project Manual: The book-sized volume that includes the procurement requirements (if any), the contracting requirements, and the specifications.
- E. Provide: To furnish and install.
- F. Supply: Same as Furnish.

**1.03 ABBREVIATIONS AND NAMES**

- A. The following acronyms or abbreviations as referenced in contract documents are defined to mean the associated names. Names are subject to change, and are believed to be, but are not assured to be, accurate and up-to-date as of date of contract documents:
  - 1. AA Aluminum Association
  - 2. AAMA American Architectural Manufacturers Association
  - 3. AAN American Association of Nurserymen
  - 4. AASHTO American Association of State Highway and Transportation Officials
  - 5. AATCC American Association of Textile Chemists and Colorists
  - 6. ACI American Concrete Institute
  - 7. ACIL American Council of Independent Laboratories
  - 8. ACPA American Concrete Pipe Association
  - 9. ADC Air Diffusion Council
  - 10. AFBMA Anti-Friction Bearing Manufacturers Association, Inc.
  - 11. AGA American Gas Association
  - 12. AHAM Association of Home Appliance Manufacturers
  - 13. AI Asphalt Institute
  - 14. AIA American Institute of Architects
  - 15. AIA American Insurance Association
  - 16. AISC American Institute of Steel Construction
  - 17. AISI American Iron and Steel Institute

18.	AITC	American Institute of Timber Construction
19.	ALSC	American Lumber Standards Committee
20.	AMCA	Air Movement and Control Association
21.	ANSI	American National Standards Institute
22.	APA	American Plywood Association
23.	APA	American Parquet Association
24.	ARI	Air Conditioning and Refrigeration Institute
25.	ASC	Adhesive and Sealant Council
26.	ASCE	American Society of Civil Engineers
27.	ASHREA	Architectural Society of Heating, Refrigerating and Air-Conditioning Engineers
28.	ASME	American Society of Mechanical Engineers
29.	ASPE	American Society of Plumbing Engineers
30.	ASSE	American Society of Sanitary Engineers
31.	ASTM	American Society for Testing and Materials
32.	AWI	Architectural Woodwork Institute
33.	AWPA	American Wood-Preservers' Association
34.	AWPB	American Wood-Preservers Bureau
35.	AWS	American Welding Society
36.	AWWA	American Water Works Association
37.	BHMA	Builders Hardware Manufacturers Association
38.	BIA	Brick Institute of America
39.	BIFMA	Business and Institutional Furniture Manufacturer's Association
40.	CBM	Certified Ballast Manufacturers
41.	CDA	Copper Development Association
42.	CE	Corps of Engineers (U.S. Dept. of the Army)
43.	CFFA	Chemical Fabrics and Film Association, Inc.
44.	CFR	Code of Federal Regulations
45.	CGA	Compressed Gas Association
46.	CISPI	Cast Iron Soil Pipe Institute
47.	CLFMI	Chain Link Fence Manufacturer's Institute
48.	CPSC	Consumer Product Safety Commission
49.	CRA	California Redwood Association
50.	CRI	Carpet and Rug Institute
51.	CRSI	Concrete Reinforcing Steel Institute
52.	CS	Commercial Standard of NBS (U.S. Dept. of Commerce)
53.	CTI	Ceramic Tile Institute
54.	DHI	Door and Hardware Institute
55.	DLPA	Decorative Laminate Products Association
56.	DOC	Department of Commerce
57.	DOT	Department of Transportation
58.	EIA	Electronic Industries Association
59.	EPA	Environmental Protection Agency
60.	FAA	Federal Aviation Administration (U.S. Dept of Transportation)
61.	FCC	Federal Communications Commission
62.	FCI	Fluid Controls Institute
63.	FGMA	Flat Glass Marketing Association

64.	FHA	Federal Housing Administration (U.S. Dept. of HUD)
65.	FM	Factory Mutual System
66.	FS	Federal Specification (General Services Admin.)
67.	FTI	Facing Tile Institute
68.	GA	Gypsum Association
69.	GSA	General Services Administration
70.	HPMA	Hardwood Plywood Manufacturers Association
71.	IEEE	Institute of Electrical and Electronic Engineers, Inc.
72.	IESNA	Illuminating Engineering Society of North America
73.	IGCC	Insulating Glass Certification Council
74.	ILI	Indiana Limestone Institute of America
75.	IMIAC	International Masonry Industry All-Weather Council
76.	IRI	Industrial Risk Insurance
77.	ISA	Instrument Society of America
78.	MBMA	Metal Building Manufacturer's Association
79.	MCAA	Mechanical Contractors Association of America
80.	MIA	Marble Institute of America
81.	MIL	Military Standardization Documents (U.S. Dept. of Defense)
82.	ML/SFA	Metal Lath/Steel Framing Association
83.	MSS	Manufacturers Standardization Society of the Valve and Fittings Industry
84.	NAAMM	National Association of Architectural Metal Manufacturers
85.	NBGQA	National Building Granite Quarries Association
86.	NBS	National Bureau of Standards (U.S. Dept. of Commerce)
87.	NCMA	National Concrete Masonry Association
88.	NCRPM	National Council on Radiation Protection and Measurement
89.	NEC	National Electrical Code (by NFPA)
90.	NECA	National Electrical Contractors Association
91.	NEII	National Elevator Industry, Inc.
92.	NEMA	National Electrical Manufacturers Association
93.	NFPA	National Fire Protection Association
94.	NFPA	National Forest Products Association
95.	NHLA	National Hardwood Lumber Association
96.	NKCA	National Kitchen Cabinet Association
97.	NOFMA	National Oak Flooring Manufacturers Association
98.	NPA	National Particleboard Association
99.	NPCA	National Paint and Coatings Association
100.	NRCA	National Roofing Contractors Association
101.	NRMCA	National Ready-Mix Concrete Association
102.	NSF	National Sanitation Foundation
103.	NSSEA	National School Supply and Equipment Association
104.	NTMA	National Terrazzo and Mosaic Association
105.	NWMA	National Woodwork Manufacturers Association
106.	OSHA	Occupational Safety Health Administration (U.S. Dept. of Labor)
107.	PCA	Portland Cement Association
108.	PCI	Prestressed Concrete Institute
109.	PDI	Plumbing and Drainage Institute

110. PEI	Porcelain Enamel Institute
111. PS	Product Standard of NBS (U.S. Dept. of Commerce)
112. RFCI	Resilient Floor Covering Institute
113. RIS	Redwood Inspection Service (Grading Rules)
114. SAMA	Scientific Apparatus Makers Association
115. SDI	Steel Deck Institute
116. SGCC	Safety Glazing Certification Council
117. SHLMA	Southern Hardwood Lumber Manufacturers Association
118. SIGMA	Sealed Insulating Glass Manufacturers Association
119. SJI	Steel Joist Institute
120. SMACNA	Sheet Metal and Air Conditioning Contractors' National Association
121. SPIB	Southern Pine Inspection Bureau (Grading Rules)
122. SPIB	Southern Pine Inspection Bureau
123. SSPC	Steel Structures Painting Council
124. SWI	Steel Window Institute
125. TCA	Tile Council of America
126. TIMA	Thermal Insulation Manufacturers Association
127. TPI	Truss Plate Institute
128. TxDOT	Texas Department of Transportation
129. UBC	Uniform Building Code
130. UL	Underwriters Laboratories
131. USDA	United States Department of Agriculture
132. USPS	United States Postal Service
133. WCLIB	West Coast Lumber Inspection Bureau (Grading Rules)
134. WRI	Wire Reinforcement Institute
135. WSC	Water Systems Council
136. WSFI	Wood and Synthetic Flooring Institute
137. WWPA	Western Wood Products Association (Grading Rules)
138. WWPA	Woven Wire Products Association

#### 1.04 GENERAL SPECIFICATION ABBREVIATIONS

A. Cubic Foot	cu. ft.
B. Degree	deg.
C. Diameter	dia.
D. Feet or Foot	ft.
E. Inch	in.
F. Inside Diameter	i.d.
G. Kips (1000 pounds)	K
H. Millimeter	mm
I. Ounce	oz.
J. Outside Diameter	o.d.
K. Pound	lb.
L. Pounds per Cubic Foot	pcf

- M. Pounds per Square Foot      psf
- N. Pounds per Square Inch      psi
- O. Square Foot                      sq. ft.
- P. Square Inch                      sq. in.

**1.05 REFERENCE STANDARDS - GENERAL**

- A. Publications of organizations and societies listed in individual Specification Sections shall be considered integral with Contract Documents to extent referenced.
- B. Publications are referred to in text by basic designation only with organizations and societies referenced by abbreviations.
- C. When standard is referenced to in individual Specification Section but is not listed in this Section by title and date, it shall be considered to be latest revision at date of Project Manual issuance.
- D. Make available at site, copies of referenced documents as Engineer or Owner may request.
- E. Following listings include full title and applicable revision date.

**1.06 REFERENCE STANDARDS**

- A. American Concrete Institute (ACI):
  - 1. 211.1-81 (Revised 1985), Standard Practice for Selecting Proportions for Normal, Heavyweight and Mass Concrete.
  - 2. 211.2-81, Standard Practice for Selecting Proportions for Structural Lightweight Concrete.
  - 3. 211.3-75 (Revised 1987), Standard Practice for Selecting Proportions for No-Slump Concrete.
  - 4. 301-84 (Revised 1987), Specification for Structural Concrete for Buildings.
  - 5. 304R-85, Guide for Measuring, Mixing, Transporting, and Placing Concrete.
  - 6. 306R-77 (Revised 1982), Hot Weather Concreting.
  - 7. 306R-78 (Revised 1983), Cold Weather Concreting.
  - 8. 308-81 (Revised 1986), Standard Practice for Curing Concrete.
  - 9. 309-72 (Revised 1982), Standard Practice for Consolidation of Concrete.
  - 10. 315-80, Details and Detailing of Reinforced Concrete (included in SP-66).
  - 11. 318-83 (Revised 1986), Building Code Requirements for Reinforced Concrete.
  - 12. 347-78 (Reapproved 1984), Recommended Practice for Concrete Formwork.
  - 13. SP-66, ACI Detailing Manual (1980 Edition).
- B. American Institute of Steel Construction (AISC):
  - 1. Code of Standard Practice for Steel Buildings and Bridges (with Commentary), September 1, 1976.
  - 2. Manual of Steel Construction, Eighth Edition.
  - 3. Specification for the Design, Fabrication, and Erection of Structural Steel for Buildings (with Commentary), November 1, 1978.
  - 4. Specification for Structural Joints Using ASTM A325 or A490 Bolts, April 26, 1978.
- C. American Society for Testing and Materials (ASTM)
  - 1. A 36-84a, Specification for Structural Steel.
  - 2. A 47-84, Specification for Ferritic Malleable Iron Castings.

3. A 53-84a, Specification for Pipe, Steel, Black and Hot-Dipped, Zinc-Coated Welded and Seamless.
4. A 82-85, Specification for Steel Wire, Plain, for Concrete Reinforcement.
5. A 108-81, Specification for Steel Bars, Carbon, Cold-Finished, Standard Quality.
6. A 116-81, Specification for Zinc-Coated (Galvanized) Steel Woven Wire Fence Fabric.
7. A 123-84, Specification for Zinc (Hot-Galvanized) Coatings on Iron and Steel Products.
8. A 153-84, Specification for Zinc Coating (Hot-Dip) on Iron and Steel Hardware.
9. A 185-85, Specification for Steel Welded Wire, Fabric, Plain, for Concrete Reinforcement.
10. A 242-87, Specification for High-Strength Low-Alloy Structural Steel.
11. A 307-86, Specification for Carbon Steel Bolts and Studs, 6000 psi Tensile Strength.
12. A 325-86a, Specification for High-Strength Bolts for Structural Steel Joints.
13. A 370-86a, Methods and Definitions for Mechanical Testing of Steel Products.
14. A 416-86, Specification for Steel Strand, Uncoated Seven-Wire Stress-Relieved for Prestressed Concrete.
15. A 421-80 (1985), Specification for Uncoated Stress-Relieved Wire for Prestressed Concrete.
16. A 446-85, Specification for Steel Sheet, Zinc-Coated (Galvanized) by the Hot-Dip Process, Structural (Physical) Quality.
17. A 475-85, Specification for Zinc-Coated Steel Wire Strand.
18. A 490-85, Specification for Heat-Treated, Steel Structural Bolts 150 KSI Tensile Strength.
19. A 500-84, Specification for Cold-Formed Welded and Seamless Carbon Steel Structural Tubing in Rounds and Shapes.
20. A 501-84, Specification for Hot-Formed Welded and Seamless Carbon Steel Structural Tubing.
21. A 514-87a, Specification for High-Yield Strength, Quenched and Tempered Alloy Steel Plate, Suitable for Welding.
22. A 525-83, Specification for Steel Sheet, Zinc-Coated (Galvanized) by the Hot-Dip Process, General Requirements.
23. A 572-84, Specification for High-Strength Low-Alloy Columbium-Vanadium Steel of Structural Quality.
24. A 576-81, Specification for Steel Bars, Carbon, Hot-Wrought, Special Quality.
25. A 588-87, Specification for High-Strength Low-Alloy Structural Steel with 50,000 psi Minimum Yield Point to 4 in. Thick.
26. A 611-85, Specification for Steel, Sheet, Carbon, Cold-Rolled Structural Quality.
27. A 615-87, Specification for Deformed and Plain Billet-Steel Bars for Concrete Reinforcement.
28. A 706-84a, Specification for Low-Alloy Steel Deformed Bars for Concrete Reinforcement.
29. A 775-85, Specification for Epoxy-Coated Reinforcing Steel Bars.
30. A 633-85, Specification for Electrodeposited Coatings of Zinc on Iron and Steel.
31. C 31-85, Method for Making and Curing Concrete Test Specimens in the Field.
32. C 33-86, Specification for Concrete Aggregates.
33. C 39-86, Test for Compressive Strength of Cylindrical Concrete Specimens.
34. C 42-84a, Methods for Obtaining and Testing Drilled Cores and Sawed Beams of Concrete.
35. C 90-85, Specification for Hollow Load-Bearing Concrete Masonry Units.
36. C 91-86, Specification for Masonry Cement.

37. C 94-86, Specification for Ready-Mixed Concrete.
38. C 109-86, Test Method for Compressive Strength of Hydraulic Cement Mortars.
39. C 138-81, Test Method for Unit Weight, Yield and Air Content (Gravimetric) of Concrete.
40. C 140-75 (1980), Method of Sampling and Testing Concrete Masonry Units.
41. C 143-78, Test Method for Slump of Portland Cement Concrete.
42. C 144-87, Specification for Aggregate for Masonry Mortar.
43. C 150-85a, Specification for Portland Cement.
44. C 171-69 (1986), Specification for Sheet Materials for Curing Concrete.
45. C 172-82, Method of Sampling Freshly Mixed Concrete.
46. C 173-78, Test Method for Air Content of Freshly Mixed Concrete by the Volumetric Method.
47. C 190-85, Test Method for Tensile Strength of Hydraulic Cement Mortars.
48. C 207-79 (1984), Specification for Hydrated Lime for Masonry Purposes.
49. C 231-82, Test Method for Air Content of Freshly Mixed Concrete by the Pressure Method.
50. C 260-85, Specification for Air-Entraining Admixtures for Concrete.
51. C 270-86b, Specification for Mortar for Unit Masonry.
52. C 309-81, Specification for Liquid Membrane-Forming Compounds for Curing Concrete.
53. C 311-85, Method for Sampling and Testing Fly Ash or Natural Pozzolans for Use as a Mineral Admixture in Portland Cement Concrete.
54. C 330-87, Specification for Lightweight Aggregates for Structural Concrete.
55. C 404-87, Specification for Aggregates for Masonry Grout.
56. C 476-83, Specification for Grout for Reinforced and Non-reinforced Masonry.
57. C 494-86, Specification for Chemical Admixtures for Concrete.
58. C 567-85, Test Method for Unit Weight of Structural Lightweight Concrete.
59. C 595-86, Specification for Blended Hydraulic Cements.
60. C 618-85, Specification for Fly Ash and Raw or Calcined Natural Pozzolan for use as a Mineral Admixture in Portland Cement.
61. C 780-80 (1985), Method for Preconstruction and Construction Evaluation of Mortars for Plain and Reinforced Unit Masonry.
62. C 827-82, Test Method for Early Volume Change of Cementitious Mixtures.
63. C 938-80 (1985), Practice for Proportioning Grout Mixtures for Preplaced-Aggregate Concrete.
64. C 942-86, Test Method for Compressive Strength of Grouts for Preplaced-Aggregate Concrete in the Laboratory.
65. C 989-85a, Specification for Ground Iron Blast-Furnace Slag for Use in Concrete and Mortars.
66. D 98-80, Specification for Calcium Chloride.
67. D 994-71 (Reaffirmed 1982), Standard Specification for Preformed Expansion Joint Filler for Concrete (Bituminous Type).
68. D 1751-83, Specification for Preformed Expansion Joint Filler for Concrete Paving and Structural Construction (Non-extruding and Resilient Bituminous Types).
69. D 1752-84, Specification for Preformed Sponge Rubber and Cork Expansion Joint Fillers for Concrete Paving and Structural Construction.
70. D 2092-86, Recommended Practices for Preparation of Zinc-Coated Galvanized Steel Surfaces for Painting.



71. D 2240-86, Test for Rubber Property - Durometer Hardness.
  72. D 3034, Type PSM Poly(Vinyl Chloride) (PVC) Sewer Pipe and Fittings.
  73. E 164-81, Practice for Ultrasonic Contact Examination of Weldments.
  74. E 329-77 (1983), Recommended Practice for Inspection and Testing Agencies for Concrete, Steel, and Bituminous Materials as Used in Construction.
  75. E 447-84, Test Methods for Compressive Strength of Masonry Prisms.
- D. American Welding Society (AWS):
1. A5.1-81, Specification for Carbon Steel Covered Arc-Welding Electrodes.
  2. A5.5-81, Specification for Low-Alloy Steel Covered Arc-Welding Electrodes.
  3. A5.17-80, Specification for Carbon Steel Electrodes and Fluxes for Submerged Arc-Welding.
  4. A5.20-79, Specification for Carbon Steel Electrodes for Flux Cored Arc-Welding.
  5. B2.1-84, Welding Procedure and Performance Qualification.
  6. D1.1-86, Structural Welding Code - Steel.
  7. D1.3-81, Specification for Welding of Sheet Metal in Structures.
  8. D1.4-79, Structural Welding Code - Reinforcing Steel.
  9. QC-1-86, Standard for Qualification and Certification of Welding Inspectors.
- E. Prestressed Concrete Institute (PCI):
1. MNL 116-77, Manual for Quality Control for Plants and Production of Precast Prestressed Concrete Products.
  2. MNL 117-77, Manual for Quality Control of Plants and Production of Architectural Precast Concrete Products
- F. Steel Deck Institute (SDI):
1. Specification for Composite Steel Floor Deck, 1987.
  2. Specification for Non-Composite Steel Form Deck, 1987.
  3. Specification for Steel Roof Deck, 1987.
- G. Steel Joist Institute (SJI):
1. Recommended Code of Standard Practice for Steel Joists, November 4, 1985.
  2. Standard Specification for Joist Girders, November 4, 1985.
  3. Standard Specification for Longspan Steel Joists, LH-Series and Deep Longspan Steel Joists, DLH-Series, November 4, 1985.
  4. Standard Specification for Open Web Steel Joists, K-Series, November 1, 1985.

**END OF SECTION**

**TEMPORARY FACILITIES AND CONTROLS****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Temporary utilities.
- B. Temporary telecommunications services.
- C. Temporary sanitary facilities.
- D. Temporary Controls: Barriers, enclosures, and fencing.
- E. Security requirements.
- F. Vehicular access and parking.
- G. Waste removal facilities and services.
- H. Project identification sign.
- I. Field offices.

**1.02 RELATED REQUIREMENTS**

- A. Section 01 3553 - Security Procedures.
- B. Section 01 5100 - Temporary Utilities.
- C. Section 01 5213 - Field Offices and Sheds.
- D. Section 01 5500 - Vehicular Access and Parking.
- E. Section 01 5813 - Temporary Project Signage.

**1.03 TEMPORARY UTILITIES**

- A. See Section 01 5100 for additional requirements.
- B. Provide and pay for all electrical power, lighting, water, heating and cooling, and ventilation required for construction purposes.
- C. Existing facilities may be used with Owner approval.
- D. New permanent facilities may be used with Owner approval.
- E. Use trigger-operated nozzles for water hoses, to avoid waste of water.

**1.04 TELECOMMUNICATIONS SERVICES**

- A. Provide, maintain, and pay for telecommunications services to field office at time of project mobilization.
- B. Telecommunications services shall include:
  - 1. Windows-based personal computer dedicated to project telecommunications, with necessary software and laser printer.
  - 2. Telephone Land Lines: One line, minimum; one handset per line.
  - 3. Internet Connections: Minimum of one; T-1 line or faster.
  - 4. Email: Account/address reserved for project use.
  - 5. Facsimile Service: Minimum of one dedicated fax machine/printer, with dedicated phone line.
  - 6. Facsimile Service: Fax-to-email software on personal computer.
  - 7. Project web site.

**1.05 TEMPORARY SANITARY FACILITIES**

- A. Provide and maintain required facilities and enclosures. Provide at time of project mobilization.
- B. Maintain daily in clean and sanitary condition.
- C. At end of construction, return facilities to same or better condition as originally found.

**1.06 BARRIERS**

- A. Provide barriers to prevent unauthorized entry to construction areas, to prevent access to areas that could be hazardous to workers or the public, to allow for owner's use of site and to protect existing facilities and adjacent properties from damage from construction operations and demolition.
- B. Provide barricades and covered walkways required by governing authorities for public rights-of-way and for public access to existing building.
- C. Provide protection for plants designated to remain. Replace damaged plants.
- D. Protect non-owned vehicular traffic, stored materials, site, and structures from damage.
- E. Traffic Controls: To be installed and maintained per State or local requirements..

**1.07 FENCING**

- A. Construction: Commercial grade chain link fence.
- B. Provide 6 foot high fence around construction site; equip with vehicular and pedestrian gates with locks.

**1.08 SECURITY**

- A. See Section 01 3553 for additional requirements.
- B. Provide security and facilities to protect Work, existing facilities, and Owner's operations from unauthorized entry, vandalism, or theft.
- C. Coordinate security program with Owner.

**1.09 VEHICULAR ACCESS AND PARKING**

- A. Comply with regulations relating to use of streets and sidewalks, access to emergency facilities, and access for emergency vehicles.
- B. Coordinate access and haul routes with governing authorities and Owner.
- C. Provide and maintain access to fire hydrants, free of obstructions.
- D. Provide means of removing mud from vehicle wheels before entering streets.
- E. Existing on-site roads or public streets shall not be used for construction traffic.
- F. Provide temporary parking areas to accommodate construction personnel. When site space is not adequate, provide additional off-site parking.
- G. Do not allow vehicle parking on existing pavement.

**1.10 WASTE REMOVAL**

- A. See Section 01 7419 - Construction Waste Management and Disposal, for additional requirements.
- B. Provide waste removal facilities and services as required to maintain the site in clean and orderly condition.

- C. Provide containers with lids. Remove trash from site.
- D. If materials to be recycled or re-used on the project must be stored on-site, provide suitable non-combustible containers; locate containers holding flammable material outside the structure unless otherwise approved by the authorities having jurisdiction.
- E. Open free-fall chutes are not permitted. Terminate closed chutes into appropriate containers with lids.

**1.11 PROJECT IDENTIFICATION**

- A. Provide project identification sign of design, construction, and location approved by Owner.
- B. No other signs are allowed without Owner permission except those required by law.

**1.12 FIELD OFFICES**

- A. Office: Weathertight, with lighting, electrical outlets, heating, cooling equipment .
- B. Provide space for Project meetings, with table and chairs to accommodate 8 persons.
- C. Locate offices a minimum distance of 50 feet from existing and new structures.

**1.13 REMOVAL OF UTILITIES, FACILITIES, AND CONTROLS**

- A. Remove temporary utilities, equipment, facilities, materials, prior to Date of Substantial Completion inspection.
- B. Remove temporary underground installations to a minimum depth of 2 feet.
- C. Clean and repair damage caused by installation or use of temporary work.
- D. Restore existing facilities used during construction to original condition.
- E. Restore new permanent facilities used during construction to specified condition.

**PART 2 PRODUCTS - NOT USED****PART 3 EXECUTION - NOT USED**

**END OF SECTION**

**FIELD OFFICES AND SHEDS****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Temporary field offices for use of Contractor.
- B. Maintenance and removal.

**1.02 RELATED REQUIREMENTS**

- A. Section 01 5000 - Temporary Facilities and Controls:
- B. Section 01 5500 - Vehicular Access and Parking.

**1.03 USE OF EXISTING FACILITIES**

- A. Existing facilities shall not be used for field offices unless otherwise indicated.

**1.04 USE OF PERMANENT FACILITIES**

- A. Permanent facilities shall not be used for field offices unless otherwise indicated.

**PART 2 PRODUCTS****2.01 MATERIALS AND EQUIPMENT**

- A. Materials and Equipment: Serviceable, new or used, adequate for required purpose.

**2.02 CONSTRUCTION**

- A. Portable or mobile buildings, or buildings constructed with floors raised above ground, securely fixed to foundations, with steps and landings at entrance doors.
- B. Construction: Structurally sound, secure, weather tight enclosures for office. Maintain during progress of Work; remove when no longer needed.
- C. Temperature Transmission Resistance of Floors, Walls, and Ceilings: Compatible with occupancy requirements.
- D. Exterior Materials: Weather resistant, finished in one color.
- E. Interior Materials in Offices: Sheet type materials for walls and ceilings, prefinished or painted; resilient floors and bases.
- F. Fire Extinguishers: Appropriate type fire extinguisher at each office.

**2.03 Contractor OFFICE AND FACILITIES**

- A. Size: For Contractor's needs and to provide space for project meetings.
- B. Telephone: As specified in Section 01 5000.
- C. Furnishings in Meeting Area: Conference table and chairs to seat at least eight persons; racks and files for Contract Documents, submittals, and project record documents.
- D. Other Furnishings: Contractor's option.
- E. Sanitary Facilities: As specified in Section 01 5000.
- F. Drinking Fountain: Convenient access by workers.

**PART 3 EXECUTION****3.01 PREPARATION**

- A. Fill and grade sites for temporary structures to provide drainage away from buildings.

**3.02 INSTALLATION**

- A. Install office spaces ready for occupancy 15 days after date fixed in Notice to Proceed.
- B. Parking: Gravel parking area near field office.
- C. Employee Residential Occupancy: Not allowed on Owner's property.

**3.03 MAINTENANCE AND CLEANING**

- A. Weekly janitorial services for offices; periodic cleaning and maintenance for offices.
- B. Maintain approach walks free of mud, water, and snow.

**3.04 REMOVAL**

- A. At completion of Work remove buildings, foundations, utility services, and debris. Restore areas.

**END OF SECTION**

**TEMPORARY EROSION AND SEDIMENT CONTROL****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Prevention of erosion due to construction activities.
- B. Prevention of sedimentation of waterways, open drainage ways, and storm and sanitary sewers due to construction activities.
- C. Restoration of areas eroded due to insufficient preventive measures.
- D. Revegetation of disturbed areas.
- E. Performance bond.
- F. Compensation of Owner for fines levied by authorities having jurisdiction due to non-compliance by Contractor.

**1.02 RELATED REQUIREMENTS**

- A. Section 31 1000 - Site Clearing.
- B. Section 31 2200 - Grading.
- C. Section 31 2316 - Excavation.
- D. Section 31 2323 - Fill.
- E. Section 31 3700 - Riprap.

**1.03 REFERENCE STANDARDS**

- A. ASTM D4355/D4355M - Standard Test Method for Deterioration of Geotextiles by Exposure to Light, Moisture and Heat in a Xenon Arc Type Apparatus; 2014.
- B. ASTM D4491/D4491M - Standard Test Methods for Water Permeability of Geotextiles by Permittivity; 2017.
- C. ASTM D4533 - Standard Test Method for Trapezoid Tearing Strength of Geotextiles; 2011.
- D. ASTM D4632/D4632M - Standard Test Method for Grab Breaking Load and Elongation of Geotextiles; 2015a.
- E. ASTM D4751 - Standard Test Method for Determining Apparent Opening Size of a Geotextile; 2012.
- F. ASTM D4873 - Standard Guide for Identification, Storage, and Handling of Geosynthetic Rolls and Samples; 2002 (Reapproved 2009).
- G. EPA (NPDES) - National Pollutant Discharge Elimination System (NPDES), Construction General Permit; Current Edition.
- H. FHWA FLP-94-005 - Best Management Practices for Erosion and Sediment Control; 1995.
- I. USDA TR-55 - Urban Hydrology for Small Watersheds; USDA Natural Resources Conservation Service; 2009.
- J. Texas Commission on Environmental Quality - Storm Water Pollution Prevention Plan (SWPPP) requirements.
- K. State, local, County and Municipal SWPPP requirements.

**1.04 PERFORMANCE REQUIREMENTS**

- A. Comply with all requirements for erosion and sedimentation control, as specified for the Texas Pollutant Discharge Elimination System (TPDES), Phases I and II, under requirements for the Construction General Permit (CGP); current edition.
- B. Comply with all more stringent requirements of the County and municipal.
- C. Comply with all requirements of the SWPPP for erosion and sedimentation control.
- D. Best Management Practices Standard: FHWA FLP-94-005.
- E. Runoff Calculation Standard for Urban Areas: USDA TR-55.
- F. Develop and follow an Erosion and Sedimentation Prevention Plan and submit periodic inspection reports.
- G. Do not begin clearing, grading, or other work involving disturbance of ground surface cover until applicable permits have been obtained; furnish all documentation required to obtain applicable permits.
  - 1. Contractor will obtain permits and pay for securities required by authority having jurisdiction.
  - 2. Owner will withhold payment to Contractor equivalent to all fines resulting from non-compliance with applicable regulations.
- H. If required by the Owner, provide a Performance Bond covering erosion and sedimentation preventive measures only, in an amount equal to 100 percent of the cost of erosion and sedimentation control work.
- I. Timing: Put preventive measures in place as soon as possible before disturbance of surface cover and before precipitation occurs.
- J. Storm Water Runoff: Control increased storm water runoff due to disturbance of surface cover due to construction activities for this project.
  - 1. Prevent runoff into storm and sanitary sewer systems, including open drainage channels, in excess of actual capacity or amount allowed by authorities having jurisdiction, whichever is less.
  - 2. Anticipate runoff volume due to the most extreme short term and 24-hour rainfall events that might occur in 10 years.
- K. Erosion On Site: Minimize wind, water, and vehicular erosion of soil on project site due to construction activities for this project.
  - 1. Control movement of sediment and soil from temporary stockpiles of soil.
  - 2. Prevent development of ruts due to equipment and vehicular traffic.
  - 3. If erosion occurs due to non-compliance with these requirements, restore eroded areas at no cost to Owner.
- L. Erosion Off Site: Prevent erosion of soil and deposition of sediment on other properties caused by water leaving the project site due to construction activities for this project.
  - 1. Prevent windblown soil from leaving the project site.
  - 2. Prevent tracking of mud and sediment onto public roads outside site.
  - 3. Prevent mud and sediment from flowing onto sidewalks and pavements.
  - 4. If erosion occurs due to non-compliance with these requirements, restore eroded areas at no cost to Owner.



- M. Sedimentation of Waterways On Site: Prevent sedimentation of waterways on the project site, including rivers, streams, lakes, ponds, open drainage ways, storm sewers, and sanitary sewers.
  - 1. If sedimentation occurs, install or correct preventive measures immediately at no cost to Owner; remove deposited sediments; comply with requirements of authorities having jurisdiction.
  - 2. If sediment basins are used as temporary preventive measures, pump dry and remove deposited sediment after each storm.
- N. Sedimentation of Waterways Off Site: Prevent sedimentation of waterways off the project site, including rivers, streams, lakes, ponds, open drainage ways, storm sewers, and sanitary sewers.
  - 1. If sedimentation occurs, install or correct preventive measures immediately at no cost to Owner; remove deposited sediments; comply with requirements of authorities having jurisdiction.
- O. Open Water: Prevent standing water that could become stagnant.
- P. Maintenance: Maintain temporary preventive measures until permanent measures have been established.

## **PART 2 PRODUCTS**

### **2.01 MATERIALS**

- A. Mulch:
  - 1. Straw or hay.
  - 2. Wood waste, chips, or bark.
  - 3. Erosion control matting or netting.
- B. Grass Seed For Temporary Cover: Select a species appropriate to climate, planting season, and intended purpose. If same area will later be planted with permanent vegetation, do not use species known to be excessively competitive or prone to volunteer in subsequent seasons.
- C. Bales: Bound, rectangular straw bales.
- D. Bale Stakes:
  - 1. Steel U- or T-section.
  - 2. Wood.
  - 3. Minimum Length: 3 feet.
- E. Silt Fence Fabric: Polypropylene, polyethylene, or polyamide woven or nonwoven geotextile fabric resistant to common soil chemicals, mildew, and insects; non-biodegradable; in longest lengths possible; fabric including seams with the following minimum average roll lengths:
  - 1. Minimum Unit Weight: 4.5 oz/yd.
  - 2. Minimum Width: 36 inches.
  - 3. Average Opening Size: 30 U.S. Std. Sieve, maximum, when tested in accordance with ASTM D4751.
  - 4. Mullen Burst Strength: 190 lb/sq in.
  - 5. Permittivity:  $0.05 \text{ sec}^{-1}$ , minimum, when tested in accordance with ASTM D4491/D4491M.
  - 6. Ultraviolet Resistance: Retaining at least 70 percent of tensile strength, when tested in accordance with ASTM D4355/D4355M after 500 hours exposure.

7. Elongation: 15 to 30 percent, when tested in accordance with ASTM D4632/D4632M.
  8. Tear Strength: 55 pounds-force, minimum, when tested in accordance with ASTM D4533.
  9. Color: Manufacturer's standard, with embedment and fastener lines preprinted.
  10. Woven Wire Backing: 2"x4" 12 gauge (min.), welded wire, galvanized.
- F. Silt Fence Posts:
1. Steel Y- or T-section, with minimum mass of 1.25 lb per linear foot.
  2. Minimum Length: 4 feet.
  3. Galvanized or painted surface.
  4. Brindle Hardness: Greater than 140.
- G. Riprap: See Section 31 3700.
- H. Filter Bags: Polypropylene, polyethylene or polyamide woven fabric.
1. Unit Weight: 4 oz/sq yd.
  2. Mullen Burst Strength: Greater than 300 psi.
  3. Ultraviolet Resistance: Retaining at least 70 percent of tensile strength, when tested in accordance with ASTM D 4355 after 500 hours exposure.
  4. Filter Bag Fill: Washed pea gravel to coarse ground (0.31" to 0.75" diameter).
- I. Rock Berms
1. Rock: Clean, open graded, 3 to 5 inch diameter; high velocity areas should use 5 to 8 inch diameter.
  2. Woven wire; 20 gauge, maximum 1" opening, galvanized, secured with shoat rings.
- J. Stabilized Construction Entrance
1. Rock: 4" to 8", washed.
  2. Fabric: Geotextile specific for soil filtration; 6 oz/sq yd; Mullen burst rating of 140 lb/sq in, greater than #50 sieve opening size.

## **PART 3 EXECUTION**

### **3.01 EXAMINATION**

- A. Examine site and identify existing features that contribute to erosion resistance; maintain such existing features to greatest extent possible.

### **3.02 PREPARATION**

- A. Schedule work so that soil surfaces are left exposed for the minimum amount of time.

### **3.03 SCOPE OF PREVENTIVE MEASURES**

- A. In all cases, if permanent erosion resistant measures have been installed temporary preventive measures are not required.
- B. Construction Entrances: Traffic-bearing aggregate surface.
1. Width: 12 feet, minimum, or full width of driveway, whichever is greater.
  2. Length: 50 feet, minimum.
  3. Provide at each construction entrance from public right-of-way.
  4. Where necessary to prevent tracking of mud onto right-of-way, provide wheel washing area out of direct traffic lane, with drain into sediment trap or basin.
- C. Linear Sediment Barriers: Made of silt fences or rock riprap.
1. Provide linear sediment barriers as detailed on drawings with the following parameters:
    - a. Along downhill perimeter edge of disturbed areas, including soil stockpiles.

- b. Along the top of the slope or top bank of drainage channels and swales that traverse disturbed areas.
  - c. Along the toe of cut slopes and fill slopes.
  - d. Perpendicular to flow across the bottom of existing and new drainage channels and swales that traverse disturbed areas or carry runoff from disturbed areas; space at maximum of 200 feet apart.
  - e. Across the entrances to culverts that receive runoff from disturbed areas.
- D. Storm Drain Curb Inlet Sediment Trap: Protect each curb inlet using one of the following measures:
  - 1. Bagged gravel inlet protection: As detailed on drawings.
- E. Storm Drain Inlet: As detailed on drawings.
- F. Temporary Splash Pads: Stone aggregate over filter fabric; size to suit application; provide at downspout outlets and storm water outlets.
- G. Soil Stockpiles: Protect using one of the following measures:
  - 1. Cover with polyethylene film, secured by placing soil on outer edges.
  - 2. Cover with mulch at least 4 inches thickness of pine needles, sawdust, bark, wood chips, or shredded leaves, or 4 inches of straw or hay.
- H. Mulching: Use only for areas that may be subjected to erosion for less than 6 months.
  - 1. Wood Waste: Use only on slopes 3:1 or flatter; no anchoring required.
- I. Temporary Seeding: Use where temporary vegetated cover is required.

### 3.04 INSTALLATION

- A. Traffic-Bearing Aggregate Surface:
  - 1. Excavate minimum of 3 inches.
  - 2. Place geotextile fabric full width and length, with minimum 12 inch overlap at joints.
  - 3. Place and compact at least 6 inches of 1 1/2 to 3 1/2 inch diameter stone.
  - 4. Reference details on the drawings.
- B. Silt Fences:
  - 1. Install with top of fabric at nominal height and embedment indicated on drawings.
  - 2. Embed bottom of fabric in a trench on the upslope side of fence, with 6 inches of fabric laid flat on bottom of trench facing upslope; backfill trench and compact.
  - 3. Do not splice fabric width; minimize splices in fabric length; splice at post only, overlapping at least 18 inches, with extra post.
  - 4. Fasten fabric to steel posts using wire, nylon cord, or integral pockets.
  - 5. Wherever runoff will flow around end of barrier or over the top, provide temporary splash pad or other outlet protection; at such outlets in the run of the barrier, make barrier not more than 12 inches high with post spacing not more than 4 feet.
  - 6. Reference details on the drawings.
- C. Straw Bale Rows:
  - 1. Install bales in continuous rows with ends butting tightly, with one bale at each end of row turned uphill.
  - 2. Install bales so that bindings are not in contact with the ground.
  - 3. Embed bales at least 4 inches in the ground.
  - 4. Anchor bales with at least two stakes per bale, driven at least 18 inches into the ground; drive first stake in each bale toward the previously placed bale to force bales together.

5. Fill gaps between ends of bales with loose straw wedged tightly.
  6. Place soil excavated for trench against bales on the upslope side of the row, compacted.
- D. Mulching Over Large Areas:
1. Dry Straw and Hay: Apply 2-1/2 tons per acre; anchor using dull disc harrow or emulsified asphalt applied using same spraying machine at 100 gallons of water per ton of mulch.
  2. Wood Waste: Apply 6 to 9 tons per acre.
  3. Erosion Control Matting: Comply with manufacturer's instructions.
- E. Mulching Over Small and Medium Areas:
1. Dry Straw and Hay: Apply 4 to 6 inches depth.
  2. Wood Waste: Apply 2 to 3 inches depth.
  3. Pine Needles: Apply 2 to 3 inches depth.
  4. Erosion Control Matting: Comply with manufacturer's instructions.
- F. Temporary Seeding:
1. When hydraulic seeder is used, seedbed preparation is not required.
  2. When surface soil has been sealed by rainfall or consists of smooth undisturbed cut slopes, and conventional or manual seeding is to be used, prepare seedbed by scarifying sufficiently to allow seed to lodge and germinate.
  3. If temporary mulching was used on planting area but not removed, apply nitrogen fertilizer at 1 pound per 1000 sq ft.
  4. On soils of very low fertility, apply 10-10-10 fertilizer at rate of 12 to 16 pounds per 1000 sq ft.
  5. Incorporate fertilizer into soil before seeding.
  6. Apply seed uniformly; if using drill or cultipacker seeders place seed 1/2 to 1 inch deep.
  7. Irrigate as required to thoroughly wet soil to depth that will ensure germination, without causing runoff or erosion.
  8. Repeat irrigation as required until grass is established.
- G. Rock Berms
1. Layout the woven wire mesh sheathing perpendicular to the direction of runoff.
  2. Rock berm will have a minimum top width of 2 feet with side slopes being 2:1 (h:v) or flatter.
  3. Place rock along the sheathing to a height not less than 18".
  4. Wrap the wire sheathing around the rock and secure with tie wire so that the ends of the sheathing overlap a minimum of 2 inches.
  5. Tie the ends of the berm into the existing upslope grade and bury the berm 3 to 4 inches into existing ground slope.
- H. Stabilized Construction Entrance
1. Remove vegetation and grade for positive drainage.
  2. Construct entrance a minimum 12 feet wide (or full width of road) by 50 feet long.
  3. If slope towards a paved street exceeds 2 percent, construct a ridge 6 to 8 inches high with 3:1 (h:v) side slopes across the foundation 15 feet from the entrance to divert runoff away from street.
  4. Place geotextile woven fabric in graded foundation.
  5. Place stone over geotextile woven fabric per the drawings.
  6. Grade to drain runoff to a sediment trap or basin.

7. Install drain pipe, as needed, to maintain street drainage in right-of-way.

**3.05 MAINTENANCE**

- A. Inspect preventive measures daily, within 24 hours after the end of any storm that produces 0.5 inches or more rainfall at the project site, and daily during prolonged rainfall.
- B. Repair deficiencies immediately.
- C. Silt Fences:
  1. Promptly replace fabric that deteriorates.
  2. Remove silt deposits that exceed one-third of the height of the fence.
  3. Repair fences that are undercut by runoff or otherwise damaged, whether by runoff or other causes.
- D. Straw Bale Rows:
  1. Promptly replace bales that fall apart or otherwise deteriorate unless need has passed.
  2. Remove silt deposits that exceed one-half of the height of the bales.
  3. Repair bale rows that are undercut by runoff or otherwise damaged, whether by runoff or other causes.
- E. Filter Bags:
  1. Promptly replace bags that have deteriorated or have been damaged.
  2. Remove silt deposits that exceed one-third the height of the bag.
  3. Repair or replace bags that are undercut by runoff or otherwise are damaged, whether by runoff or other causes.
- F. Stabilized Construction Entrance
  1. Promptly replace rock that has deteriorated or been damaged.
  2. Remove excess dirt and sediment accumulations as needed.
- G. Rock Berms
  1. Promptly repair or replace rock berms that have been undercut by runoff or otherwise damaged.
  2. Remove silt deposits that exceed one-third the height of the rock berm.
  3. Repair any loose wire sheathing.
- H. Clean out temporary sediment control structures weekly and relocate soil on site.
- I. Place sediment in appropriate locations on site; do not remove from site.

**3.06 CLEAN UP**

- A. Remove temporary measures after permanent measures have been installed, unless permitted to remain by Engineer.
- B. Clean out temporary sediment control structures that are to remain as permanent measures.
- C. Where removal of temporary measures would leave exposed soil, shape surface to an acceptable grade and finish to match adjacent ground surfaces.

**END OF SECTION**

**SPILL RESPONSE****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Guideline for Contractors for handling petroleum hydrocarbon spills and leaks on the project site.

**1.02 RELATED SECTIONS -- NOT USED****1.03 REFERENCES**

- A. Texas Commission on Environmental Quality Regulations, TAC Ch. 327.
- B. Edwards Aquifer Authority, Ch. 713, Subchapter E.

**1.04 SUBMITTALS**

- A. Summary report of procedures and operational sequence for review and approval by Pape-Dawson Engineers (Engineer) as Owner's Representative.
- B. Manifest and disposal records.
- C. Testing and laboratory sampling results.

**PART 2 PRODUCTS****2.01 MATERIALS**

- A. Contractor to pay for any and all testing, excavation, disposal, and consultants/contractors required for the execution of the work.

**PART 3 EXECUTION****3.01 CONDITIONS**

- A. Texas Commission on Environmental Quality (TCEQ) and Edwards Aquifer Authority (EAA) regulations require reporting of fuel spills or leaks of 25 gallons or more to the land surface, or any quantity sufficient to create a sheen on surface water. For oils, the reporting requirements are a spill of 210-gallons to the land surface or a quantity sufficient to create a sheen on surface water.
- B. Spills of quantities to the land in excess of 1-gallon, or spills that affect more than 10-square feet of ground surface, although not large enough to require reporting to TCEQ, must be reported by the Contractor to the Engineer and appropriate emergency response actions should be implemented.
- C. Cleanup of any spills related to the project shall be completed to non-detect conditions, unless the constituent is naturally occurring in which case TCEQ State specific background concentrations may be used as cleanup goals, as determined by the Engineer.

**3.02 SPILL RESPONSE ACTIONS**

- A. Appropriate response actions may vary, depending on the volume spilled or leaked and the location of the spill.
  - 1. In all cases, attempts to minimize the volume lost and the size of the area affected should be made.
  - 2. Spills/leaks on asphalt, concrete or other impervious surface, may be cleaned using absorbent materials, i.e., oil absorbent pads or socks, or granular material.
  - 3. Spills/leaks to soil or bedrock will likely require excavation.

- B. Contractor to take immediate action to contain the spill. Implement emergency response measures to stop and contain leak/spill. Appropriate emergency response measures include, but are not limited to, placement of absorbent materials on top of and downstream of leak/spill or construction of berm downstream of leak/spill to prevent further spread of material.
- C. Contact Fire Department or other appropriate Emergency Management Agency as necessary to minimize loss of property or life.
- D. Notify Engineer within 24 hours of the leak/spill.
- E. Notify TCEQ and EAA if the project is within the Edwards Aquifer Recharge Zone or 5 miles upgrade within 24 hours if quantity of material leaked/spilled meets Section 3.01, A. above.
- F. Follow protocol as shown on attached flow chart that describes some possible scenarios.

### **3.03 CLEANUP**

- A. Impacted material should be segregated from material not impacted by the leak/spill. Impacted materials may include loose material, absorbent material, or excavated material.
- B. Excavated and segregated materials should be stored on plastic and covered with plastic, or stored within a drum, roll-off box, or other covered container, pending characterization and disposal at an approved disposal facility.
- C. The volume of material segregated and disposed should be recorded, and copies of disposal manifests should be provided to the Engineer.
- D. After excavation of impacted soil, the Engineer will evaluate the area excavated and determine if sampling of remaining subgrade will be necessary.
- E. The determination will be made on a site-specific basis and will be a function of volume spilled/leaked, size of area affected, soil type, type of product spilled/leaked, location of spill/leak, etc.
- F. If sampling and analysis indicate constituents of concern are detected in the soil or rock subgrade, then additional excavation will be necessary. The Contractor shall continue to excavate at the direction of the Engineer until further sample and analysis results indicate that constituents of concern are no longer detected.
- G. After Engineers approval, the excavated area may be backfilled.
- H. The Contractor may hire their own environmental consultant/contractor to perform sampling, analysis, and cleanup oversight. In this case the Contractor's consultant must coordinate all their activities with the Engineer prior to each step, phase, or course of action.
- I. The Engineer may collect additional confirmation samples if warranted, and will provide approval that corrective actions are complete before the excavation is backfilled.
- J. Contractor must notify Engineer 48-hours in advance of sample collection and disposal. Contractor must provide Engineer or designated construction observer with opportunity to observe Contractor's implementation of approved cleanup method and to collect and split samples, if so desired by Engineer.

### **END OF SECTION**

**TEMPORARY PROJECT SIGNAGE****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Project identification sign.
- B. Project informational signs.

**1.02 RELATED REQUIREMENTS -- NOT USED****1.03 REFERENCE STANDARDS -- NOT USED****1.04 QUALITY ASSURANCE**

- A. Design sign and structure to withstand 30 miles/hr wind velocity.
- B. Sign Painter: Experienced as a professional sign painter.
- C. Finishes, Painting: Adequate to withstand weathering, fading, and chipping for duration of construction.

**1.05 SUBMITTALS**

- A. Shop Drawing: Show content, layout, lettering, and color to the Owner for approval prior to installation .

**PART 2 PRODUCTS****2.01 SIGN MATERIALS**

- A. Structure and Framing: New, wood, structurally adequate.
- B. Sign Surfaces: Exterior grade plywood with medium density overlay, minimum 3/4 inch thick, standard large sizes to minimize joints.
- C. Rough Hardware: Galvanized, aluminum, or brass.
- D. Paint and Primers: Exterior quality, two coats; sign background of color as selected.
- E. Lettering: Exterior quality paint, colors as selected.

**2.02 PROJECT IDENTIFICATION SIGN**

- A. One painted sign of construction, design, and content indicated on drawings, location designated.
- B. Content:
  - 1. Project title and name of Owner as indicated on Contract Documents.
  - 2. Names and titles of authorities.
  - 3. Name of Prime Contractor and major Subcontractors.
- C. Graphic Design, Colors, Style of Lettering: Designated by Owner.

**2.03 PROJECT INFORMATIONAL SIGNS**

- A. Painted informational signs of same colors and lettering as Project Identification sign, or standard products; size lettering to provide legibility at 100 foot distance.
- B. Provide at each field office, storage shed, and directional signs to direct traffic into and within site. Relocate as Work progress requires.

**PART 3 EXECUTION****3.01 INSTALLATION**

- A. Install project identification sign within 20 days after date fixed by Notice to Proceed.



- B. Erect at designated location.
- C. Erect supports and framing on secure foundation, rigidly braced and framed to resist wind loadings.
- D. Install sign surface plumb and level, with butt joints. Anchor securely.
- E. Paint exposed surfaces of sign, supports, and framing.
- F. No other signs or advertising will be permitted on the site without specific authorization.
- G. Maintain sign in good condition for the duration of the job.

**3.02 MAINTENANCE**

- A. Maintain signs and supports clean, repair deterioration and damage.

**3.03 REMOVAL**

- A. Remove signs, framing, supports, and foundations at completion of Project and restore the area.

**END OF SECTION**

**FIELD ENGINEERING****PART 1 - GENERAL****1.01 DESCRIPTION**

- A. The Contractor shall, at his expense, perform the following:
  - 1. Engage a licensed Surveyor, approved by the Owner, and acceptable to the Engineer, to locate all surveyor marks, including bench marks in order that the exact lines of the property, building and grades may be determined.
  - 2. Lay out entire Project prior to start of construction.
  - 3. Locate and protect control points prior to starting site work, and preserve all permanent reference points during construction. Replace project control points which may be lost or destroyed.
  - 4. Establish a minimum of two permanent bench marks on the site, referenced to data established by survey control points. Record locations, with horizontal and vertical data, on Project Record Documents.
  - 5. Establish all construction lines and levels by instrumentation and similar appropriate means.
- B. Any discrepancies arising in locating the work in respect to property and building line shall be reported immediately to the Owner and the Engineer.

**END OF SECTION**

**WARRANTIES AND BONDS****PART 1 - GENERAL****1.01 SUBMITTAL REQUIREMENTS**

- A. Assemble warranties, bonds and services and maintenance contracts, executed by each of the respective manufacturers, suppliers, and subcontractors.
- B. Review submittals to verify compliance with Contract Documents. Submit to Engineer for review and transmittal to Owner.

**1.02 TIME OF SUBMITTALS**

- A. For equipment or component parts of equipment put into service during progress of construction submit within 10 days after acceptance.
- B. Otherwise make submittals within ten days after Date of Substantial Completion, prior to final request for payment.
- C. For items of work, where acceptance is delayed materially beyond the Date of Substantial Completion, provide updated submittal within ten days after acceptance, listing the date of acceptance as the start of the warranty period.

**END OF SECTION**

**SUBSURFACE INVESTIGATION****PART 1 - GENERAL****1.01 SUMMARY**

- A. A Geotechnical Report (available for review in the office of the Engineer) was obtained for use in preparing the design.
- B. The Geotechnical Report is not a conclusive indication of the soil conditions other than where the borings were taken.
- C. The accuracy of the Geotechnical Report is not guaranteed in any respect by the Owner, and the Owner accepts no responsibility for interpretation of conclusions drawn therefrom.
- D. The information contained in the Geotechnical Report is made available in order that the Contractor may have ready access to the same information available to the Owner as of this date.
- E. Contractor is invited and encouraged to make his own interpretation and evaluation of the information and by starting work shall be assumed to have fully accepted responsibility for the subsurface conditions that may hereafter be encountered in performing the excavation work.
- F. Contractor is to examine the project site and the record of investigation and make, to whatever extent they deem appropriate, his own investigation of existing subsurface conditions to determine the nature, kind and character of materials to be encountered.
- G. Extra payment will not be authorized for work which should have been anticipated or could have been anticipated upon careful examination of the site, or upon soil investigation, or upon consideration of factors generally recognized as being inherent in excavation work of the nature indicated by the Contract Documents.
- H. The Contractor shall advise Engineer of discovery of any unknown or undetermined items.
- I. The Contractor shall make their own investigation into the location and size of existing site utilities whether represented on the drawings or not.

**END OF SECTION**

**MAINTENANCE OF CONCRETE****PART 1 GENERAL****1.01 Section Includes**

- A. Cleaning of existing concrete surfaces.
- B. Repair of exposed structural, shrinkage, and settlement cracks.
- C. Resurfacing of concrete surfaces having spalled areas and other damage.
- D. Repair of deteriorated concrete.
- E. Repair of internal concrete reinforcement.
- F. Restoration and patching of concrete surfaces.

**1.02 Related Requirements**

- A. Section 03 1000 - Concrete Forming and Accessories.
- B. Section 03 2000 - Concrete Reinforcing.
- C. Section 03 3000 - Cast-in-Place Concrete.

**1.03 Price and Payment Procedures**

- A. Repair Surface: By the square foot. Includes surface preparation, repair, finishing.
- B. Preparation for Resurfacing: By the square foot. Includes surface preparation, cleaning.

**1.04 Reference Standards**

- A. ASTM A82/A82M - Standard Specification for Steel Wire, Plain, for Concrete Reinforcement; 2007.
- B. ASTM A615/A615M - Standard Specification for Deformed and Plain Carbon Steel Bars for Concrete Reinforcement; 2015.
- C. ASTM A767/A767M - Standard Specification for Zinc-Coated (Galvanized) Steel Bars for Concrete Reinforcement; 2009.
- D. ASTM A775/A775M - Standard Specification for Epoxy-Coated Steel Reinforcing Bars; 2007b (Reapproved 2014).
- E. ASTM A996/A996M - Standard Specification for Rail-Steel and Axle-Steel Deformed Bars for Concrete Reinforcement; 2014.
- F. ASTM A1064/A1064M - Standard Specification for Carbon-Steel Wire and Welded Wire Reinforcement, Plain and Deformed, for Concrete; 2015.
- G. ASTM C33/C33M - Standard Specification for Concrete Aggregates; 2016.
- H. ASTM C150/C150M - Standard Specification for Portland Cement; 2016.
- I. ASTM C348 - Standard Test Method for Flexural Strength of Hydraulic-Cement Mortars; 2014.
- J. ASTM C404 - Standard Specification for Aggregates for Masonry Grout; 2011.
- K. ASTM C882 - Standard Test Method for Bond Strength of Epoxy-Resin Systems Used with Concrete by Slant Shear; 2012.
- L. ASTM C928/C928M - Standard Specification for Packaged, Dry, Rapid-Hardening Cementitious Material for Concrete Repairs; 2013.
- M. ASTM C1059/C1059M - Standard Specification for Latex Agents for Bonding Fresh to Hardened Concrete; 2013.

- N. ASTM D3039/D3039M - Standard Test Method for Tensile Properties of Polymer Matrix Composite Materials; 2017.
- O. ASTM D638 - Standard Test Method for Tensile Properties of Plastics; 2010.
- P. ASTM D695 - Standard Test Method for Compressive Properties of Rigid Plastics; 2010.
- Q. AWS D1.4/D1.4M - Structural Welding Code - Reinforcing Steel; 2011.
- R. ICC-ES AC178 - Acceptance Criteria for Inspection and Verification of Concrete and Reinforced and Unreinforced Masonry Strengthening Using Fiber-Reinforced Polymer (FRP) or Steel-Reinforced Polymer (SRP) Composite Systems; 2017, with Editorial Revision 2020.

#### **1.05 Administrative Requirements**

- A. Scheduling: Perform work during a time period allowed by authority having jurisdiction of road right-of-way.

#### **1.06 Submittals**

- A. Product Data: Indicate product standards, physical and chemical characteristics, technical specifications, limitations, maintenance instructions, and general recommendations regarding each material.
- B. Field quality control submittals.
- C. Field quality control submittals for CFRP.
- D. Manufacturer's Certificate: Certify that specified products meet or exceed specified requirements.
- E. Project Record Documents: Accurately record actual locations of structural reinforcement repairs and type of repair.

#### **1.07 Quality Assurance**

- A. Designer Qualifications: Design reinforcement splices under direct supervision of a Professional Structural Engineer experienced in design of this type of work and licensed in the State in which the Project is located.
- B. Manufacturer Qualifications: Company specializing in manufacturing products specified in this section.
- C. Cleaner Qualifications: Company specializing in performing work of the type specified.
- D. Installer Qualifications: Company specializing in performing work of the type specified.
- E. Perform welding work in accordance with AWS D1.4.

#### **1.08 Delivery, Storage, and Handling**

- A. Comply with manufacturers' instructions for storage, shelf life limitations, and handling of products.

### **PART 2 PRODUCTS**

#### **2.01 Cleaning Materials**

- A. Detergent: Non-ionic detergent.
- B. Blasting Medium: Sand.

#### **2.02 Cementitious Patching and Repair Materials**

- A. Cementitious Resurfacing Mortar: One- or two-component, factory-mixed, polymer-modified cementitious mortar designed for continuous thin-coat application.

1. In-place material resistant to freezing conditions.
  2. Mixed with water or latex type bonding agent in proportions as recommended by manufacturer.
  3. Integral corrosion inhibitor.
  4. Recommended Thickness: Feather edge to 1/8 inch.
  5. Color: Gray.
- B. Cementitious Repair Mortar, Trowel Grade: One- or two-component, factory-mixed, polymer-modified cementitious mortar.
1. In-place material resistant to freezing conditions.
  2. Mixed with water or latex type bonding agent in proportions as recommended by manufacturer.
  3. Dry Material: Complies with ASTM C928/C928M.
  4. Integral corrosion inhibitor.
- C. Cementitious Repair Mortar, Form and Pour/Pump Grade: Flowable, one- or two-component, factory-mixed, polymer-modified cementitious mortar; in-place material resistant to freezing conditions.
1. Mixed with water in proportions as recommended by manufacturer.
- D. Cementitious Pavement Repair Mortar: Fast hardening, flowable; composed of cement, sand, and additives; capable of setting in cold weather conditions without the aid of chloride- or gypsum-based accelerators; in-place material resistant to freezing conditions.
1. Dry Material: Complies with ASTM C928/C928M.
  2. Integral corrosion inhibitor.
  3. Time To Open To Traffic: 1 hour, maximum.
  4. Time to Top-Coating: 4 hours, maximum.
- E. Cementitious Hydraulic Waterstop: Very fast setting, low slump, hand formable, and capable of stopping active water leaks; in-place material resistant to freezing conditions.
- F. Mix cementitious mortar and grout in accordance with manufacturer's instructions for purpose intended.
- G. Include bonding agent as additive to mix.

### 2.03 Epoxy Patching and Repair Materials

- A. Epoxy Repair Mortar: Epoxy resin mixed with aggregate and other materials in accordance with manufacturer's instructions for purpose intended; comply with pot life and workability limits.
- B. Mix epoxy mortars in accordance with manufacturer's instructions for purpose intended.
- C. Mix components in clean equipment or containers. Conform to pot life and workability limits.
- D. Epoxy Bonding Adhesive: Non-sag, two-component, 100 percent solids; recommended by manufacturer for purpose and conditions under which used.
1. Bond Strength (ASTM C882): 1,500 psi, minimum.
  2. Tensile Strength (ASTM D638): 6,600 psi, minimum.
  3. Percent Elongation (ASTM D638): 3.3 percent at 7 days at 70 degrees F, maximum.
  4. Compressive Strength (ASTM D695): 10,000 psi, minimum.

### 2.04 Accessories

- A. Anchoring Adhesive: Self-leveling or non-sag as applicable.

- B. Portland Cement: ASTM C 150, Type I, II, or III; gray or white.
- C. Sand: ASTM C33/C33M or ASTM C404; uniformly graded, clean.
- D. Water: Clean and potable.
- E. Reinforcing Steel: ASTM A615/A615M Grade 40 (40,000 psi) billet-steel deformed bars, unfinished.
- F. Reinforcing Steel: Deformed bars, ASTM A996/A996M Grade 40 (280), Type A.
  - 1. Galvanized in accordance with ASTM A 767/A 767M, Class I or II.
- G. Stirrup Steel: ASTM A1064/A1064M.
- H. Splicing Sleeves: Per shop drawings.

### **PART 3 EXECUTION**

#### **3.01 Examination**

- A. Verify that surfaces are ready to receive work.
- B. Beginning of installation means acceptance of substrate.

#### **3.02 Cleaning Existing Concrete**

- A. Provide enclosures, barricades, and other temporary construction as required to protect adjacent work from damage.
- B. Clean concrete surfaces of dirt or other contamination using the gentlest method that is effective.
  - 1. Try the gentlest method first, then, if not clean enough, use a less gentle method taking care to watch for impending damage.
  - 2. Clean out cracks and voids using same methods.
- C. The following are acceptable cleaning methods, in order from gentlest to less gentle:
  - 1. Water washing using low-pressure, maximum of 100 psi, and, if necessary, brushes with natural or synthetic bristles.
  - 2. Increasing the water washing pressure to maximum of 400 psi.
  - 3. Adding detergent to washing water; with final water rinse to remove residual detergent.
  - 4. Steam-generated low-pressure hot-water washing.
  - 5. Abrasive blasting: Use sand.

#### **3.03 Concrete Structural Member Repair**

- A. See drawings for specific areas to be repaired.
- B. Remove broken and soft concrete at least 1/4 inch deep.
- C. Mechanically cut away damaged portions of reinforcement.
- D. Remove corrosion from steel and clean mechanically.
- E. Blast clean remaining exposed reinforcement surfaces.
- F. Repair by welding new bar reinforcement to existing reinforcement using sleeve splices.
  - 1. Perform welding work in accordance with AWS D1.4/D1.4M.
  - 2. Make welded sleeve splices to achieve strength to exceed strength of new reinforcement.
- G. Cover exposed steel reinforcement with epoxy mortar.
- H. Work epoxy mortar into broken surface and build up patch to match original.



- I. Feather edges of repairs flush to sound surface and trowel surface to match surrounding area.

### **3.04 Crack Repair Using Epoxy Adhesive Injection**

- A. Repair exposed cracks.
- B. Provide temporary entry ports spaced to accomplish movement of fluids between ports; no deeper than the depth of the crack to be filled or port size diameter no greater than the thickness of the crack. Provide temporary seal at concrete surface to prevent leakage of adhesive.
- C. Inject adhesive into ports under pressure using equipment appropriate for particular application.
- D. Begin injection at lower entry port and continue until adhesive appears in adjacent entry port. Continue from port to port until entire crack is filled.
- E. Remove temporary seal and excess adhesive.
- F. Clean surfaces adjacent to repair and blend finish.

### **3.05 Concrete Surface Repair Using Cementitious Materials**

- A. Clean concrete surfaces, cracks, and joints of dirt, laitance, corrosion, and other contamination using method(s) specified above and allow to dry.
- B. Apply coating of bonding agent to entire concrete surface to be repaired.
- C. Fill voids with cementitious mortar flush with surface.
- D. Apply repair mortar by steel trowel to a minimum thickness of 1/4 inch over entire surface, terminating at a vertical change in plane on all sides.
- E. Trowel finish to match adjacent concrete surfaces.
- F. Damp cure for four days.

### **3.06 Field Quality Control**

- A. See Section 01 4000 - Quality Requirements for additional requirements.
- B. An independent testing agency will perform field inspection and testing.
  - 1. Test concrete for calcium chloride content during the execution of the Work.
  - 2. Field Quality Control for CFRP:
    - a. Inspect installation and test for compliance with ICC-ES AC178.
    - b. Inspect for voids, bubbles, and delaminations by performing a visual and acoustic tap test of layered surface after 24 hours of initial resin saturant cure.
    - c. Test for material properties of CFRP in accordance with ASTM D3039/D3039M.
    - d. Nonconforming Work: Repair defective work after minimum cure time for CFRP laminates.

**END OF SECTION**

**CONCRETE FORMING AND ACCESSORIES****PART 1 GENERAL****1.01 Section Includes**

- A. Formwork for cast-in-place concrete, with shoring, bracing and anchorage.
- B. Openings for other work.
- C. Form accessories.
- D. Form stripping.

**1.02 Related Requirements**

- A. Section 03 2000 - Concrete Reinforcing.
- B. Section 03 3000 - Cast-in-Place Concrete.

**1.03 Price and Payment Procedures**

- A. See Section 01 2200 - Unit Prices, for additional unit price requirements.
- B. Measurement and payment of forming work will be by the unit price method.
- C. Formwork (Vertical Structures): Measure by the square foot. Includes form materials, placement, placing accessories, stripping.
- D. Formwork (Horizontal Structures): Measure by the square foot. Includes form materials, placement, placing accessories, stripping.

**1.04 Reference Standards**

- A. ACI CODE-318 - Building Code Requirements for Structural Concrete and Commentary; 2019 (Reapproved 2022).
- B. ACI PRC-347 - Guide to Formwork for Concrete; 2014 (Reapproved 2021).
- C. ACI SPEC-117 - Specification for Tolerances for Concrete Construction and Materials; 2010 (Reapproved 2015).
- D. ACI SPEC-301 - Specifications for Concrete Construction; 2020.
- E. ACI 117 - Standard Specifications for Tolerances for Concrete Construction and Materials; 2010.
- F. ACI 301 - Specifications for Structural Concrete; 2010 (Errata 2012).
- G. ACI 318 - Building Code Requirements for Structural Concrete and Commentary; 2011.
- H. ACI 347R - Guide to Formwork for Concrete; 2014.
- I. ASME A17.1 - Safety Code for Elevators and Escalators; 2013.
- J. PS 1 - Structural Plywood; 2009.

**1.05 Submittals**

- A. Product Data: Provide data on void form materials and installation requirements.
- B. Shop Drawings: Indicate pertinent dimensions, materials, bracing, and arrangement of joints and ties.
- C. Permanent Insulated Foam Panel Formwork Shop Drawings: Include calculations or selections from manufacturer's prescriptive design tables that indicate compliance with applicable building code and manufacturer's requirements.
  - 1. Include test reports for performance criteria specified.

- 2. Include the design engineer's stamp or seal on each sheet of shop drawings.
- D. Design Data: As required by authorities having jurisdiction.
- E. Construction Joints: Submit diagram of proposed construction joints not shown on the Drawings prior to or concurrent with reinforcing bar shop drawings.

#### **1.06 Quality Assurance**

- A. Perform work of this section in accordance with Texas Department of Transportation standards.
- B. Maintain one copy of each installation standard on site throughout the duration of concrete work.

#### **1.07 Delivery, Storage, and Handling**

- A. Deliver prefabricated forms and installation instructions in manufacturer's packaging.
- B. Store prefabricated forms off ground in ventilated and protected manner to prevent deterioration from moisture.
- C. Protect plastic foam products from damage and exposure to sunlight.

### **PART 2 PRODUCTS**

#### **2.01 Formwork - General**

- A. Provide concrete forms, accessories, shoring, and bracing as required to accomplish cast-in-place concrete work.
- B. Design and construct concrete that complies with design with respect to shape, lines, and dimensions.
- C. Chamfer outside corners of beams, joists, columns, and walls.
- D. Comply with applicable state and local codes with respect to design, fabrication, erection, and removal of formwork.
- E. Comply with relevant portions of ACI CODE-318, ACI PRC-347, and ACI SPEC-301.
- F. Comply with Texas Department of Transportation standards.
- G. Use the following form types:
  - 1. Walls Not Exposed To View: Site fabricated plywood of sufficient thickness capable of sustaining the loads.
  - 2. Walls Exposed To View: Commercial grade, moisture resistant, smooth-faced plywood of sufficient thickness capable of sustaining the loads.
  - 3. Elevated Floor/Roof Slabs: Permanent prefabricated foam panel formwork; formwork to remain.
- H. Form Ties:
  - 1. Form ties for exposed concrete surfaces shall be manufactured to allow a positive break back of no less than one inch (1") inside the concrete surface.
  - 2. Ties shall be equipped with a plastic cone of not less than five-eighths inch (5/8") diameter and one inch (1") long which will completely cover the hole and prevent the leakage of any mortar.
  - 3. Form ties for unexposed surfaces shall be bolt rods or patented devices having a minimum tensile strength of three thousand (3,000) pounds when fully assembled.

4. Ties shall be adjustable in length and free of lugs, cones, washers or other features which would leave a hole larger than seven-eighths inch (7/8") in diameter, or depressions back of the exposed surface of the concrete.
5. Ties shall be of such construction that, when the forms are removed, there will be no metal remaining within one inch (1") of the finished surface of the concrete.

## **2.02 Wood Form Materials**

- A. Softwood Plywood: PS 1, B-B High Density Concrete Form Overlay, Class I.
- B. Plywood: Douglas Fir, Spruce, or Yellow Pine species; solid one side grade; sound undamaged sheets with clean, true edges.
- C. Lumber: Yellow Pine species; #2 grade; with grade stamp clearly visible.

## **2.03 Removable Prefabricated Forms**

- A. Preformed Steel Forms: Minimum 16 gauge, 0.0598 inch thick, matched, tight fitting, stiffened to support weight of concrete without deflection detrimental to tolerances and appearance of finished surfaces.
- B. Preformed Plastic Forms: Thermoplastic polystyrene, thermoplastic, thermosetting, filled polyurethane elastomer; or polyurethane elastomer form liner, tight fitting, stiffened to support weight of concrete without deflection detrimental to tolerances and appearance of finished surfaces.
- C. Glass Fiber Fabric Reinforced Plastic Forms: Matched, tight fitting, stiffened to support weight of concrete without deflection detrimental to tolerances and appearance of finished concrete surfaces.
- D. Pan Type: Steel or glass fiber, of size and profile indicated.
- E. Tubular Column Type: Round, Spirally wound laminated fiber, wood, or glass fiber material, surface treated with release agent, non-reusable, of sizes indicated.
- F. Void Forms: Moisture resistant treated paper faces, biodegradable, structurally sufficient to support weight of wet concrete mix until initial set.

## **2.04 Permanent Prefabricated Foam Panel Formwork**

- A. Floor/Roof Deck Forms: Pre-engineered expanded polystyrene foam plastic deck and beam/joist forms with factory installed metal channel furring strips flush with face of panel and field installed form stiffener slots.
  1. Structural Performance: In accordance with applicable code.
  2. Form Cross Section: As indicated on drawings; flat-bottomed solid foam blocks with voids only for stiffeners and beam/joist cross-section; interlocking long edges.

## **2.05 Formwork Accessories**

- A. Form Ties: Removable or snap-off type, galvanized metal or plastic, fixed length, cone type, 1 inch back break dimension, free of defects that could leave holes larger than 1 inch in concrete surface.
- B. Form Release Agent: Colorless mineral oil that will not stain concrete, absorb moisture, impair natural bonding of concrete finish coatings, or affect color characteristics of concrete finish coatings.
- C. Filler Strips for Chamfered Corners: Rigid plastic or wood strip type.

- D. Dovetail Anchor Slot: Galvanized steel, at least 22 gauge, 0.0299 inch thick, foam filled, release tape sealed slots, anchors for securing to concrete formwork.
- E. Flashing Reglets: Galvanized steel, at least 22 gauge, 0.0299 inch thick, longest possible lengths, with alignment splines for joints, foam filled, release tape sealed slots, anchors for securing to concrete formwork.
- F. Nails, Spikes, Lag Bolts, Through Bolts, Anchorages: Sized as required, of sufficient strength and character to maintain formwork in place while placing concrete.
- G. Waterstops: Rubber or polyvinyl chloride, minimum 1,750 psi tensile strength, minimum 50 degrees F to plus 175 degrees F working temperature range.

### **PART 3 EXECUTION**

#### **3.01 Examination**

- A. Verify lines, levels and centers before proceeding with formwork. Ensure that dimensions agree with drawings.

#### **3.02 Earth Forms**

- A. Hand trim sides and bottom of earth forms. Remove loose soil prior to placing concrete.

#### **3.03 Erection - Formwork**

- A. Erect formwork, shoring and bracing to achieve design requirements, in accordance with requirements of ACI SPEC-301.
- B. Provide bracing to ensure stability of formwork. Shore or strengthen formwork subject to overstressing by construction loads.
- C. Install permanent insulated foam panel formwork per manufacturer's recommendations.
- D. Arrange and assemble formwork to permit dismantling and stripping. Do not damage concrete during stripping. Permit removal of remaining principal shores.
- E. Align joints and make watertight. Keep form joints to a minimum.
- F. Obtain approval before framing openings in structural members that are not indicated on drawings.
- G. Install void forms in accordance with manufacturer's recommendations. Protect forms from moisture or crushing.
- H. Coordinate this section with other sections of work that require attachment of components to formwork.
- I. If formwork is placed after reinforcement, resulting in insufficient concrete cover over reinforcement, request instructions from Engineer before proceeding.

#### **3.04 Application - Form Release Agent**

- A. Apply form release agent on formwork in accordance with manufacturer's recommendations.
- B. Apply prior to placement of reinforcing steel, anchoring devices, and embedded items.
- C. Do not apply form release agent where concrete surfaces will receive special finishes or applied coverings that are affected by agent. Soak inside surfaces of untreated forms with clean water. Keep surfaces coated prior to placement of concrete.

**3.05 Inserts, Embedded Parts, and Openings**

- A. Provide formed openings where required for items to be embedded in passing through concrete work.
- B. Locate and set in place items that will be cast directly into concrete.
- C. Coordinate with work of other sections in forming and placing openings, slots, reglets, recesses, sleeves, bolts, anchors, other inserts, and components of other work.
- D. Install accessories in accordance with manufacturer's instructions, so they are straight, level, and plumb. Ensure items are not disturbed during concrete placement.
- E. Install waterstops in accordance with manufacturer's instructions, so they are continuous without displacing reinforcement. Heat seal joints so they are watertight.
- F. Provide temporary ports or openings in formwork where required to facilitate cleaning and inspection. Locate openings at bottom of forms to allow flushing water to drain.
- G. Close temporary openings with tight fitting panels, flush with inside face of forms, and neatly fitted so joints will not be apparent in exposed concrete surfaces.

**3.06 Form Cleaning**

- A. Clean forms as erection proceeds, to remove foreign matter within forms.
- B. Clean formed cavities of debris prior to placing concrete.
  - 1. Flush with water or use compressed air to remove remaining foreign matter. Ensure that water and debris drain to exterior through clean-out ports.
  - 2. During cold weather, remove ice and snow from within forms. Do not use de-icing salts. Do not use water to clean out forms, unless formwork and concrete construction proceed within heated enclosure. Use compressed air or other means to remove foreign matter.

**3.07 Formwork Tolerances**

- A. Construct formwork to maintain tolerances required by ACI SPEC-117, unless otherwise indicated.
- B. Construct permanent insulated foam panel formwork to maintain tolerances required by ACI SPEC-301.
- C. Construct and align formwork for elevator hoistway in accordance with ASME A17.1.
- D. Camber slabs and beams in accordance with ACI SPEC-301.

**3.08 Field Quality Control**

- A. Inspect erected formwork, shoring, and bracing to ensure that work is in accordance with formwork design, and to verify that supports, fastenings, wedges, ties, and items are secure.

**3.09 Form Removal**

- A. Do not remove forms or bracing until concrete has gained sufficient strength to carry its own weight and imposed loads.
- B. Loosen forms carefully. Do not wedge pry bars, hammers, or tools against finish concrete surfaces scheduled for exposure to view.
- C. Store removed forms to prevent damage to form materials or to fresh concrete. Discard damaged forms.

**END OF SECTION**



**CONCRETE REINFORCING****PART 1 GENERAL****1.01 Section Includes**

- A. Reinforcing steel for cast-in-place concrete.
- B. Supports and accessories for steel reinforcement.

**1.02 Related Requirements**

- A. Section 03 1000 - Concrete Forming and Accessories.
- B. Section 03 3000 - Cast-in-Place Concrete.

**1.03 Price and Payment Procedures**

- A. Bar Reinforcement: By the ton. Includes reinforcement, placement, and accessories.
- B. Welded Wire Reinforcement: By the square foot. Includes welded wire reinforcement, placement, and accessories.

**1.04 Reference Standards**

- A. ACI MNL-66 - ACI Detailing Manual; 2020.
- B. ACI 301 - Specifications for Structural Concrete; 2010 (Errata 2012).
- C. ACI 318 - Building Code Requirements for Structural Concrete and Commentary; 2011.
- D. ACI SP-66 - ACI Detailing Manual; 2004.
- E. ACI SPEC-301 - Specifications for Concrete Construction; 2020.
- F. ASTM A82/A82M - Standard Specification for Steel Wire, Plain, for Concrete Reinforcement; 2007.
- G. ASTM A184/A184M - Standard Specification for Welded Deformed Steel Bar Mats for Concrete Reinforcement; 2006 (Reapproved 2011).
- H. ASTM A185/A185M - Standard Specification for Steel Welded Wire Reinforcement, Plain, for Concrete; 2007.
- I. ASTM A497/A497M - Standard Specification for Steel Welded Wire Reinforcement, Deformed, for Concrete; 2007.
- J. ASTM A615/A615M - Standard Specification for Deformed and Plain Carbon Steel Bars for Concrete Reinforcement; 2015.
- K. ASTM A641/A641M - Standard Specification for Zinc-Coated (Galvanized) Carbon Steel Wire; 2009a (Reapproved 2014).
- L. ASTM A704/A704M - Standard Specification for Welded Steel Plain Bar or Rod Mats for Concrete Reinforcement; 2006 (Reapproved 2011).
- M. ASTM A706/A706M - Standard Specification for Low-Alloy Steel Deformed and Plain Bars for Concrete Reinforcement; 2014.
- N. ASTM A767/A767M - Standard Specification for Zinc-Coated (Galvanized) Steel Bars for Concrete Reinforcement; 2009.
- O. ASTM A775/A775M - Standard Specification for Epoxy-Coated Steel Reinforcing Bars; 2007b (Reapproved 2014).
- P. ASTM A884/A884M - Standard Specification for Epoxy-Coated Steel Wire and Welded Wire Reinforcement; 2014.



- Q. ASTM A996/A996M - Standard Specification for Rail-Steel and Axle-Steel Deformed Bars for Concrete Reinforcement; 2014.
- R. ASTM A1064/A1064M - Standard Specification for Carbon-Steel Wire and Welded Wire Reinforcement, Plain and Deformed, for Concrete; 2015.
- S. ASTM D3963/D3963M - Standard Specification for Fabrication and Jobsite Handling of Epoxy-Coated Reinforcing Steel Bars; 2001 (Reapproved 2007).
- T. AWS D1.4/D1.4M - Structural Welding Code - Reinforcing Steel; 2011.
- U. CRSI (DA4) - Manual of Standard Practice; 2009.
- V. CRSI (P1) - Placing Reinforcing Bars; 2011.

#### **1.05 Submittals**

- A. Shop Drawings: Comply with requirements of ACI MNL-66. Include bar schedules, shapes of bent bars, spacing of bars, and location of splices.
  - 1. Prepare shop drawings under seal of a Professional Structural Engineer experienced in design of work of this type and licensed in the State in which the Project is located.
- B. Manufacturer's Certificate: Certify that reinforcing steel and accessories supplied for this project meet or exceed specified requirements.
- C. Reports: Submit certified copies of mill test report of reinforcement materials analysis.

#### **1.06 Quality Assurance**

- A. Perform work of this section in accordance with ACI SPEC-301.
  - 1. Maintain one copy of each document on project site.
- B. Provide Engineer with access to fabrication plant to facilitate inspection of reinforcement. Provide notification of commencement and duration of shop fabrication in sufficient time to allow inspection.

### **PART 2 PRODUCTS**

#### **2.01 Reinforcement**

- A. Reinforcing Steel: ASTM A 615/A 615M Grade 40 (280), Unfinished.
- B. Reinforcing Steel: ASTM A 706/A 706M, deformed low-alloy steel bars, Unfinished.
- C. Reinforcing Steel: Deformed bars, ASTM A996/A996M Grade 40 (280), Type A.
  - 1. Galvanized in accordance with ASTM A767/A767M, Class I.
- D. Reinforcing Steel Mat: ASTM A704/A704M, using ASTM A615/A615M, Grade 40 (40,000 psi) steel bars or rods, unfinished.
- E. Stirrup Steel: ASTM A1064/A1064M steel wire, unfinished.
- F. Steel Welded Wire Reinforcement (WWR): Galvanized, deformed type; ASTM A1064/A1064M.
  - 1. Form: Flat Sheets.
  - 2. WWR Style: 4 x 8-W6 x W10.
  - 3. Wire Gage: W4xW4 unless otherwise indicated on drawings.
- G. Reinforcement Accessories:
  - 1. Tie Wire: Annealed, minimum 16 gauge, 0.0508 inch.
  - 2. Chairs, Bolsters, Bar Supports, Spacers: Sized and shaped for adequate support of reinforcement during concrete placement.

3. Provide stainless steel, galvanized, plastic, or plastic coated steel components for placement within 1-1/2 inches of weathering surfaces.

## **2.02 Fabrication**

- A. Fabricate concrete reinforcing in accordance with CRSI (DA4) - Manual of Standard Practice.
- B. Welding of reinforcement is permitted only with the specific approval of Engineer. Perform welding in accordance with AWS D1.4/D1.4M.
  1. Galvanized and Epoxy Coated Reinforcement: Clean surfaces, weld and re-protect welded joint in accordance with CRSI (DA4).
- C. Fabricate and handle epoxy-coated reinforcing in accordance with ASTM D3963/D3963M.
- D. Locate reinforcing splices not indicated on drawings at point of minimum stress.
  1. Review locations of splices with Engineer.

## **PART 3 EXECUTION**

### **3.01 Placement**

- A. Place, support and secure reinforcement against displacement. Do not deviate from required position.
- B. Do not displace or damage vapor barrier.
- C. Accommodate placement of formed openings.
- D. Maintain concrete cover around reinforcing as follows:
  1. Beams: 2 inch or per Engineer's drawings.
  2. Supported Slabs and Joists: 2 inch or per Engineer's drawings.
  3. Column Ties: 2 inch or per Engineer's drawings.
  4. Walls (exposed to weather or backfill): 2 inch or per Engineer's drawings.
  5. Footings and Concrete Formed Against Earth: 2 inch or per Engineer's drawings.
  6. Slabs on Fill: 2 inch or per Engineer's drawings.
- E. Reinforcement shall be accurately placed and securely saddle tied at every other intersection with No. 18 gauge black annealed wire, and shall be rigidly held in place during the placing of the concrete by means of metal chairs or spacers.
- F. Bars in concrete walls shall be held in position, and to proper clearance, by means of concrete or metal spacer made especially for the locations where spacers are required.
- G. Bars in beams and slabs shall be held to exact location during placing of concrete by spacers, chairs, or other necessary supports.
- H. Comply with applicable code for concrete cover over reinforcement.
- I. Bond and ground all reinforcement.

### **3.02 Field Quality Control**

- A. An independent testing agency will inspect installed reinforcement for conformance to contract documents before concrete placement.

### **3.03 Schedules**

- A. Reinforcement For Superstructure Framing Members: Deformed bars, unfinished.
- B. Reinforcement For Foundation Wall Framing Members and Slab-on-Grade: Deformed bars and welded wire reinforcement, galvanized finish.

- C. Reinforcement For Parking Structure Framing Members: Deformed bars, epoxy coated finish.

**END OF SECTION**

**CAST-IN-PLACE CONCRETE****PART 1 GENERAL****1.01 Section Includes**

- A. Concrete formwork.
- B. Concrete shear walls, elevator shaft walls, and foundation walls.
- C. Joint devices associated with concrete work.
- D. Miscellaneous concrete elements.
- E. Concrete curing.

**1.02 Related Requirements**

- A. Section 03 1000 - Concrete Forming and Accessories.
- B. Section 03 2000 - Concrete Reinforcing.
- C. Section 07 9200 - Joint Sealants: Products and installation for sealants and joint fillers for saw cut joints and isolation joints in slabs.
- D. Section 32 1250 - Site Pavement.

**1.03 Price and Payment Procedures**

- A. Cement: By the cubic yard.

**1.04 Reference Standards**

- A. ACI 117 - Standard Specifications for Tolerances for Concrete Construction and Materials; 2010.
- B. ACI 211.1 - Standard Practice for Selecting Proportions for Normal, Heavyweight, and Mass Concrete; 1991 (Reapproved 2009).
- C. ACI 211.2 - Standard Practice for Selecting Proportions for Structural Lightweight Concrete; 1998 (Reapproved 2004).
- D. ACI CODE-318 - Building Code Requirements for Structural Concrete and Commentary; 2019 (Reapproved 2022).
- E. ACI PRC-211.1 - Selecting Proportions for Normal-Density and High Density-Concrete - Guide; 2022.
- F. ACI 301 - Specifications for Structural Concrete; 2010 (Errata 2012).
- G. ACI 302.1R - Guide for Concrete Floor and Slab Construction; 2004 (Errata 2007).
- H. ACI 304R - Guide for Measuring, Mixing, Transporting, and Placing Concrete; 2000.
- I. ACI 305R - Hot Weather Concreting; 2010.
- J. ACI 306R - Cold Weather Concreting; 2010.
- K. ACI 308R - Guide to Curing Concrete; 2001 (Reapproved 2008).
- L. ACI 318 - Building Code Requirements for Structural Concrete and Commentary; 2011.
- M. ACI 347R - Guide to Formwork for Concrete; 2014.
- N. ACI PRC-304 - Heavyweight Concrete: Measuring, Mixing, Transporting and Placing; 2020.
- O. ACI PRC-305 - Guide to Hot Weather Concreting; 2020.
- P. ACI PRC-306 - Guide to Cold Weather Concreting; 2016.

- Q. ACI PRC-308 - Guide to External Curing of Concrete; 2016.
- R. ACI SPEC-301 - Specifications for Concrete Construction; 2020.
- S. ASTM A185/A185M - Standard Specification for Steel Welded Wire Reinforcement, Plain, for Concrete; 2007.
- T. ASTM A497/A497M - Standard Specification for Steel Welded Wire Reinforcement, Deformed, for Concrete; 2007.
- U. ASTM A615/A615M - Standard Specification for Deformed and Plain Carbon Steel Bars for Concrete Reinforcement; 2015.
- V. ASTM A767/A767M - Standard Specification for Zinc-Coated (Galvanized) Steel Bars for Concrete Reinforcement; 2009.
- W. ASTM A775/A775M - Standard Specification for Epoxy-Coated Steel Reinforcing Bars; 2007b (Reapproved 2014).
- X. ASTM A884/A884M - Standard Specification for Epoxy-Coated Steel Wire and Welded Wire Reinforcement; 2014.
- Y. ASTM C33/C33M - Standard Specification for Concrete Aggregates; 2016.
- Z. ASTM C39/C39M - Standard Test Method for Compressive Strength of Cylindrical Concrete Specimens; 2015a.
- AA. ASTM C94/C94M - Standard Specification for Ready-Mixed Concrete; 2015.
- AB. ASTM C109/C109M - Standard Test Method for Compressive Strength of Hydraulic Cement Mortars (Using 2-in. or (50-mm) Cube Specimens); 2013.
- AC. ASTM C143/C143M - Standard Test Method for Slump of Hydraulic-Cement Concrete; 2012.
- AD. ASTM C150/C150M - Standard Specification for Portland Cement; 2016.
- AE. ASTM C171 - Standard Specification for Sheet Materials for Curing Concrete; 2007.
- AF. ASTM C173/C173M - Standard Test Method for Air Content of Freshly Mixed Concrete by the Volumetric Method; 2014.
- AG. ASTM C260/C260M - Standard Specification for Air-Entraining Admixtures for Concrete; 2010a.
- AH. ASTM C309 - Standard Specification for Liquid Membrane-Forming Compounds for Curing Concrete; 2011.
- AI. ASTM C330/C330M - Standard Specification for Lightweight Aggregates for Structural Concrete; 2014.
- AJ. ASTM C494/C494M - Standard Specification for Chemical Admixtures for Concrete; 2013.
- AK. ASTM C618 - Standard Specification for Coal Fly Ash and Raw or Calcined Natural Pozzolan for Use in Concrete; 2015.
- AL. ASTM C685/C685M - Standard Specification for Concrete Made by Volumetric Batching and Continuous Mixing; 2014.
- AM. ASTM C881/C881M - Standard Specification for Epoxy-Resin-Base Bonding Systems for Concrete; 2014.
- AN. ASTM C979/C979M - Standard Specification for Pigments for Integrally Colored Concrete; 2010.

- AO. ASTM C1059/C1059M - Standard Specification for Latex Agents for Bonding Fresh to Hardened Concrete; 2013.
- AP. ASTM C1107/C1107M - Standard Specification for Packaged Dry, Hydraulic-Cement Grout (Nonshrink); 2014.
- AQ. ASTM C1240 - Standard Specification for Silica Fume Used in Cementitious Mixtures; 2014.
- AR. ASTM C1602/C1602M - Standard Specification for Mixing Water Used in the Production of Hydraulic Cement Concrete; 2012.
- AS. ASTM D994/D994M - Standard Specification for Preformed Expansion Joint Filler for Concrete (Bituminous Type); 2011.
- AT. ASTM D1751 - Standard Specification for Preformed Expansion Joint Filler for Concrete Paving and Structural Construction (Nonextruding and Resilient Bituminous Types); 2004 (Reapproved 2013).
- AU. ASTM D2103 - Standard Specification for Polyethylene Film and Sheeting; 2015.
- AV. ASTM D3963/D3963M - Standard Specification for Fabrication and Jobsite Handling of Epoxy-Coated Reinforcing Steel Bars; 2001 (Reapproved 2007).
- AW. ASTM E154/E154M - Standard Test Methods for Water Vapor Retarders Used in Contact with Earth Under Concrete Slabs, on Walls, or as Ground Cover; 2008a (Reapproved 2013).
- AX. ASTM E1155 - Standard Test Method for Determining F(F) Floor Flatness and F(L) Floor Levelness Numbers; 1996 (Reapproved 2008).
- AY. ASTM E1643 - Standard Practice for Selection, Design, Installation and Inspection of Water Vapor Retarders Used in Contact with Earth or Granular Fill Under Concrete Slabs; 2011.
- AZ. ASTM E 1155M - Standard Test Method for Determining F(F) Floor Flatness and F(L) Floor Levelness Numbers [Metric]; 1996 (Reapproved 2008).
- BA. ASTM E1745 - Standard Specification for Plastic Water Vapor Retarders Used in Contact with Soil or Granular Fill under Concrete Slabs; 2011.
- BB. ASTM E1993/E1993M - Standard Specification for Bituminous Water Vapor Retarders Used in Contact with Soil or Granular Fill Under Concrete Slabs; 1998 (Reapproved 2013).
- BC. COE CRD-C 48 - Method of Test for Water Permeability of Concrete; 1992.
- BD. COE CRD-C 513 - COE Specifications for Rubber Waterstops; 1974.
- BE. COE CRD-C 572 - Corps of Engineers Specifications for Polyvinylchloride Waterstop; 1974.
- BF. NSF 61 - Drinking Water System Components - Health Effects; 2014 (Errata 2015).

#### **1.05 Submittals**

- A. Product Data: Submit manufacturers' data on manufactured products showing compliance with specified requirements and installation instructions.
  - 1. For curing compounds, provide data on method of removal in the event of incompatibility with floor covering adhesives.
- B. Construction Joints: Submit drawing of proposed construction joints for slab on grade, etc. where they are not indicated on the drawings.
- C. Laboratory Test Reports and Mix Design: Submit laboratory test reports for concrete materials and mix designs as specified in the Testing Laboratory section of the Specifications.
- D. Test Reports: Submit report for each test or series of tests specified.

- E. Manufacturer's Installation Instructions: For concrete accessories, indicate installation procedures and interface required with adjacent construction.
- F. Sustainable Design Submittal: If any fly ash, ground granulated blast furnace slag, silica fume, rice hull ash, or other waste material is used in mix designs to replace Portland cement, submit the total volume of concrete cast in place, mix design(s) used showing the quantity of portland cement replaced, reports showing successful cylinder testing, and temperature on day of pour if cold weather mix is used.

### 1.06 Quality Assurance

- A. Perform work of this section in accordance with ACI SPEC-301 and ACI CODE-318.
  - 1. Maintain one copy of each document on site.
- B. Follow recommendations of ACI PRC-305 when concreting during hot weather.
- C. Follow recommendations of ACI PRC-306 when concreting during cold weather.

## PART 2 PRODUCTS

### 2.01 Formwork

- A. Form Materials: Contractor's choice of standard products with sufficient strength to withstand hydrostatic head without distortion in excess of permitted tolerances.
  - 1. Form Facing for Exposed Finish Concrete: Contractor's choice of materials that will provide smooth, stain-free final appearance.
  - 2. Earth Cuts: Do not use earth cuts as forms for vertical surfaces. Natural rock formations that maintain a stable vertical edge may be used as side forms.
  - 3. Form Coating: Release agent that will not adversely affect concrete or interfere with application of coatings.
  - 4. Form Ties: Cone snap type that will leave no metal within 1-1/2 inches of concrete surface.

### 2.02 Reinforcement Materials

- A. Comply with requirements of Section 03 2000.
- B. Reinforcing Steel: ASTM A615/A615M, Grade 60 (60,000 psi).
  - 1. Type: Deformed billet-steel bars.
  - 2. Finish: Unfinished, unless otherwise indicated.
- C. Steel Welded Wire Reinforcement: ASTM A 185/A 185M, plain type.
  - 1. Form: Flat sheets or coiled rolls.
  - 2. Mesh Size: 6"x6", unless otherwise noted on drawings.
  - 3. Wire Gage: W 4 x W 4, unless otherwise noted on drawings.
- D. Reinforcement Accessories:
  - 1. Tie Wire: Annealed, minimum 16 gauge, 0.0508 inch.
  - 2. Chairs, Bolsters, Bar Supports, Spacers: Sized and shaped for adequate support of reinforcement during concrete placement.
  - 3. Provide stainless steel, galvanized, plastic, or plastic coated steel components for placement within 1-1/2 inches of weathering surfaces.

### 2.03 Concrete Materials

- A. Cement: ASTM C 150, Type I - Normal portland type. Use one brand of cement throughout project.

1. Acquire cement for entire project from same source.
- B. Fine and Coarse Aggregates: ASTM C33/C33M.
  1. Acquire all aggregates for entire project from same source for exposed concrete only.
- C. Lightweight Aggregate: ASTM C330/C330M.
- D. Fly Ash: ASTM C618, Class C or F.
- E. Calcined Pozzolan: ASTM C618, Class N.
- F. Silica Fume: ASTM C1240, proportioned in accordance with ACI PRC-211.1.
- G. Waterproofing Additive: Crystalline waterproofing intended for mixing into concrete to close concrete pores by growth of crystals, with no decrease in concrete strength or chemical resistance.
  1. Permeability of Cured Concrete: No measurable leakage when tested in accordance with COE CRD-C 48 at 350 feet of head; provide test reports.
  2. Potable Water Contact Approval: NSF certification for use on structures holding potable water, based on testing in accordance with NSF 61.
- H. Water: ASTM C1602/C1602M; clean, potable, and not detrimental to concrete.
- I. Fiber Reinforcement: Synthetic fiber shown to have long-term resistance to deterioration when exposed to moisture and alkalis; 1/2 inch length.

#### 2.04 Admixtures

- A. Prohibited Admixtures: Calcium chloride, thiocyanates or admixtures containing more than 0.05% chloride ions are not permitted.
- B. Do not use chemicals that will result in soluble chloride ions in excess of 0.1 percent by weight of cement.
- C. Air Entrainment Admixture: ASTM C260/C260M.
- D. High Range Water Reducing and Retarding Admixture: ASTM C494/C494M Type G.
- E. High Range Water Reducing Admixture: ASTM C494/C494M Type F.
- F. Water Reducing and Accelerating Admixture: ASTM C494/C494M Type E.
- G. Water Reducing and Retarding Admixture: ASTM C494/C494M Type D.
- H. Accelerating Admixture: ASTM C494/C494M Type C.
- I. Retarding Admixture: ASTM C494/C494M Type B.
- J. Water Reducing Admixture: ASTM C494/C494M Type A.

#### 2.05 Accessory Materials

- A. Non-Shrink Cementitious Grout: Premixed compound consisting of nonmetallic aggregate, cement, water reducing and plasticizing agents.
  1. Grout: Comply with ASTM C1107/C1107M.
  2. Minimum Compressive Strength at 48 Hours, ASTM C109/C109M: 2,000 pounds per square inch.
  3. Minimum Compressive Strength at 28 Days, ASTM C109/C109M: 7,000 pounds per square inch.
- B. Non-Shrink Epoxy Grout: Moisture-insensitive, two-part; consisting of epoxy resin, nonmetallic aggregate, and activator.



**2.06 Bonding and Jointing Products**

- A. Latex Bonding Agent: Non-redispersable acrylic latex, complying with ASTM C1059/C1059M, Type II.
- B. Epoxy Bonding System:
- C. Waterproofing Admixture Slurry: Slurry coat of Portland cement, sand, and crystalline waterproofing additive, mixed with water in proportions recommended by manufacturer to achieve waterproofing at cold joints in concrete.
- D. Waterstops: Rubber, complying with COE CRD-C 513.
  - 1. Configuration: As indicated on drawings.
  - 2. Size: As indicated on drawings.
- E. Waterstops: PVC, complying with COE CRD-C 572.
  - 1. Configuration: As indicated on drawings.
  - 2. Size: As indicated on drawings.
- F. Reglets: Formed steel sheet, galvanized, with temporary filler to prevent concrete intrusion during placement.
  - 1. Size: As indicated on drawings.
- G. Slab Contraction Joint Device: Preformed linear strip intended for pressing into wet concrete to provide straight route for shrinkage cracking.
- H. Slab Construction Joint Devices: Combination keyed joint form and screed, galvanized steel, with rectangular or round knockout holes for conduit or rebar to pass through joint form at 6 inches on center; ribbed steel stakes for setting.
  - 1. Provide removable plastic cap strip that forms wedge-shaped joint for sealant installation.
  - 2. Height: To suit slab thickness.

**2.07 Curing Materials**

- A. Evaporation Reducer: Liquid thin-film-forming compound that reduces rapid moisture loss caused by high temperature, low humidity, and high winds; intended for application immediately after concrete placement.
- B. Curing Compound, Naturally Dissipating: Clear, water-based, liquid membrane-forming compound; complying with ASTM C309.
- C. Curing and Anti-Spalling Compound: Boiled linseed oil compound.
  - 1. Application: Use on roadway, bridge deck, parking deck, and ramps.
- D. Curing and Sealing Compound, Low Gloss: Liquid, membrane-forming, clear, non-yellowing acrylic; complying with ASTM C1315 Type 1 Class A.
  - 1. Vehicle: Water-based.
  - 2. Solids by Mass: 25 percent, minimum.
  - 3. VOC Content: OTC compliant.
- E. Curing and Sealing Compound, High Gloss: Liquid, membrane-forming, clear, nonyellowing acrylic; complying with ASTM C1315 Type 1 Class A.
  - 1. Vehicle: Solvent-based.
  - 2. Solids by Mass: 25 percent, minimum.
  - 3. VOC Content: Ozone Transport Commission (OTC) compliant.

- F. Moisture-Retaining Sheet: ASTM C171.
  - 1. Curing paper, regular.
  - 2. Polyethylene film, white opaque, minimum nominal thickness of 4 mil, 0.004 inch.
  - 3. White-burlap-polyethylene sheet, weighing not less than 3.8 ounces per square yard.
- G. Polyethylene Film: ASTM D2103, 4 mil, 0.004 inch thick, clear.
- H. Water: Potable, not detrimental to concrete.

## 2.08 Concrete Mix Design

- A. Proportioning Normal Weight Concrete: Comply with ACI PRC-211.1 recommendations.
  - 1. Replace as much Portland cement as possible with fly ash, ground granulated blast furnace slag, silica fume, or rice hull ash as is consistent with ACI recommendations.
- B. Proportioning Structural Lightweight Concrete: Comply with ACI 211.2 recommendations.
  - 1. Replace as much Portland cement as possible with fly ash, ground granulated blast furnace slag, silica fume, or rice hull ash as is consistent with ACI recommendations.
- C. Concrete Strength: Establish required average strength for each type of concrete on the basis of field experience or trial mixtures, as specified in ACI SPEC-301.
  - 1. For trial mixtures method, employ independent testing agency acceptable to Engineer for preparing and reporting proposed mix designs.
- D. Admixtures: Add acceptable admixtures as recommended in ACI PRC-211.1 and at rates recommended or required by manufacturer.
- E. Normal Weight Concrete:
  - 1. Compressive Strength, when tested in accordance with ASTM C39/C39M at 28 days: 3,000 pounds per square inch or as indicated on drawings.
  - 2. Calcined Pozzolan Content: Maximum 10 percent of cementitious materials by weight.
  - 3. Silica Fume Content: Maximum 5 percent of cementitious materials by weight.
  - 4. Cement Content: Minimum 5 sacks per cubic yard.
  - 5. Water-Cement Ratio: Maximum 59 percent by weight.
  - 6. Total Air Content: 4 percent, determined in accordance with ASTM C173/C173M.
  - 7. Maximum Slump: 4 inches.
  - 8. Maximum Aggregate Size: 5/8 inch.

## 2.09 Mixing

- A. On Project Site: Mix in drum type batch mixer, complying with ASTM C685/C685M. Mix each batch not less than 1-1/2 minutes and not more than 5 minutes.
- B. Transit Mixers: Comply with ASTM C94/C94M.
- C. Adding Water: If concrete arrives on-site with slump less than suitable for placement, do not add water that exceeds the maximum water-cement ratio or exceeds the maximum permissible slump.

## PART 3 EXECUTION

### 3.01 Examination

- A. Verify lines, levels, and dimensions before proceeding with work of this section.

**3.02 Preparation**

- A. Formwork: Comply with requirements of ACI SPEC-301. Design and fabricate forms to support all applied loads until concrete is cured and for easy removal without damage to concrete.
- B. Verify that forms are clean and free of rust before applying release agent.
- C. Coordinate placement of embedded items with erection of concrete formwork and placement of form accessories.
- D. Where new concrete is to be bonded to previously placed concrete, prepare existing surface by cleaning and applying bonding agent in according to bonding agent manufacturer's instructions.
  - 1. Use epoxy bonding system for bonding to damp surfaces, for structural load-bearing applications, and where curing under humid conditions is required.
  - 2. Use latex bonding agent only for non-load-bearing applications.
- E. Where new concrete with integral waterproofing is to be bonded to previously placed concrete, prepare surfaces to be treated in accordance with waterproofing manufacturer's instructions. Saturate cold joint surface with clean water, and remove excess water before application of coat of waterproofing admixture slurry. Apply slurry coat uniformly with semi-stiff bristle brush at rate recommended by waterproofing manufacturer.
- F. In locations where new concrete is doweled to existing work, drill holes in existing concrete, insert steel dowels and pack solid with non-shrink grout.

**3.03 Installing Reinforcement and Other Embedded Items**

- A. Fabricate and handle epoxy-coated reinforcing in accordance with ASTM D3963/D3963M.
- B. Comply with requirements of ACI SPEC-301. Clean reinforcement of loose rust and mill scale, and accurately position, support, and secure in place to achieve not less than minimum concrete coverage required for protection.
- C. Install welded wire reinforcement in maximum possible lengths, and offset end laps in both directions. Splice laps with tie wire.
- D. Verify that anchors, seats, plates, reinforcement and other items to be cast into concrete are accurately placed, positioned securely, and will not interfere with concrete placement.

**3.04 Placing Concrete**

- A. Place concrete in accordance with ACI PRC-304.
- B. Maintain records of concrete placement. Record date, location, quantity, air temperature, and test samples taken.
- C. Ensure reinforcement, inserts, waterstops, embedded parts, and formed construction joint devices will not be disturbed during concrete placement.
- D. Place concrete continuously without construction (cold) joints wherever possible; where construction joints are necessary, before next placement prepare joint surface by removing laitance and exposing the sand and sound surface mortar, by sandblasting or high-pressure water jetting.
- E. Finish concrete within the tolerances specified below.

**3.05 Slab Jointing**

- A. Locate joints as indicated in the geotechnical report unless otherwise shown on the drawings.

- B. Anchor joint fillers and devices to prevent movement during concrete placement.
- C. Saw Cut Contraction Joints: Saw cut joints before concrete begins to cool, within 4 to 12 hours after placing; use 3/16 inch thick blade and cut at least 1 inch deep but not less than one quarter (1/4) the depth of the slab.
- D. Construction Joints: Where not otherwise indicated, use metal combination screed and key form, with removable top section for joint sealant.
- E. Extend joint filler from bottom of slab to within 1/4 inch of finished slab surface. Conform to manufacturer's requirements for joint sealer finish.

### 3.06 Concrete Finishing

- A. Repair surface defects, including tie holes, immediately after removing formwork.
- B. Unexposed Form Finish: Rub down or chip off fins or other raised areas 1/4 inch or more in height.
- C. Exposed Form Finish: Rub down or chip off and smooth fins or other raised areas 1/4 inch or more in height. Provide finish as follows:
  - 1. Smooth Rubbed Finish: Wet concrete and rub with carborundum brick or other abrasive, not more than 24 hours after form removal.
  - 2. Grout Cleaned Finish: Wet areas to be cleaned and apply grout mixture by brush or spray; scrub immediately to remove excess grout. After drying, rub vigorously with clean burlap, and keep moist for 36 hours.
  - 3. Cork Floated Finish: Immediately after form removal, apply grout with trowel or firm rubber float; compress grout with low-speed grinder, and apply final texture with cork float.
- D. Concrete Slabs: Finish to requirements of ACI 302.1R.
- E. In areas with floor drains, maintain floor elevation at walls; pitch surfaces uniformly to drains as indicated on drawings.

### 3.07 Curing and Protection

- A. Comply with requirements of ACI PRC-308. Immediately after placement, protect concrete from premature drying, excessively hot or cold temperatures, and mechanical injury.
- B. Maintain concrete with minimal moisture loss at relatively constant temperature for period necessary for hydration of cement and hardening of concrete.
  - 1. Normal concrete: Not less than seven days.
  - 2. High early strength concrete: Not less than four days.
- C. Formed Surfaces: Cure by moist curing with forms in place for full curing period.
- D. Surfaces Not in Contact with Forms:
  - 1. Final Curing: Begin after initial curing but before surface is dry.
    - a. Moisture-Retaining Cover: Seal in place with waterproof tape or adhesive.
    - b. Curing Compound: Apply in two coats at right angles, using application rate recommended by manufacturer.

### 3.08 Field Quality Control

- A. An independent testing agency will perform field quality control tests.
- B. Provide free access to concrete operations at project site and cooperate with appointed firm.

- C. Submit proposed mix design of each class of concrete to inspection and testing firm for review prior to commencement of concrete operations.
- D. Tests of concrete and concrete materials may be performed at any time to ensure compliance with specified requirements.
- E. Compressive Strength Tests: ASTM C39/C39M, for each test, mold and cure three concrete test cylinders. Obtain test samples for every 100 cubic yards or less of each class of concrete placed.
- F. Take one additional test cylinder during cold weather concreting, cured on job site under same conditions as concrete it represents.
- G. Perform one slump test for each set of test cylinders taken, following procedures of ASTM C143/C143M.

**3.09 Defective Concrete**

- A. Test Results: The testing agency shall report test results in writing to Engineer and Contractor within 24 hours of test.
- B. Defective Concrete: Concrete not complying with required lines, details, dimensions, tolerances or specified requirements.
- C. Repair or replacement of defective concrete will be determined by the Engineer. The cost of additional testing shall be borne by Contractor when defective concrete is identified.
- D. Do not patch, fill, touch-up, repair, or replace exposed concrete except upon express direction of Engineer for each individual area.

**3.10 Schedule - Concrete Types and Finishes**

- A. Structural Walls: 3,000 psi 28 day concrete or as indicated on drawings.

**END OF SECTION**

**CONTROLLED LOW STRENGTH BACKFILL****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Contractor shall furnish all labor, materials, equipment and incidentals as shown, specified and required to provide for the proportioning, mixing, transportation and placement of Controlled Low Strength Backfill at the specified locations and dimensions shown on the plans, or as directed by the Engineer.
- B. The Controlled Low Strength Backfill shall be composed of portland cement, fly ash, natural fine aggregate and water, proportioned and mixed as herein specified.

**1.02 RELATED REQUIREMENTS**

- A. Section 31 2200 – Grading.
- B. Section 31 2316 – Excavation.
- C. Section 31 2316.13 – Trenching.
- D. Section 31 2316.14 – Trench Excavation Protection.
- E. Section 31 2323 - Fill.

**1.03 QUALITY CONTROL**

- A. Testing
  - 1. Test materials for compliance with technical requirements of the Specifications shall be performed by a testing laboratory.
  - 2. Testing Services Include:
    - a. Test the Contractor's proposed materials in the laboratory and/or field for compliance with the Specifications.
    - b. Report test results to the Engineer and the Contractor.

**1.04 PRICE AND PAYMENT PROCEDURES**

- A. Cement: By the cubic yard.

**1.05 REFERENCE STANDARDS**

- A. Comply with applicable provisions and recommendation of the following, except as otherwise shown or specified.
  - 1. ASTM C 150, Portland Cement.
  - 2. ASTM C 618, Fly Ash.
  - 3. ASTM C 494, Water Reducing Admixtures.

**1.06 SUBMITTALS**

- A. Certificates: Submit certificates of compliance with referenced standards.
- B. The testing laboratory shall submit copies of the reports directly to the Engineer, with copy to the Contractor.

**PART 2 PRODUCTS****2.01 MATERIALS**

- A. Cement: Furnish hydraulic cement that meets the requirements of TxDOT's DMS-4600, "Hydraulic Cement," TxDOT's Hydraulic Cement Quality Monitoring Program (HCQMP), and ASTM C-150 Type I Portland Cement. Sources not on the HCQMP or other sources to be used in combination with an approved source will require approval before use.

- B. Fly Ash: Furnish fly ash conforming to TxDOT DMS-4610, "Fly Ash".
- C. Chemical Admixtures: Furnish chemical admixtures conforming to TxDOT DMS-4640, "Chemical Admixtures for Concrete."
- D. Fine Aggregate: Provide fine aggregate that will stay in suspension in the mortar to the extent required for proper flow and that meets the gradation requirements of Table 1. Test fine aggregate gradation in accordance with TxDOT standard laboratory test procedure Tex-401-A. Plasticity Index (PI) must not exceed 6 when tested in accordance with TxDOT standard laboratory test procedure Tex-106-A.

TABLE 1 - GRADATION FOR FINE AGGREGATE

SIEVE SIZE	PERCENT BY WEIGHT PASSING SIEVES
3/8 inch	100
No. 8	80-100
No. 16	60-100
No. 30	45-80
No. 50	12-40
No. 100	1.5-25
No. 200	0-5

- E. Admixtures
  - 1. The use of any material added to the Controlled Low Strength Backfill shall be reviewed by the Engineer.
  - 2. Water-reducing, set-controlling admixtures shall meet the requirements of ASTM C494, Type A, water-reducing, or Type D, water reducing and retarding. Water reducing admixtures shall be added to the mixer in accordance with manufacturer's printed instructions.
- F. Water
  - 1. Water used in mixing shall be as clean and free of oil, salt, acid, alkali, sugar, vegetable, or other substances injurious to the finished product as possible.
  - 2. Water will be tested and shall meet the suggested requirements of AASHTO T26.
  - 3. Water known to be of potable quality may be used without testing.

### PART 3 EXECUTIONS

#### 3.01 MIXING AND PROPORTIONING

- A. The Contractor or Controlled Low Strength Backfill supplier shall submit for acceptance test data from prior experience if available.
- B. The supplier also must provide 5 copies of the recommended mix proportions and trial batch test results prepared by an independent testing laboratory three weeks prior to use.
- C. Submitted data should include product performance curves indicating 1, 7 and 28 day unconfined compressive strengths.
- D. Proportions must be selected to produce the specified unconfined compressive strength and workability designed below.
- E. Proportions shall be selected on the basis of unconfined compressive strength tests of specimens continuously moist cured, for testing at the age or ages specified below.

**3.02 PERFORMANCE**

- A. Strengths: for trench backfill, the Controlled Low Strength Backfill shall have an unconfined compressive strength of 21 psi minimum, no more than 24 hours after watering. The 28 day unconfined compressive strength shall not exceed 500 psi.
- B. Workability: The Controlled Low Strength Backfill shall be flowable and shall have an initial slump greater than 9" and a minimum slump of 4-½" after one hour.
- C. When a sample of Controlled Low Strength Backfill is poured out on the ground, the aggregate and mortar should stay well mixed with no separation. The Engineer may reject the mix if excessive separation is found.

**3.03 PLACING**

- A. For pipe trench backfill, provide bulkheads at units of fill placement sufficient to confine backfill to area designated in the plans. Bulkheads may be structural or earthen.
- B. During placement, station workers in a safe location where they can view inside of pipe to check for leakage. When any leakage into pipe is discovered, stop placement and repair leak before resuming placement.

**3.04 TESTING**

- A. A minimum of three test cylinders per 1000 cubic yards of Controlled Low Strength Backfill shall be drawn and tested for 7 and 28 day unconfined compressive strengths by the independent testing laboratory engaged by the Owner.

**3.05 COVERING**

- A. Controlled Low Strength Backfill placed in pipe trenches may be covered when it adequately supports the weight of construction equipment, no less than 24 hours after completion of placement.
- B. Other Controlled Low Strength Backfill should be allowed to cure for a minimum of 72 hours before covering.

**END OF SECTION**



**SITE CLEARING****PART 1 GENERAL****1.01 Section Includes**

- A. Clearing and grubbing.
- B. Selective removal and trimming.
- C. Debris removal.

**1.02 Related Requirements**

- A. Section 01 5713 - Temporary Erosion and Sediment Control.
- B. Section 02 4100.10 - Site Demolition.
- C. Section 31 1000.10 - Tree Protection.
- D. Section 31 2200 - Grading.
- E. Section 31 2323 - Fill.

**1.03 REFERENCE STANDARDS -- NOT USED****1.04 SUBMITTALS -- NOT USED**

- A. Clearing Firm Qualification Statement: Documentation of specialized experience.

**1.05 Quality Assurance**

- A. Clearing Firm Qualifications: Company specializing in performing work of type specified and with at least five years of documented experience.
- B. State and local laws and code requirements shall govern the hauling and disposal of trees, shrubs, stumps, roots, rubbish, debris and other matter.
- C. Contractor shall not clear site until a permit is obtained from the authorized regulatory agency.
- D. Air pollution caused by dust and dirt shall be controlled and comply with governing regulations.

**PART 2 PRODUCTS****PART 3 EXECUTION****3.01 Examination**

- A. Remove from site and satisfactorily dispose of all trees, shrubs, stumps, roots, brush, masonry, rubbish, scrap, debris, pavement, curbs, fences and miscellaneous other structures required to permit construction of new work.
- B. Minimize production of dust due to clearing operations; do not use water if that will result in ice, flooding, sedimentation of public waterways or storm sewers, or other pollution.

**3.02 Preparation**

- A. Coordinate work with utility companies; notify before starting work and comply with their requirements; obtain and pay for required permits.
- B. Protect existing utilities to remain from damage.
- C. Do not disrupt public utilities without permit from authority having jurisdiction.
- D. Protect existing structures and other elements that are to remain.

**3.03 Clearing and Grubbing**

- A. Scope: Remove trees, shrubs, brush, and stumps in areas to be covered by building structure, paving, landscape areas, and planting beds.
- B. Clear site after relocating vegetation in accordance with ANSI A300 Part 6.
- C. Do not remove or damage vegetation beyond limits indicated on drawings.
- D. Install substantial, highly visible chain link or orange mesh fences at least 4 feet high to prevent inadvertent damage to vegetation at the removal limits. Reference tree protection detail in drawings.
- E. In areas where vegetation must be removed but no construction will occur, remove vegetation with minimum disturbance of the subsoil.
- F. Vegetation Removed: Do not burn, bury, landfill, or leave on site, unless indicated on drawings.
  - 1. Chip, grind, crush, or shred vegetation for mulching, composting, or other purposes; preference should be given to on-site uses.
  - 2. Trees: Sell if marketable; if not, treat as specified for other vegetation removed.
  - 3. Existing Stumps: Treat as specified for other vegetation removed; completely remove stumps and roots to depth of 6 inches below subgrade.
  - 4. Fill holes left by removal of stumps and roots, using suitable fill material, with top surface neat in appearance and smooth enough not to constitute a hazard to pedestrians.
- G. Dead Wood: Remove all dead trees (standing or down), limbs, and dry brush on entire site; treat as specified for vegetation removed.
- H. Restoration: If vegetation outside removal limits or within specified protective fences is damaged or destroyed due to subsequent construction operations, replace at no cost to Owner. Contractor shall warrant damaged vegetation for 18 months.

**3.04 Selective Removal and Trimming**

- A. Selective Removal: Individual tree and shrub identified for removal as indicated on drawings according to 29 CFR 1910.266.
- B. Selective Trimming: Individual limbs and branches cut back according to ANSI A300 Part 1 identified for removal as indicated on drawings. Follow recommendations of ANSI Z133 and best local practices for species involved.

**3.05 Removed Vegetation Processing**

- A. Do not burn, bury, landfill, or leave on-site, except as indicated on drawings.
- B. Trees: Sell if marketable.

**3.06 Debris Removal**

- A. Remove debris, junk, and trash from site.
- B. Leave site in clean condition, ready for subsequent work.
- C. Clean up spillage and windblown debris from public and private lands.

**END OF SECTION**

**TREE PROTECTION****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Protection of Existing Trees.

**1.02 RELATED REQUIREMENTS**

- A. Section 02 4100.10 - Site Demolition.
- B. Section 31 1000 - Site Clearing.
- C. Section 31 2200 - Grading.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Tree Protection Fence: By the linear foot. Includes chain link or plastic mesh fence, posts, tie wire, and installation.

**1.04 REFERENCE STANDARDS**

- A. ANSI A300-2008 Pruning.
- B. Local Municipal Code.

**1.05 SUBMITTALS -- NOT USED****1.06 QUALITY ASSURANCE**

- A. Employ certified arborist or landscape architect to supervise or perform tree protection work as required.

**PART 2 PRODUCTS****2.01 MATERIALS**

- A. Tree Protection Fence: 4 feet high galvanized chain link.
  - 1. Posts: 1-1/2 inch at 6 feet on center, 2 feet deep.
  - 2. Tension Wire: Not less than 12 gauge at top and 6 inches above existing grade.
- B. Tree Protection Fence: 4 feet high heavy gauge orange plastic mesh with 2" openings.
  - 1. Posts: "T" posts at 6 feet on center, 2 feet deep.

**PART 3 EXECUTION****3.01 TREE PROTECTION FENCE**

- A. Install at Root Protection Zone of all existing trees to be protected as shown on drawing.
- B. Root Protection Zone shall be located 1 foot radius from trunk for every 1 inch diameter of trunk at 4 feet from ground. The diameter of a multi-trunk tree is calculated as the sum of the largest trunk plus half of the sum of additional trunks at 4.5 feet from ground.
- C. Fence may be located a minimum of half of the root protection radius if approved by the regulatory authority, Engineer, or Owner.
- D. Fence Location Detail: See detail as shown on drawings.

**3.02 TREE PROTECTION REQUIREMENTS**

- A. Install tree protection fence prior to any clearing, excavation, or grading and maintain in good repair for the duration of all construction work unless otherwise directed.
- B. No construction operations are allowed within the Root Protection Zone.

- C. Root Protection Zone shall be sustained in a natural state and shall be free from vehicular or mechanical traffic; no fill, equipment, liquids, or construction debris shall be placed inside the protective barrier.
- D. Root Protection Zone shall be covered with 6" of mulch to reduce moisture stress.
- E. The proposed finished grade and elevation of land within the Root Protection Zone of any trees to be preserved shall not be raised or lowered more than 3 inches. Welling and retaining methods are allowed outside the Root Protection Zone.
- F. Root Protection Zone shall remain pervious, i.e. ground cover or turf at completion of landscape design.
- G. No roots may be cut closer than 6 feet from the base of any tree. Roots cut within the Root Protection Zone will only be allowed on one side of the tree. Any roots that need to be cut within the Root Protection Zone will be cut using a saw-type trencher, and all cut roots will be painted.
- H. All trees impacted by construction shall be fertilized with an organic tree fertilizer prior to construction and again at the end of construction. The area within the protective fencing shall be mulched with about 6 inches of mulch. Water barrels shall be placed within the Root Protection Zone to irrigate these trees if necessary.
- I. No trash or warming fires shall be placed within 50 feet of any tree.
- J. No pedestrian traffic shall occur within dripline of any tree.

### **3.03 DAMAGE TO PROTECTED TREES**

- A. Trim trees and shrubs when doing so will prevent removal or damage. Trimmed or damaged trees shall be treated or repaired under supervision of a certified arborist or landscape architect.
- B. Any damage done to existing tree crowns or root systems shall be repaired immediately under supervision of a certified arborist. All wounds to oaks shall be painted with pruning paint within 20 minutes after damage. Roots exposed during construction operations will be cut cleanly. Cut surfaces shall be painted and topsoil and mulch placed over exposed root area immediately.
- C. Branch Pruning Detail: See detail as shown on drawings.
- D. Contractor shall compensate owner for damage to existing trees designated to remain in the amount of \$200 per caliper inch measured 4 feet from ground. This amount will be deducted from final payment.

**END OF SECTION**

**GRADING****PART 1 GENERAL****1.01 Section Includes**

- A. Removal of topsoil.
- B. Rough grading the site.
- C. Fine grading.
- D. Finish grading.

**1.02 Related Requirements**

- A. Section 31 1000 - Site Clearing.
- B. Section 31 1000.10 - Tree Protection.
- C. Section 31 2316 - Excavation.
- D. Section 31 2323 - Fill.
- E. Project Geotechnical Report.

**1.03 Price and Payment Procedures**

- A. Topsoil:
  - 1. Measurement Method: By the cubic yard.
  - 2. Includes: scarifying substrate surface, placing where required, and compacting.

**1.04 Submittals**

- A. Project Record Documents: Accurately record actual locations of utilities remaining by horizontal dimensions, elevations or inverts, and slope gradients.

**1.05 Quality Assurance**

- A. Perform Work in accordance with available geotechnical engineering and landscape specifications.

**PART 2 PRODUCTS****2.01 Materials**

- A. Gravel: Excavated on-site.
  - 1. Graded according to ASTM D2487 Group Symbol GW, GP, or SP.
- B. Topsoil: Topsoil excavated on-site; friable loam, imported borrow; local borrow.
  - 1. Graded.
  - 2. Free of roots, rocks larger than 1/2 inch, subsoil, debris, large weeds and foreign matter.
- C. Other Fill Materials: See Section 31 2323.

**PART 3 EXECUTION****3.01 Examination**

- A. Verify survey bench mark and intended elevations for grading areas are as indicated.
- B. Verify the absence of standing or ponding water.

**3.02 Preparation**

- A. Identify required lines, levels, contours, and datum.
- B. Stake and flag locations of known utilities.

- C. Locate, identify, and protect above- and below-grade utilities to remain.
- D. Notify utility company to remove and relocate utilities.
- E. Provide temporary means and methods to remove standing or ponding water from areas prior to grading.
- F. Protect site features to remain, including but not limited to bench marks, survey control points, and fences.
- G. Protect trees to remain. Reference Section 31 1000.10.
- H. Protect features to remain as a portion of final landscaping.

### **3.03 Rough Grading**

- A. Excavate and fill subgrade material to elevations indicated on plans.
- B. Remove topsoil from areas to be further excavated, without mixing with foreign materials.
- C. Do not remove topsoil when wet.
- D. Remove subsoil from areas to be further excavated.
- E. Do not remove wet subsoil, unless it is subsequently processed to obtain optimum moisture content.
- F. When excavating through roots, perform work by hand and cut roots with sharp axe.
- G. See Section 31 2323 for filling procedures.
- H. Benching Slopes: Horizontally bench slopes greater than 4:1 to key fill material to slope for firm bearing.
- I. Replace displaced subgrade in accordance with Section 31 2323.
- J. Remove and replace unsuitable materials as specified fill.

### **3.04 Fine Grading**

- A. Scrape and spread subgrade material uniformly smooth and without disruptions as indicated on drawings.

### **3.05 SOIL REMOVAL**

- A. Stockpile topsoil to be re-used on site; remove remainder from site.
- B. Stockpile subsoil to be re-used on site; remove remainder from site.
- C. Stockpiles: Use areas designated on site; pile depth not to exceed 8 feet; protect from erosion.

### **3.06 FINISH GRADING**

- A. Before Finish Grading:
  - 1. Verify building and trench backfilling have been inspected.
  - 2. Verify subgrade has been contoured and compacted.
- B. Remove debris, roots, branches, stones, in excess of 1-1/2 inch in size. Remove soil contaminated with petroleum products.
- C. Where topsoil is to be placed, scarify surface to depth of 3 inches.
- D. In areas where vehicles or equipment have compacted soil, scarify surface to depth of 3 inches.
- E. Place topsoil in areas indicated on drawings.

- F. If not otherwise indicated, place topsoil to the following compacted thicknesses:
  - 1. Areas to be Seeded with Grass: 4 inches.
  - 2. Areas to be Sodded: 4 inches.
- G. Place topsoil during dry weather.
- H. Remove roots, weeds, rocks, and foreign material while spreading.
- I. Near plants, buildings, and curbs spread topsoil manually to prevent damage.
- J. Fine grade topsoil to eliminate uneven areas and low spots. Maintain profiles and contour of subgrade.
- K. Lightly compact placed topsoil.
- L. Maintain stability of topsoil during inclement weather. Replace topsoil in areas where surface water has eroded thickness below specifications.

**3.07 Tolerances**

- A. Top Surface of Subgrade: Plus or minus 0.10 foot (1-3/16 inches) from required elevation.
- B. Top Surface: Plus or minus 1/2 inch.

**3.08 REPAIR AND RESTORATION**

- A. Existing Facilities, Utilities, and Site Features to Remain: If damaged due to this work, repair or replace to original condition.
- B. Trees to Remain: If damaged due to this work, trim broken branches and repair bark wounds; if root damage has occurred, obtain instructions from Engineer as to remedy.
- C. Other Existing Vegetation to Remain: If damaged due to this work, replace with vegetation of equivalent species and size.

**3.09 FIELD QUALITY CONTROL**

- A. See Section 31 2323 for compaction density testing.

**3.10 Cleaning**

- A. See Section 01 7000 - Execution and Closeout Requirements for additional requirements.
- B. Remove unused stockpiled subsoil. Grade stockpile area to prevent standing water.
- C. Leave site clean and raked, ready to receive work.

**END OF SECTION**

**EXCAVATION****PART 1 GENERAL****1.01 Section Includes**

- A. Excavating for site grading, paving, structures.
- B. Trenching for utilities from 5 feet outside the building to utility main connections.
- C. Excavation.

**1.02 Related Requirements**

- A. Section 01 5713 - Temporary Erosion and Sediment Control.
- B. Section 31 2200 - Grading.
- C. Section 31 2323 - Fill.
- D. Section 31 2316.13 - Trenching.
- E. Section 31 2316.14 - Trench Excavation Protection.
- F. Project Geotechnical Report.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Excavating Soil Materials:
  - 1. Measurement method: By the cubic yard measured before removal.

**PART 2 PRODUCTS - NOT USED****PART 3 EXECUTION****3.01 Examination**

- A. Verify survey bench mark elevations are as indicated on drawings.
- B. Verify that existing topography is as shown in the plans. Coordinate with the engineer for any discrepancies prior to start of excavation.

**3.02 Preparation**

- A. Identify required lines, levels, contours, and datum locations.
- B. Excavate and backfill, in advance of construction, test pits to determine conditions or location of existing utilities.
- C. Locate and identify known utilities to remain and protect from damage.
- D. Notify utility company to remove and relocate utilities.
- E. Protect bench marks, survey control points, existing structures, fences, sidewalks, paving, curbs, and existing utilities from excavating equipment and vehicular traffic. Repair damage at no additional charge to Owner, including utility company charges.
- F. Protect plants, lawns, rock outcroppings, and other features to remain.
- G. Grade top perimeter of excavation to prevent surface water from draining into excavation. Provide temporary means and methods, as required, to maintain surface water diversion until no longer needed, or as directed by Engineer.

**3.03 Excavation**

- A. General Excavation:
  - 1. Excavate to indicated contours, elevations, and grades.



- B. Excavate to accommodate construction operations and to lines and grades indicated on the drawings.
- C. Notify Engineer of unexpected subsurface conditions and discontinue affected Work in area until notified to resume work.
- D. Excavate to provide adequate work space and clearance for concrete forms. Do not undercut excavation face for extended footings.
- E. Steep slope and trench excavations shall conform with OSHA standards for shoring and safety protection.
- F. Do not interfere with 45-degree bearing splay of foundations.
- G. Cut utility trenches wide enough to allow inspection of installed utilities. Reference Sections 31 2316.13 and .
- H. Remove lumped subsoil, boulders, and rock up to 1/3 cu yd measured by volume.
- I. Provide temporary means and methods, as required, to remove all water from excavations until directed by Engineer. Remove and replace soils deemed suitable by classification and which are excessively moist due to lack of dewatering or surface water control.
- J. Stockpile excavated material to be re-used in area designated on site.
- K. Remove excess excavated material from site. Transport and place in accordance with all applicable regulations. Do not dispose of excess material in any stream or watercourse. Do not dump excess material on public property. Do not dispose of excess material on private property unless authorized by Owner.
- L. Structure and Roadway Excavations:
  - 1. Subgrade shall be firm, dense, and compacted to 95 percent maximum density at a moisture content between optimum and optimum plus or minus 4 percent unless otherwise indicated in the Project Geotechnical Report.
  - 2. Bottom of excavations for footings and slabs shall be level, clean, dry, and clear of loose material.
  - 3. Remove unsuitable material and replace with suitable material as required or directed by Testing Laboratory.
  - 4. Refill over-excavated areas with properly compacted select backfill material.
  - 5. Extend excavation 5 feet minimum on each side of structure or footing unless otherwise indicated on drawings.
  - 6. Proof roll exposed design subgrade using a 25 ton pneumatic tire roller, maintaining a minimum tire pressure of 75 psi. Proof rolling operation shall be inspected by Testing Laboratory. Any soft or unconsolidated zones or areas detected by proof rolling operations shall be undercut as directed by the Engineer or the Testing Laboratory. Undercut subgrade shall be scarified to a minimum depth of six inches and compacted to a minimum of 95 percent maximum density at a moisture content between optimum and optimum plus 4 percent unless otherwise indicated in the Project Geotechnical Report. After the undercut subgrade has been scarified and compacted, the undercut shall be backfilled with select backfill to the design subgrade elevation. The final subgrade shall be reviewed by the Engineer or the Testing Laboratory.
  - 7. The surface of the subgrade for street excavations shall be finished to the lines and grades as established, and be in conformity with the typical sections shown on the plans. Any deviation in excess of one-half inch in cross section, and in a length of sixteen feet,

measured longitudinally, shall be corrected by loosening, adding, or removing material, reshaping and compacting by sprinkling and rolling.

**3.04 Field Quality Control**

- A. Provide access for visual inspection of loadbearing excavated surfaces by Engineer before proceeding with work.

**3.05 Protection**

- A. Divert surface water away from excavations.
- B. Maintain stability of banks and loose soils; prevent from falling into excavations.
- C. Maintain excavations in satisfactory, undisturbed condition.
- D. Protect bottom of excavations and exposed soil against physical disturbance, rain, and freeze.
- E. Keep excavations free of standing water and completely free of water during concrete placement.
- F. Protect footing excavations; construct concrete footings same day excavation is made wherever possible.

**END OF SECTION**

**TRENCHING****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Trench excavation.
- B. Backfill and compaction.
- C. Excavating, backfilling and compacting for utilities outside of any structure to utility main connections or storm drainage outfalls.

**1.02 RELATED REQUIREMENTS**

- A. Project Geotechnical Report.
- B. Section 03 3000.10 - Controlled Low Strength Backfill.
- C. Section 31 2200 - Grading.
- D. Section 31 2316 - Excavation.
- E. Section 31 2323 - Fill.
- F. Section 31 2316.14 - Trench Excavation Protection.
- G. Section 33 4600 - Subdrainage.

**1.03 Price and Payment Procedures**

- A. Unit Prices:
  - 1. See Section 01 2200 - Unit Prices for additional requirements.
  - 2. Basis of Measurement for Trenching: By linear foot.
- B. Excavating Soil Materials:
  - 1. Measurement method: By the cubic yard.
  - 2. Includes: Excavating to required elevations, loading and placing materials in stockpile or removing from site.
  - 3. Does Not Include Over-Excavation: Payment will not be made for over-excavated work nor for replacement materials.
- C. General Fill:
  - 1. Measurement Method: By the cubic yard.
  - 2. Includes: Excavating existing soil, stockpiling, scarifying substrate surface, placing where required, compacting, and dewatering.
- D. Structural Fill:
  - 1. Measurement Method: By the cubic yard.
  - 2. Includes: Excavating existing soil, stockpiling, scarifying substrate surface, placing where required, and compacting.
- E. Granular Fill:
  - 1. Measurement Method: By the cubic yard.
  - 2. Includes: Excavating existing material, stockpiling, scarifying substrate surface, placing where required, compacting, and dewatering.
- F. Aggregates:
  - 1. Measurement Method: By the cubic yard.
  - 2. Includes: Excavating existing material, stockpiling, scarifying substrate surface, placing where required, compacting, and dewatering.

**1.04 DEFINITIONS**

- A. Finish Grade Elevations: Top of paving or proposed contours indicated on drawings.
- B. Subgrade Elevations: Bottom of paving section indicated on drawings.

**1.05 REFERENCE STANDARDS**

- A. AASHTO T 180 - Standard Specification for Moisture-Density Relations of Soils Using a 4.54 kg (10-lb) Rammer and a 457 mm (18 in.) Drop; 2010.
- B. ASTM C136/C136M - Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates; 2014.
- C. ASTM D698 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft<sup>3</sup> (600 kN-m/m<sup>3</sup>)); 2012.
- D. ASTM D1556 - Standard Test Method for Density and Unit Weight of Soil in Place by the Sand-Cone Method; 2007.
- E. ASTM D1557 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft<sup>3</sup> (2,700 kN m/m<sup>3</sup>)); 2012.
- F. ASTM D2167 - Standard Test Method for Density and Unit Weight of Soil in Place by the Rubber Balloon Method; 2008.
- G. ASTM D2487 - Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System); 2011.
- H. ASTM D 2922 - Standard Test Methods for Density of Soil and Soil-Aggregate in Place by Nuclear Methods (Shallow Depth); 2005.
- I. ASTM D4318 - Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils; 2010.
- J. TxDOT TEX-113-E - Laboratory Compaction Characteristics and Moisture-Density Relationship of Base Materials; 2010.
- K. TxDOT TEX-114-E - Laboratory Compaction Characteristics and Moisture-Density Relationship of Subgrade, Embankment Soils, and Backfill Material; 2005.
- L. NFPA 24 - Installation of Private Fire Service Mains and their appurtenances.

**1.06 Submittals**

- A. Samples: sample of each type of fill; submit each material sample in three 5-gallon air-tight containers to testing laboratory.
- B. Source Quality Control Submittals: Submit name of imported materials source.
  - 1. Results of gradation tests on proposed and actual materials used.
- C. Materials Sources: Submit name of imported materials source.
- D. Fill Composition Test Reports: Results of laboratory tests on proposed and actual materials used.
- E. Compaction Density Test Reports.

**1.07 Delivery, Storage, and Handling**

- A. See Section 01 7419 - Construction Waste Management and Disposal for packaging waste requirements.
- B. Deliver fill to project site in advance of need.

- C. When fill materials need on-site storage, locate stockpiles where indicated on drawings.
  - 1. Separate differing materials with dividers or stockpile separately to prevent intermixing.
  - 2. Prevent contamination.
  - 3. Protect stockpiles from erosion and deterioration of materials.
  - 4. Provide positive drainage away from stockpiled material.
  - 5. Install erosion control measures around stockpiles as needed.

## **PART 2 PRODUCTS**

### **2.01 Fill Materials**

- A. General Fill: Comprised of sand and gravel; free of shale, clay, friable materials, and debris.
- B. General Fill: Subsoil excavated on-site or imported borrow.
  - 1. For imported borrow, gradation less than 15 percent passing No. 200 sieve as determined by ASTM D 1140.
  - 2. For imported borrow, plasticity index less than 20 percent as determined by ASTM D 4318.
  - 3. No organic matter or debris.
  - 4. For subsoil excavated onsite, reference Geotechnical Engineering Report.
- C. Structural Fill:
  - 1. Reference Geotechnical Engineering Report.
  - 2. If a Geotechnical Engineering Report is not available then comply with TxDOT Item 247 Type A Grade 2 Base.
  - 3. Graded in accordance with the following limits unless otherwise indicated:
    - a. 1-3/4 inch sieve: 90-100 percent passing.
    - b. No. 4 sieve: 25-55 percent passing.
    - c. No. 40 sieve: 15-40 percent passing.
  - 4. Mixture shall be crushed stone and contain no clay lumps or organic matter.
  - 5. Fraction passing No. 40 sieve shall have a liquid limit less than 40 and a plasticity index less than 12 as determined by ASTM D 4318.
- D. Concrete for Fill: As specified in Section 03 3000.10 - Controlled Low Strength Backfill.
- E. Granular Fill : Crushed limestone or pea gravel ; free of shale, clay, friable material and debris.
  - 1. Graded in accordance with ASTM C136/C136M, within the following limits:
    - a. 1/2 inch sieve: 95 percent passing.
    - b. No. 4 sieve: 5 percent passing.
- F. Select Initial Backfill or Bedding Material: Clean, well graded crushed stone or gravels, crushed screenings or sand.
  - 1. Modified Grade 5:
    - a. 1/2 inch sieve: 100 percent passing.
    - b. 3/8 inch sieve: 100 to 95 percent passing.
    - c. No. 4 sieve: 80 to 20 percent passing.
    - d. No. 10 sieve: less than 25 percent passing.
    - e. No. 20 sieve: less than 2 percent passing.
  - 2. Plasticity index 12 or less as determined by TxDOT Test Method TEX-106-E.

**2.02 Accessories**

- A. Geotextile Fabric: Non-biodegradable.

**2.03 Source Quality Control**

- A. Test fill materials in accordance with specified standard before delivery to site.
- B. Nonconforming Materials: Change and retest.
- C. Provide materials of each type from same source or as directed by Engineer.

**PART 3 EXECUTION****3.01 EXAMINATION**

- A. Verify that survey bench marks and intended elevations for the work are as indicated.
- B. Identify required lines, levels, contours, and datum locations.

**3.02 Preparation**

- A. Locate and identify existing utilities to remain as indicated on drawings and protect from damage.
- B. Notify utility company to remove and relocate utilities as indicated on plans.
- C. Protect bench marks, survey control points, existing structures, fences, sidewalks, paving, and curbs from excavating equipment and vehicular traffic.
- D. Protect other features to remain.
- E. Grade top perimeter of trenching area to prevent surface water from draining into trench. Provide temporary means and methods, as required, to maintain surface water diversion until no longer needed, or as directed by the Engineer.

**3.03 TRENCHING**

- A. Grade top perimeter of excavation to prevent surface water collection.
- B. Notify Engineer of unexpected subsurface conditions and discontinue affected work in area until notified to resume.
- C. General: Cut trenches neat and clean.
  - 1. Slope banks of excavations deeper than 4 feet to angle of repose or less until shored.
- D. Utility Preparation: Rake trench bottom to uniform grade.
  - 1. Remove unsuitable subgrade and backfill.
  - 2. Compact subgrade to density equal to or greater than subsequent fill material requirements.
- E. Maintain trenches and prevent loose soil or rocks from entering.
- F. Notify Engineer of unexpected subsurface conditions and discontinue affected Work in area until notified to resume work.
- G. Banks of excavations may be cut back on slopes to angle of repose or less until shored. Slope shall not extend lower than 12 inches above top of pipe.
- H. Do not interfere with 45 degree bearing splay of foundations.
- I. Cut trenches wide enough to allow proper installation, jointing, embedment, and inspection of utilities. See drawing details for minimum trench width.
- J. Hand trim excavations. Remove loose matter.

- K. Remove large stones and other hard matter that could damage piping or impede consistent backfilling or compaction.
- L. Remove lumped subsoil, boulders, and rock up to 1/3 cubic yard measured by volume. See Section 31 2316.26 for removal of larger material.
- M. Excavate pipe trenches six inches (6") below the underside of the pipe to provide for the installation of a granular fill pipe foundation material except where otherwise required.
- N. Where in earth, trench bottoms for pipe six inches (6") or smaller, may be graded to provide uniform and continuous support (between bell holes or end joints) of the installed pipe.
- O. Remove excavated material that is unsuitable for re-use from site.
- P. Stockpile excavated material to be re-used in area designated on site. Stockpile material a sufficient distance from the banks of the trench to avoid overloading and to prevent slides or cave-ins.
- Q. Remove excess excavated material from site.
- R. Provide temporary means and methods, as required, to remove all water from trenching until directed by the Engineer. Remove and replace soils deemed unsuitable by classification and which are excessively moist due to lack of dewatering or surface water control.
- S. Determine the prevailing groundwater level prior to trenching. If the proposed trench extends less than 1 foot into the prevailing groundwater, control groundwater intrusion with perimeter drains routed to sump pumps, or as directed by the Engineer.

#### **3.04 PREPARATION FOR UTILITY PLACEMENT**

- A. Cut out soft areas of subgrade not capable of compaction in place. Backfill with properly compacted structural fill.
- B. Backfill over-depths in trench excavation with properly compacted structural fill.
- C. Compact subgrade to density equal to or greater than requirements for subsequent fill material.
- D. Provide adequate clearance at bell holes for tools and methods used in installing pipe. No part of any bell or coupling shall be in contact with the trench bottom, trench walls, or granular embedment when the pipe is jointed.
- E. Where existing piping constructed by others cross the new pipeline trench excavation, the existing piping or ductbank shall be adequately supported and protected from damage due to construction. All methods for supporting and maintaining these facilities shall be subject to review by the Engineer or the Testing Laboratory. Backfill between utility crossings shall meet the requirements of standard backfill compact. See Section 03 3000.10 for low strength flowable fill option.
- F. Until ready to backfill, maintain excavations and prevent loose soil from falling into excavation. Prevent surface water from flowing into trenches. Any water accumulating in trenches shall be removed by pumping or other approved methods.
- G. If this project is within the Edwards Aquifer Recharge Zone, follow all requirements and recommendations of approved Water Pollution Abatement Plan (WPAP) and Sewer Collection System (SCS), including, plans, reports, and TCEQ approval letters. If, during construction, any recharge features are found, all activities near the feature must be suspended immediately and the Engineer must be notified. No activities near the feature may proceed until TCEQ has approved a plan to address the feature. If any sanitary sewer lines cross geologic fault lines,

as indicated in the SCS plans and report, contact the Engineer 48 hours prior to excavating in the vicinity of the fault. The Engineer's geologist is required to assess such faults during excavation.

### 3.05 BACKFILLING

- A. See Section 31 2323 for general backfill requirements.
- B. Backfill is divided into three (3) separate zones:
  - 1. Bedding: The material in trench bottom in direct contact with the bottom of the pipe.
  - 2. Initial backfill: The backfill zone extending from the surface of the bedding to a point one foot (1') above the top of the pipe.
  - 3. Secondary backfill: The backfill zone extending from the initial backfill surface to the top of the trench. Placement of materials for each of the zones is described herein.
- C. Bedding
  - 1. When unacceptable materials such as water, silt, muck, trash or debris, or rock boulder or coarse gravel (particle size greater than 1 ¾ inch) exist at the bearing level or for pipes with a nominal inner diameter greater than six inches (6"), use a bedding of granular embedment material.
  - 2. Unstable materials shall be removed at the direction of the Engineer and replaced to a minimum depth of four inches (4") or one-eighth (1/8) of the outside diameter of the pipe, whichever is greater, with granular embedment material. Extend this material up to the sides of the pipe sufficiently to embed the lower quadrant of the pipe. If stability is not accomplished by using the above procedure, the Engineer may require additional granular embedment.
  - 3. Spread and grade granular embedment to provide a uniform and continuous bedding zone beneath the pipe at all points between bell holes or pipe joints. It will be permissible to slightly disturb the finished subgrade surface to withdraw pipe slings or other lifting tackle. After each pipe has been graded, aligned, and shoved home, deposit and compact sufficient pipe embedment material under and around each side of the pipe and back of the bell or end thereof to hold the pipe in proper position and alignment during subsequent pipe jointing and embedment operations. Deposit and compact embedment material uniformly and simultaneously on each side of the pipe to prevent lateral displacement.
  - 4. Compact each layer of embedment material by at least two complete coverages of all portions of the surface of each lift using adequate compaction equipment. One coverage is defined as the conditions reached when all portions of the lift fill have been subjected to the direct contact of the compacting surface of the compactor.
  - 5. The method of compaction and the equipment used shall be appropriate for the material to be compacted and shall not transmit damaging shocks to the pipe.
- D. Initial Backfill: Initial backfill is defined as backfill having a thickness in its compacted state from the surface of the bedding to a point one foot (1') above the top of the pipe. Initial backfill shall be constructed in accordance with details shown on the plans and these specifications.
  - 1. Select Initial Backfill: Where pipe is to be laid in a rock cut or where rock in boulder ledge or coarse gravel (particle size larger than 1¾ inch) formations exist in the initial backfill zone, or where trench walls or conditions are unstable or where the pipe to be laid is flexible pipe, use granular embedment for initial backfill.



2. For conduits less than twenty-four inches (24") in diameter select initial backfill material shall be placed in two (2) lifts. The first lift shall be spread uniformly and simultaneously on each side and under the shoulders of the pipe to the mid-point or spring line of the pipe. The first lift of select initial backfill shall be inspected and approved prior to placement of the second lift. The second lift of select initial backfill material shall extend from the spring line of the pipe to a depth sufficient to produce a compacted depth of material a minimum of one foot (1') above the top of the pipe. The second lift shall be evenly spread in a similar manner as the first lift.
  3. For conduits twenty-four (24") in diameter and larger, select initial backfill material shall be evenly and simultaneously spread alongside, under the shoulders or haunches of the pipe and over the pipe in six-inch (6") lifts to a point sufficient to produce a compacted depth of material a minimum of one foot (1') above the top of the pipe.
  4. Natural Initial Backfill: Where the pipe to be laid is rigid pipe and where stable materials and laying conditions exist at the pipe bearing level and initial backfill zone and existing excavated materials are acceptable to the Engineer, such excavated natural materials may be utilized as initial backfill material.
  5. For dedicated water lines used for fire protection and trenched in rock, tamped initial backfill and bedding material shall be used for at least 6 inches under and around the pipe and for at least 2 feet above the pipe.
- E. Secondary Backfill: Secondary backfill is defined as backfill from one foot (1') above the top of the pipe of the trench. Secondary backfill shall be constructed in accordance with details shown on the plans and these specifications.
1. Secondary backfill shall generally consist of materials removed from the trench and shall be free of trash brush and other debris. No rock or stones having any dimension larger than one half of the trench width, or four inches (4"), whichever is less, shall be used in the secondary backfilling zone. In special cases where excessive width and/or depth of the trench permit, and only with approval of the Engineer, larger rocks up to six inches (6") in diameter may be incorporated into the backfill provided that the surrounding compactable soil may be properly and adequately compacted.
- F. General Fill: Place and compact materials in equal continuous layers not exceeding 8 inches compacted depth.
- G. Slope grade away from building minimum 2 inches in 10 feet, unless noted otherwise. Make gradual grade changes. Blend slope into level areas.
- H. Restore the surface of the backfilled trench, if not disturbed by surrounding construction, to match previous existing conditions.
- I. Compaction Density unless otherwise indicated in the project Geotechnical Report:
1. Compaction to meet the requirements of TxDOT TEX-114-E.
  2. Lift Thickness: 8 inches.
  3. Moisture Content: Between optimum and optimum +4 percent.
  4. Testing laboratory will perform density tests at completion of each lift.
  5. If the tests indicate unsatisfactory compaction, the Contractor shall provide the additional compaction necessary to obtain the specified degree of compaction. All additional compaction work shall be performed by the Contractor at no additional cost to the Owner until the specified compaction is obtained. This work shall include complete removal of unacceptable (as determined by the Testing Laboratory) fill areas and replacement and recompaction until acceptable fill is provided.

**3.06 BEDDING AND FILL AT SPECIFIC LOCATIONS**

- A. Use general fill unless otherwise specified or indicated.

**3.07 Tolerances**

- A. Maximum Variation from Top Surface of General Backfilling: Plus or minus 1 inch from required elevations.

**3.08 Cleaning**

- A. See Section 01 7000 - Execution and Closeout Requirements for additional requirements.
- B. Stockpile excavated material re-used in area designated on-site; see Section 31 2200.
- C. Remove excavated material unsuitable for re-use from site.
- D. Remove excess excavated material from site.
- E. Leave unused materials in a neat, compact stockpile.
- F. Remove unused stockpiled materials, leave area in a clean and neat condition. Grade stockpile area to prevent standing surface water.
- G. Leave borrow areas in a clean and neat condition. Grade to prevent standing surface water.

**END OF SECTION**

**TRENCH EXCAVATION PROTECTION****PART I - GENERAL****1.01 SECTION INCLUDES:**

- A. Trench Excavation Protection required for the construction of all trench excavation protection systems to be utilized in the project and including all additional excavation and backfill necessitated by the protection and backfill necessitated by the protection system.

**1.02 MEASUREMENT AND PAYMENT**

- A. Trench Excavation Protection is to be included in the cost of installation of trenched underground utilities.

**1.03 RELATED SECTIONS**

- A. Trench Excavation Protection shall be accomplished as required by the provisions of Part 1926, Subpart P - Excavations, Trenching, and Shoring of the Occupational Safety and Health Administration Standards and Interpretations.
- B. A copy of the Document is available for review at the office of the Engineer. It shall be construed that this document is included in this Project Manual and shall apply to every Section as if written in full therein.

**PART 2 - PRODUCTS -- NOT USED****PART 3 - EXECUTION****3.01 CONSTRUCTION METHODS**

- A. Trench Excavation Protection shall be accomplished as required by the provisions of, Part 1926, Subpart P - Excavations, Trenching, and Shoring of the Occupational Safety and Health Administration Standards and Interpretations.

**END OF SECTION**

**FILL****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Filling, backfilling, and compacting for building volume below grade, footings, slabs-on-grade, paving, and utilities within the building.
- B. Filling holes, pits, and excavations.

**1.02 RELATED REQUIREMENTS**

- A. Section 31 2200 - Grading.
- B. Section 31 2316 - Excavation.
- C. Section 31 2316.13 - Trenching.
- D. Section 32 9119 - Landscape Grading.
- E. Project Geotechnical Report.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. General Fill:
  - 1. Measurement Method: By the cubic yard.
  - 2. Includes: Excavating existing soil, stockpiling, scarifying substrate surface, placing where required, compacting, and dewatering.
- B. Structural Fill:
  - 1. Measurement Method: By the cubic yard.
  - 2. Includes: Excavating existing soil, stockpiling, scarifying substrate surface, placing where required, compacting, and dewatering.
- C. Granular Fill:
  - 1. Measurement Method: By the cubic yard.
  - 2. Includes: Excavating existing material, stockpiling, scarifying substrate surface, placing where required, compacting, and dewatering.
- D. Aggregates:
  - 1. Measurement Method: By the cubic yard.
  - 2. Includes: Excavating existing material, stockpiling, scarifying substrate surface, placing where required, compacting, and dewatering.

**1.04 DEFINITIONS**

- A. Finish Grade Elevations: Indicated on drawings.
- B. Subgrade Elevations: Indicated on drawings.

**1.05 REFERENCE STANDARDS**

- A. AASHTO T 180 - Standard Specification for Moisture-Density Relations of Soils Using a 4.54 kg (10-lb) Rammer and a 457 mm (18 in.) Drop; 2010.
- B. ASTM C136/C136M - Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates; 2014.
- C. ASTM D698 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft<sup>3</sup> (600 kN-m/m<sup>3</sup>)); 2012.
- D. ASTM D1556 - Standard Test Method for Density and Unit Weight of Soil in Place by the Sand-Cone Method; 2007.

- E. ASTM D1557 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft<sup>3</sup> (2,700 kN m/m<sup>3</sup>)); 2012.
- F. ASTM D2167 - Standard Test Method for Density and Unit Weight of Soil in Place by the Rubber Balloon Method; 2008.
- G. ASTM D2487 - Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System); 2011.
- H. ASTM D 2922 - Standard Test Methods for Density of Soil and Soil-Aggregate in Place by Nuclear Methods (Shallow Depth); 2005.
- I. ASTM D4318 - Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils; 2010.
- J. TxDOT TEX-113-E, Laboratory Compaction Characteristics and Moisture-Density Relationship of Base Materials, using 5.5 lb rammer and 12-inch drop.

#### **1.06 SUBMITTALS**

- A. Samples: sample of each type of fill; submit each material sample in three 5-gallon air-tight containers to testing laboratory.
- B. Materials Sources: Submit name of imported materials source.
- C. Fill Composition Test Reports: Results of laboratory tests on proposed and actual materials used, including manufactured fill.
- D. Compaction Density Test Reports.

#### **1.07 DELIVERY, STORAGE, AND HANDLING**

- A. When necessary, store materials on site in advance of need.
- B. When fill materials need to be stored on site, locate stockpiles where designated.
  - 1. Separate differing materials with dividers or stockpile separately to prevent intermixing.
  - 2. Prevent contamination.
  - 3. Protect stockpiles from erosion and deterioration of materials.

### **PART 2 PRODUCTS**

#### **2.01 FILL MATERIALS**

- A. General Fill: Subsoil excavated onsite, imported borrow.
  - 1. For imported borrow, gradation less than 15 percent passing No. 200 sieve as determined by ASTM D 1140.
  - 2. For imported borrow, plasticity index less than 20 percent as determined by ASTM D 4318.
  - 3. No vegetative matter or debris.
  - 4. No rocks larger than half of the lift thickness.
- B. Structural Fill:
  - 1. Refer to Geotechnical Engineering Report.
  - 2. If a Geotechnical Engineering Report is not available then comply with TxDOT Item 247 Type A Grade 2 Base.
  - 3. Graded in accordance with the following limits unless otherwise indicated:
    - a. 1-3/4 inch sieve: 90-100 percent passing.
    - b. No. 4 sieve: 25-55 percent passing.
    - c. No. 40 sieve: 15-40 percent passing.

4. Mixture shall be crushed stone and contain no clay lumps or organic matter.
  5. Fraction passing No. 40 sieve shall have a liquid limit less than 40 and a plasticity index less than 12 as determined by ASTM D 4318.
- C. Granular Fill : Crushed limestone or pea gravel ; free of shale, clay, friable material and debris.
1. Graded in accordance with ASTM C136/C136M, within the following limits:
    - a. 1/2 inch sieve: 95 percent passing.
    - b. No. 4 sieve: 5 percent passing.
- D. Sand: Natural river or bank sand; free of silt, clay, loam, friable or soluble materials, and organic matter.
1. Graded in accordance with ASTM C136/C136M; within the following limits:
    - a. No. 4 sieve: 100 percent passing.
    - b. No. 200 sieve: 0 to 10 percent passing.
- E. Drain Gravel: Washed gravel.
1. Material shall have an LA abrasion number of 35 or less.
  2. Graded within the following limits:
    - a. 2 inch sieve: 100 percent passing.
    - b. 1-1/2 inch sieve: 90-100 percent passing.
    - c. 1 inch sieve: 25-55 percent passing.
    - d. 1/2 inch sieve: 0-10 percent passing.
    - e. 1/4 inch sieve: 0-5 percent passing.
- F. Topsoil: See Section 32 9119.

## **2.02 SOURCE QUALITY CONTROL**

- A. Where fill materials are specified by reference to a specific standard, test and analyze samples for compliance before delivery to site.
- B. If tests indicate materials do not meet specified requirements, change material and retest.
- C. Provide materials of each type from same source throughout the Work.

## **PART 3 EXECUTION**

### **3.01 EXAMINATION**

- A. Verify that survey bench marks and intended elevations for the Work are as indicated.
- B. Verify that existing topography is as shown in the plans. Coordinate with the engineer for any discrepancies prior to start of excavation.
- C. Identify required lines, levels, contours, and datum locations.
- D. See Section 31 2200 for additional requirements.
- E. Verify subdrainage, dampproofing, or waterproofing installation has been inspected.
- F. Verify structural ability of unsupported walls to support imposed loads by the fill.
- G. Verify areas to be filled are not compromised with surface or ground water.

### **3.02 PREPARATION**

- A. Scarify and proof roll subgrade surface to a depth of 6 inches to identify soft spots.
- B. Cut out soft areas of subgrade not capable of compaction in place. Backfill with general fill.

- C. Compact subgrade to density equal to or greater than requirements for subsequent fill material.
- D. Until ready to fill, maintain excavations and prevent loose soil from falling into excavation.
- E. Record locations of underground utilities.
- F. If required, remove concrete formwork.
- G. Remove trash and debris.

### 3.03 FILLING

- A. Fill to contours and elevations indicated using suitable materials.
- B. All select backfill, backfill and fill required for structures and trenches and required to provide the finished grades shown and as described herein shall be furnished, placed and compacted by the Contractor.
- C. Employ a placement method that does not disturb or damage other work.
- D. Systematically fill to allow maximum time for natural settlement. Do not fill over porous, wet, frozen or spongy subgrade surfaces.
- E. Maintain optimum moisture content of fill materials to attain required compaction density.
- F. Slope grade away from building minimum 2 percent, unless noted otherwise. Make gradual grade changes. Blend slope to transition at grade changes.
- G. Correct areas that are over-excavated.
- H. All material shall be placed in horizontal loose lifts not exceeding eight inches (8") in thickness and shall be mixed and spread in a manner assuring uniform lift thickness after placing. Each lift shall be compacted by not less than two complete coverages of the specified compactor. Select backfill shall be placed to the underside of all concrete slabs or paved areas. The fill material shall extend a minimum of five feet (5') outside the face of each structure and be twelve inches (12") below finished grade. The maximum slope of select backfill to the subgrade shall be one vertical to one and one half horizontal.
- I. Backfill around and outside of structures and over select backfill shall be deposited in layers not to exceed eight inches (8") in uncompacted thickness and mechanically compacted, using platform type tampers. Compaction of structural backfill, by rolling will be permitted provided the desired compaction is obtained and damage to the structure is prevented. Compaction of select backfill and/or backfill by inundation with water will not be permitted. All materials shall be deposited as specified herein and as shown on the drawings.
- J. Unless otherwise indicated in the Geotechnical Report, all material shall be placed at a moisture content that falls in the range of laboratory optimum moisture content and laboratory optimum +4%. It shall be compacted to a density of 95 percent (95%) of the maximum laboratory dry density for that material as determined by TxDOT TEX-113-E. The Contractor shall provide equipment capable of adding measured amounts of water to the material to bring it to a condition within the range of the required moisture content. The Contractor shall provide equipment capable of discing, aerating, and mixing the soil to insure reasonable uniformity of moisture content throughout the material and to reduce the moisture content of the material by air drying if necessary. If the subgrade material must be moisture conditioned before compaction, the material shall be sufficiently mixed or worked on the subgrade to insure a uniform moisture content throughout the lift of material to be

compacted. Materials at moisture content in excess of the specified limit shall be dried by aeration or stockpiled for drying.

- K. No material shall be placed when free water is standing on the surface of the area where the material is to be placed. No compaction of material will be permitted with free water on any portion of the material to be compacted. No material shall be placed or compacted in a frozen condition or on top of frozen material. Any material containing organic materials or other unacceptable material previously described shall be removed and replaced with acceptable material prior to compaction.
- L. Each lift of compacted material shall be compacted by the designated number of coverages of all portions of the surface of each lift by a smooth drum vibratory roller for granular material having a static weight not less than 5,500 pounds, a sheepsfoot roller for cohesive material exerting a pressure of 250 psi on the surface of the feet, or equivalent equipment, prior to commencement of the work. One coverage is defined as the condition obtained when all portions of the surface of the backfill material have been subjected to the direct contact of the compactor. The compactor shall be operated at a forward speed not exceeding 40 feet per minute.
- M. Compaction shall be performed with equipment suitable for the type of material being placed. The contractor shall select equipment which is capable of providing the minimum density required by these Specifications. The gross weight of compacting equipment shall not exceed 7,000 pounds within a distance of ten feet (10') from the wall of any existing structure or completed structure under this contract. Equipment shall be provided that is capable of compacting in restricted areas next to structures and around piping. The effectiveness of the equipment selected by the Contractor shall be tested at the commencement of compacted material work by construction of a small section of material within the area where material is to be placed. If tests on this section of backfill show that the specified compaction is not obtained, the Contractor shall increase the amount of coverages, decrease the lift thicknesses or obtain a different type of compactor.
- N. Particular care shall be taken to compact structure backfill which will be beneath pipes, roads, or other surface construction or structures. In addition, wherever a trench passes through structure backfill, the structure backfill shall be placed and compacted to an elevation twelve inches (12") above the top of the pipe before the trench is excavated. Compacted areas, in each case, shall be adequate to support the item to be constructed or placed thereon.
- O. The compaction requirements specified are predicated on the use of normal materials and compaction equipment. In order to establish criteria for the placement of a controlled fill so that it will have compressibility and strength characteristics compatible with the proposed structural loadings, a series of laboratory compaction and/or compressive strength tests will be performed on the samples of materials submitted by the Contractor. From the results of the laboratory tests, the final values of the required percent compaction, the allowable compaction moisture content range, and the maximum permissible lift thickness will be established for the fill material and construction equipment proposed.
- P. Compaction Density, unless otherwise specified or indicated:
  - 1. Standard: TxDOT TEX-113-E.
  - 2. Required Density: 95 percent of the maximum dry density.
  - 3. Lift Thickness: 8 inches.
  - 4. Moisture Content: Between optimum and optimum +4 percent.
  - 5. Testing laboratory will perform density tests at completion of each lift.



6. If the tests indicate unsatisfactory compaction, the Contractor shall provide the additional compaction necessary to obtain the specified degree of compaction. All additional compaction work shall be performed by the Contractor at no additional cost to the Owner until the specified compaction is obtained. This work shall include complete removal of unacceptable (as determined by the Testing Laboratory) fill areas and replacement and recompaction until acceptable fill is provided.
  7. Pit Run Sand Placement: Pit run sand shall be placed and compacted to the limits shown on the drawings.
  8. Drainage Gravel: Drain gravel shall be compacted in maximum 8-inch lifts with a minimum of two passes of a hand operated vibratory plate compactor weighing between 150 and 500 pounds.
- Q. Reshape and re-compact fills subjected to vehicular traffic.
- R. Maintain temporary means and methods, as required, to remove all water while fill is being placed as required, or until directed by the Engineer. Remove and replace soils deemed unsuitable by classification and which are excessively moist due to lack of dewatering or surface water control.

**3.04 FILL AT SPECIFIC LOCATIONS****3.05 TOLERANCES**

- A. Top Surface of General Filling: Plus or minus 1 inch from required elevations.

**3.06 FIELD QUALITY CONTROL**

- A. Refer to the Geotechnical Engineer for general requirements for field inspections and testing.

**3.07 CLEANING**

- A. See Section 01 7419 - Construction Waste Management and Disposal, for additional requirements.
- B. Leave unused materials in a neat, compact stockpile.
- C. Remove unused stockpiled materials, leave area in a clean and neat condition. Grade stockpile area to prevent standing surface water.
- D. Leave borrow areas in a clean and neat condition. Grade to prevent standing surface water.

**END OF SECTION**

**LIME SOIL STABILIZATION****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Excavating, treatment, and placement of lime treated subsoil mix.

**1.02 RELATED REQUIREMENTS**

- A. Section 31 2316 - Excavation.
- B. Section 31 2323 - Fill.
- C. Section 31 2316.13 - Trenching.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Measurement Method: Lime: By the ton (2000 lbs) of the type specified.
- B. Measurement Method: Lime Treatment: By the square yard of lime/subsoil mix. Includes supplying ingredient materials, scarifying substrate surface, mixing and placing where required, compacting and curing.

**1.04 REFERENCE STANDARDS**

- A. AASHTO M 216 - Standard Specification for Lime for Soil Stabilization; 2005.
- B. ASTM C977 - Standard Specification for Quicklime and Hydrated Lime for Soil Stabilization; 2010.
- C. ASTM D698 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft<sup>3</sup> (600 kN-m/m<sup>3</sup>)); 2012.
- D. ASTM D1556 - Standard Test Method for Density and Unit Weight of Soil in Place by the Sand-Cone Method; 2007.
- E. ASTM D1557 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft<sup>3</sup> (2,700 kN m/m<sup>3</sup>)); 2012.
- F. NLA Bull 326 - Lime-Treated Soil Construction Manual: Lime Stabilization & Lime Modification; 2004, 11th Edition.
- G. Project Geotechnical Report.

**1.05 SUBMITTALS**

- A. Submit mix design and materials mix ratio that will achieve specified requirements.
- B. Samples: Submit 10 lb sample of each type of fill in air-tight containers, to testing laboratory.

**1.06 QUALITY ASSURANCE**

- A. Perform Work in accordance with NLA Bull 326.
- B. Maintain one copy on site.

**1.07 FIELD CONDITIONS**

- A. Do not install mixed materials in wind in excess of 10 mph or when temperature is below 40 degrees F.

**PART 2 PRODUCTS****2.01 MIX MATERIALS**

- A. Subsoil: Existing reused.
- B. Type A, Hydrated Lime.
- C. Type B, Commercial Lime Slurry.
  - 1. Lime: TxDOT DMS-6350 "Lime and Lime Slurry."
  - 2. Mix Design: The Engineer will determine the target lime content and optimum moisture content in accordance with TxDOT Tex-121-E.
- D. Type C, Quicklime.
- E. The Contractor shall select, prior to construction, the grade to be used and shall notify the Engineer in writing before changing from one grade to another. Lime shall be placed in slurry form only, unless written permission is granted by the Engineer and a safety and containment plan is submitted to the Engineer by the Contractor seven days prior to use. In circumstances where it would be beneficial to utilize lime for "drying" subgrade materials to expedite construction, the Contractor may request approval from the Engineer to use pelletized lime.

**2.02 EQUIPMENT**

- A. Equipment: Capable of excavating subsoil, mixing and placing materials, wetting, consolidation, and compaction of material.

**2.03 LIME/SOIL MIX**

- A. Mix materials in accordance with Texas Department of Transportation Item No. 260, Lime Treatment-Road Mixed.
- B. Carefully add water to the mix to achieve a consistent mixture without lumping yet not create a wet plastic consistency.
- C. Obtain approval of the mix by Geotechnical Engineer before proceeding with placement.

**PART 3 EXECUTION****3.01 EXAMINATION**

- A. Do not place fill over frozen, wet or soft subgrade surfaces.

**3.02 EXCAVATION**

- A. Protect adjacent structures from damage by this work.
- B. Excavate subsoil to depth and configuration as indicated in the drawings. Minimum depth is 6 inches.
- C. Proof roll subgrade to identify soft areas; excavate those areas.
- D. Do not excavate within normal 45 degree bearing splay of any foundation.
- E. Notify Engineer of unexpected subsurface conditions. Discontinue affected Work in area until notified to resume work.
- F. Correct areas over-excavated in accordance with Section 31 2316.
- G. Stockpile excavated material in area designated on site; remove excess material not being reused from site.

- H. In lieu of using the cutting and pulverizing machine, the Contractor may excavate and windrow the material to expose the secondary grade to the typical sections, lines and grades as shown on the plans or as established by the Engineer.
- I. Pulverization: The existing pavement or base material shall be pulverized or scarified so that 100 percent shall pass the 2-1/2 inch sieve.

### 3.03 SOIL TREATMENT AND BACKFILLING

- A. Application: The percentage by weight or pounds per square yard of lime to be added will be as shown on the plans and may be varied by the Engineer if conditions warrant.
  - 1. Lime shall be spread only on that area where the mixing operations can be completed during the same working day.
  - 2. Unless otherwise approved by the Engineer, the lime operation shall not be started when the air temperature is below 40 degrees F. Lime shall not be placed when weather conditions in the opinion of the Engineer are unsuitable.
- B. Slurry Placing: When Type A Hydrated Lime is specified and slurry placement is to be used, the Type A Hydrated Lime shall be mixed with water to form a slurry with a solids content approved by the Engineer.
  - 1. Type B Commercial Lime Slurry:
    - a. Deliver to the project in slurry form at or above the minimum dry solids content approved by the Engineer.
    - b. The distribution of lime at the rate(s) shown on the plans or approved by the Engineer shall be attained by successive passes over a measured section of roadway until the proper lime content has been secured.
  - 2. Type C Quicklime:
    - a. Apply as slurry, the amount of dry quicklime shall be 80 percent of the amount shown on the plans.
    - b. The slurry shall contain at least the minimum dry solids content approved by the Engineer.
    - c. The residue from the slurring procedure shall be spread uniformly over the length of the roadway currently being processed unless otherwise approved by the Engineer.
  - 3. Slurry shall be of such consistency that it can be applied uniformly without difficulty.
  - 4. When the distributor truck is not equipped with an agitator, the Contractor shall have a standby pump available on the project for agitating the lime and water as required by the Engineer in case of undue delays in dispersing the slurry.
- C. Dry Placing: Dry placing is not allowed unless approved by the Engineer in writing.
  - 1. The lime shall be distributed by an approved spreader at the rate shown on the plans or as directed by the Engineer.
  - 2. The lime shall be distributed at a uniform rate and in such a manner as to reduce the scattering of lime by the wind.
  - 3. The material shall be sprinkled as approved by the Inspector.
- D. Mixing: The mixing procedure shall be the same for "Slurry Placing" or "Dry Placing" as herein described.
  - 1. Begin Mixing within 6 hours of lime application. During the interval between application and mixing, hydrated lime that has been exposed to the open air for a period of six (6)

hours or more or to excessive loss due to washing or blowing will not be accepted for payment.

- a. Initial Mixing: The material and lime shall be thoroughly mixed. The material and lime shall be brought to the proper moisture content and left to mellow for 1 to 4 days. When pebble grade quicklime is used, allow the mixture to mellow for 2 to 4 days as approved by the Engineer.
  - 1) If Type C Quicklime, Grade "DS," is approved for use by the Engineer under "Dry Placing," the material and lime shall be mixed as thoroughly as possible at the time of the lime application. Sufficient moisture shall be added during the mixing to hydrate the quicklime.
  - 2) During the mellowing period, the material shall be kept moist as directed by the Inspector.
  - 3) When shown on the plans or approved by the Engineer, the pulverization requirement may be waived when the material contains a substantial quantity of aggregate.
2. Final Mixing: After the required mellowing time, the material shall be uniformly mixed by approved methods. If the soil binder-lime mixture contains clods, they shall be reduced in size by the use of approved pulverization methods.
  - a. Following mixing, a sample of the material at roadway moisture will be obtained for pulverization testing. All non-slaking aggregates retained on the 3/4 inch sieve will be removed from the sample. The remainder of the material shall meet the following pulverization requirement when tested by TxDOT Test Method Tex-101-E, Part III:

1) Minimum passing 1-3/4 " sieve	100
2) Minimum passing 3/4" sieve	85
3) Minimum passing No. 4 sieve	60
- E. Compaction: Prior to compaction, the material shall be aerated or sprinkled as necessary to provide the optimum moisture. Compaction of the mixture shall begin immediately after final mixing and in no case more than 24 hours after final mixing
  1. Compaction shall continue until the entire depth of the mixture is uniformly compacted. Throughout this entire operation, the shape of the course shall be maintained by blading, and the surface upon completion shall be smooth and in conformity with the typical sections, lines and grades as shown on the plans or as established by the Engineer.
    - a. Ordinary Compaction: Roll with approved compaction equipment, as directed. Correct irregularities, depressions, and weak spots immediately by scarifying the areas affected, adding or removing treated material as required, reshaping, and recompact.
    - b. Density Control: Each course shall be sprinkled as required and compacted to the extent necessary to provide not less than 95 percent of the optimum density. Unless otherwise shown on the plans, the Engineer will determine roadway density of completed sections in accordance with TxDOT Test Method Tex-115-E. The Engineer may accept the section if no more than 1 of the 5 most recent density tests is below the specified density and the failing test is no more than 3 pcf below the specified density.

2. When the material fails to meet the density requirements, or should the material lose the required stability, density or finish before the next course is placed, or the project is accepted, it shall be reworked as specified below.
- F. Reworking a Section:
1. When a section is reworked within 72 hours after completion of compaction, the Contractor shall rework the section to provide the required compaction.
  2. When a section is reworked more than 72 hours after completion of compaction, the Contractor shall add 25 percent of the specified rate of lime.
  3. Reworking shall include loosening, road mixing as approved by the Engineer, compacting, and finishing.
  4. When a section is reworked, a new optimum density will be determined from the reworked material in accordance with TXDOT Test Method Tex-121-E, Part II and shall compact in-place to a minimum of 95 percent of this density.
  5. Do not surface patch.
- G. Finishing:
1. Site mix subsoil, backfill and compact. Blend treated subsoil mix to achieve mix formulation and required stabilization.
  2. Place mix material in continuous layers not exceeding 12 inches depth unless otherwise indicated in drawings. Maintain optimum moisture content of mix materials to attain required stabilization.
  3. Slope grade away from building minimum 2 inches in 10 ft, unless noted otherwise.
  4. Shape to required line, grade, and cross section.
  5. Make grade changes gradual. Blend slope into level areas.
  6. Immediately after completing compaction, clip, skin, or tight-blade the surface of the lime treated material with a maintainer or subgrade trimmer to a depth of approximately 1/4-inch.
  7. Remove loosened material and dispose of it at an approved location.
  8. Roll the clipped surface immediately with a pneumatic-tire roller until a smooth surface is attained.
  9. Add small increments of water as needed during rolling.
  10. Shape and maintain the course and surface in conformity with the typical sections, lines and grades shown on the plans or as directed.
  11. At end of day, terminate completed Work by forming a straight and vertical construction joint.
  12. Replace damaged fill with new mix to full depth of original mix.
  13. Remove surplus mix materials from site.

### 3.04 CURING

- A. After the final layer or course of the lime treated material has been compacted, it shall be brought to the required lines and grades in accordance with the typical sections.
- B. The completed section shall then be finished by rolling with a pneumatic tire or other suitable roller.
1. The completed section shall be moist cured or prevented from drying by addition of an asphalt material at the rate of 0.05 to 0.20 gallons per square yard.
  2. Curing shall continue for 2 to 5 days before further courses are added or traffic is permitted, unless otherwise approved by the Engineer.

- C. The lime treated material may be covered by other courses, the day following finishing, when approved by the Engineer. When the plans provide for the treated material to be covered by other courses of material, the next course shall be applied within 14 calendar days after final compaction is completed, unless otherwise approved by the Engineer.

**3.05 TOLERANCES**

- A. Top Surface of Fill: Plus or minus 0.1 inch from required elevations.

**3.06 FIELD QUALITY CONTROL**

- A. Compression test and analysis of hardened fill material will be performed in accordance with ASTM D 698.
- B. If tests indicate Work does not meet specified requirements, remove Work, replace and retest.
- C. Frequency of Tests: As determined by construction materials testing firm.

**END OF SECTION**

**AGGREGATE BASE COURSE****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Aggregate base course.
- B. Paving aggregates.

**1.02 RELATED REQUIREMENTS**

- A. Section 31 2200 - Grading.
- B. Section 31 2323 - Fill.
- C. Section 32 1250 - Site Pavement.
- D. Project Geotechnical Report.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Coarse Aggregate Type Per geotechnical report: By the cubic yard. Includes supplying aggregate material, stockpiling, scarifying substrate surface, placing, and compacting.
- B. Fine Aggregate Type Per geotechnical report: By the cubic yard. Includes supplying aggregate material, stockpiling, scarifying substrate surface, placing where required, and compacting.

**1.04 REFERENCE STANDARDS**

- A. AASHTO M 147 - Standard Specification for Materials for Aggregate and Soil-Aggregate Subbase, Base and Surface Courses; 1965 (2004).
- B. AASHTO T 180 - Standard Specification for Moisture-Density Relations of Soils Using a 4.54 kg (10-lb) Rammer and a 457 mm (18 in.) Drop; 2010.
- C. ASTM C136/C136M - Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates; 2014.
- D. ASTM D698 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft<sup>3</sup> (600 kN-m/m<sup>3</sup>)); 2012.
- E. ASTM D1556 - Standard Test Method for Density and Unit Weight of Soil in Place by the Sand-Cone Method; 2007.
- F. ASTM D1557 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft<sup>3</sup> (2,700 kN m/m<sup>3</sup>)); 2012.
- G. ASTM D2167 - Standard Test Method for Density and Unit Weight of Soil in Place by the Rubber Balloon Method; 2008.
- H. ASTM D2487 - Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System); 2011.
- I. ASTM D 2922 - Standard Test Methods for Density of Soil and Soil-Aggregate in Place by Nuclear Methods (Shallow Depth); 2005.
- J. ASTM D4318 - Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils; 2010.
- K. ASTM D6938 - Standard Test Method for In-Place Density and Water Content of Soil and Soil-Aggregate by Nuclear Methods (Shallow Depth); 2010.
- L. TxDOT TEX-113-E - Laboratory Compaction Characteristics and Moisture-Density Relationship of Base Materials.



**1.05 SUBMITTALS**

- A. Samples: 10 lb sample of each type of aggregate; submit in air-tight containers to testing laboratory.
- B. Materials Sources: Submit name of imported materials source.
- C. Aggregate Composition Test Reports: Results of laboratory tests on proposed and actual materials used.
- D. Compaction Density Test Reports.

**1.06 DELIVERY, STORAGE, AND HANDLING**

- A. When necessary, store materials on site in advance of need.
- B. Aggregate Storage, General:
  - 1. Separate differing materials with dividers or stockpile separately to prevent intermixing.
  - 2. Prevent contamination.
  - 3. Protect stockpiles from erosion and deterioration of materials.

**PART 2 PRODUCTS****2.01 MATERIALS**

- A. Limestone, coarse, crushed rock meeting the requirements of 2004 TxDOT Item 247, Type A, Grade 1 or 2.

**2.02 SOURCE QUALITY CONTROL**

- A. Where aggregate materials are specified using ASTM D2487 classification, test and analyze samples for compliance before delivery to site.
- B. If tests indicate materials do not meet specified requirements, change material and retest.
- C. Provide materials of each type from same source throughout the Work.

**PART 3 EXECUTION****3.01 EXAMINATION**

- A. Verify that survey bench marks and intended elevations for the work are as indicated.
- B. Verify substrate has been inspected, gradients and elevations are correct, and is dry.

**3.02 PREPARATION**

- A. Correct irregularities in substrate gradient and elevation by scarifying, reshaping, and re-compacting.
- B. Do not place aggregate on soft, muddy, or frozen surfaces.

**3.03 INSTALLATION**

- A. Spread aggregate over prepared substrate to a total compacted minimum thickness of 6 inches or as indicated on the drawings.
- B. Under Bituminous Concrete Paving:
  - 1. Place coarse aggregate to a total compacted minimum thickness of 6 inches or as indicated on the drawings.
  - 2. Compact to 95 percent of maximum dry density as determined by ASTM D 1557 at a moisture content ranging from -2 to +3 percent of the optimum moisture content unless otherwise indicated on the geotechnical report.
- C. Under Portland Cement Concrete Paving:

1. Place coarse aggregate to a total compacted minimum thickness of 6 inches or as indicated on the drawings.
  2. Compact to 95 percent of maximum dry density as determined by ASTM D 1557 at a moisture content ranging from -2 to +3 percent of the optimum moisture content unless otherwise indicated on the geotechnical report.
- D. Roller compact to specified density.
- E. Level and contour surfaces to elevations and gradients indicated.
- F. Add small quantities of fine aggregate to coarse aggregate as appropriate to assist compaction.
- G. Add water to assist compaction. If excess water is apparent, remove aggregate and aerate to reduce moisture content.
- H. Use mechanical tamping equipment in areas inaccessible to compaction equipment.

### 3.04 TOLERANCES

- A. Flatness: Maximum variation of 1/4 inch measured with 10 foot straight edge.
- B. Scheduled Compacted Thickness: Within 1/4 inch.
- C. Variation From Design Elevation: Within 1/4 inch.

### 3.05 FIELD QUALITY CONTROL

- A. Compaction density testing will be performed on compacted aggregate base course in accordance with ASTM D1556, ASTM D2167, or ASTM D6938.
- B. Results will be evaluated in relation to compaction curve determined by testing uncompacted material in accordance with AASHTO T 180, ASTM D698 ("standard Proctor"), or ASTM D1557 ("modified Proctor").
- C. If tests indicate work does not meet specified requirements, remove work, replace and retest.
- D. Frequency of Tests: To be determined by the Owner's Construction Materials Testing Contractors.
- E. Proof roll compacted aggregate at surfaces that will be under slabs-on-grade, pavers, and paving.

### 3.06 CLEANING

- A. Remove unused stockpiled materials, leave area in a clean and neat condition. Grade stockpile area to prevent standing surface water.
- B. Leave borrow areas in a clean and neat condition. Grade to prevent standing surface water.

**END OF SECTION**

**ASPHALT PAVING****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Aggregate base course.
- B. Single course bituminous concrete paving.
- C. Double course bituminous concrete paving.
- D. Surface sealer.

**1.02 RELATED REQUIREMENTS**

- A. Section 31 2200 - Grading: Preparation of site for paving and base.
- B. Section 31 2323 - Fill: Compacted subgrade for paving.
- C. Section 32 1123 - Aggregate Base Course:

**1.03 QUALITY ASSURANCE**

- A. Perform Work in accordance with State of Texas Highways standard.
- B. Mixing Plant: Complying with State of Texas Highways standard.
- C. Obtain materials from same source throughout.

**PART 2 PRODUCTS****2.01 MATERIALS**

- A. Aggregate for Base Course: In accordance with State of Texas Highways standards.

**2.02 ASPHALT PAVING MIXES AND MIX DESIGN**

- A. Asphalt Base Course: 3.0 to 6 percent of asphalt cement by weight in mixture in accordance with AI MS-2.

**PART 3 EXECUTION****3.01 AGGREGATE BASE COURSE**

- A. Place and compact aggregate base course.

**3.02 PLACING ASPHALT PAVEMENT - SINGLE COURSE**

- A. Install Work in accordance with State of Texas Highways standards.
- B. Place asphalt within 24 hours of applying primer or tack coat.
- C. Compact pavement by rolling to specified density. Do not displace or extrude pavement from position. Hand compact in areas inaccessible to rolling equipment.
- D. Perform rolling with consecutive passes to achieve even and smooth finish without roller marks.

**3.03 PLACING ASPHALT PAVEMENT - DOUBLE COURSE**

- A. Place asphalt binder course within 24 hours of applying primer or tack coat.
- B. Place asphalt wearing course within two hours of placing and compacting binder course.
- C. Compact pavement by rolling to specified density. Do not displace or extrude pavement from position. Hand compact in areas inaccessible to rolling equipment.
- D. Perform rolling with consecutive passes to achieve even and smooth finish, without roller marks.

**3.04 SEAL COAT**

- A. Apply seal coat to asphalt surface course and asphalt curbs in accordance with AI MS-19.

**END OF SECTION**

**SITE PAVEMENT****PART 1 GENERAL****1.01 DESCRIPTION**

- A. Scope
  - 1. Contractor shall furnish all labor, materials, equipment and incidentals required to provide hot mix-hot laid bituminous paving, as shown and specified for roadways.
  - 2. The work includes the following:
    - a. Lime Stabilized Subbase
    - b. Flexible Base
    - c. Cement Stabilized Base
    - d. Asphalt Stabilized Base
    - e. Surface Treatments
    - f. Hot Mix Asphaltic Concrete
    - g. Portland Cement Concrete Pavement
    - h. Testing As Specified

**1.02 RELATED REQUIREMENTS**

- A. Section 31 2200 - Grading.
- B. Section 31 2316 - Excavation.
- C. Section 31 2323 – Fill.
- D. Section 32 1123 – Aggregate Base Course.

**1.03 QUALITY ASSURANCE**

- A. Testing Services
  - 1. General: Testing of materials and of compaction requirements for compliance with technical requirements of the Specifications shall be the duty of a testing laboratory as provided for by the specifications for this Project.
  - 2. Testing Services:
    - a. The testing laboratory shall:
      - 1) Test the Contractor's proposed materials in the laboratory and field for compliance with the Specifications.
      - 2) Perform field density tests to assure that the specified compaction of surface and base course materials has been obtained.
      - 3) Report all test results to the Engineer and the Contractor.
      - 4) Perform concrete testing in accordance with the Project specifications.
  - 3. Authority and Duties of Testing Laboratory:
    - a. Technicians representing the testing laboratory shall inspect the materials in the field and perform compaction tests, and shall report their findings to the Engineer and the Contractor. When the materials furnished or work performed by the Contractor fails to fulfill Specifications requirements, the technician will direct the attention of the Engineer and the Contractor to such failure.
    - b. The technician shall not act as foreman or perform other duties for the Contractor. Work will be checked as it progresses, but failure to detect any defective work or materials shall not in any way prevent later rejection when such defect is discovered, nor shall it obligate the Engineer for final acceptance. Technicians are

not authorized to revoke, alter, relax, enlarge, or release any requirements of the Specifications, nor to approve or accept any portion of the Work.

4. Responsibilities and Duties of Contractor:
  - a. The use of testing services shall in no way relieve the Contractor of his responsibility to furnish materials and construction in full compliance with the Drawings and Specifications. To facilitate testing services, the Contractor shall:
    - 1) Secure and deliver to the Testing Laboratory, representative samples of the materials he proposes to use and which are required to be tested.
    - 2) Furnish such casual labor as is necessary to obtain and handle samples at the project or at other sources of material.
    - 3) Advise the testing laboratory and Engineer sufficiently in advance of operations (24 hrs. minimum) to allow for completion of quality tests and for the assignment of personnel.
- B. Referenced Standards
  1. Comply with the applicable provisions and recommendations of the following, unless otherwise shown or specified.
    - a. Texas Department of Transportation 2004, Standard Specification for Construction and Maintenance of Highways, Streets and Bridges.
    - b. Standard Specifications for Public Works Construction, City of San Antonio, Texas.
    - c. TxDOT TEX-113E - Laboratory Compaction Characteristics and Moisture-Density Relationship of Base Materials; 2010.

#### 1.04 SUBMITTALS

- A. Certificates: Submit certificates of compliance with these specifications for the following materials:
  1. Lime.
  2. Flexible Base.
  3. Asphalt Stabilized Base.
  4. Cement.
  5. Curing Seal Coat.
  6. Tack Coat.
  7. Hot Mix Asphaltic Concrete.
  8. Aggregates for Surface Treatments.
  9. Reinforcing Steel.
- B. Shop Drawings
  1. Submit the following:
    - a. Detailed reinforcing steel layout.
    - b. Detailed construction and control joint layout.
- C. Laboratory Test Reports
  1. Submit copies of laboratory test reports for optimum lime content for subgrade stabilization, and mix designs for Hot Mix Asphaltic Concrete and Portland Cement Concrete.
- D. All paving and base course materials shall be tested and approved prior to delivery to the site. Samples of materials proposed for use as pavement and base course should be submitted by the Contractor to the Testing Laboratory for testing. Samples of materials shall be submitted at least 14 days in advance of its use.

**PART 2 PRODUCTS****2.01 MATERIALS**

- A. The source of materials shall be acceptable to the Engineer. Materials shall conform to the following:
1. Flexible Base
    - a. Base material shall be crushed or uncrushed as necessary to meet the requirements hereinafter specified, and shall consist of durable stone or gravel, crushed and/or screened to the required particle size, with or without other approved fine sized materials. The material shall be from approved sources.
      - 1) Testing of flexible base material shall be in accordance with the following standard laboratory test procedures:
        - (a) Preparation of Soil
        - (b) Constants and Sieve Analysis: TxDOT TEX-110-E
        - (c) Liquid Limit: TxDOT TEX-104-E
        - (d) Plastic Limit: TxDOT TEX-105-E
        - (e) Plasticity Index: TxDOT TEX-106-E
        - (f) Linear Shrinkage: TxDOT TEX-107-E
        - (g) Sieve Analysis: TxDOT TEX-110-E
        - (h) Wet Ball Mill: TxDOT TEX-116-E
        - (i) Los Angeles Abrasion: ASTM C 131 (Grad. A)
      - 2) Samples for testing the material shall be taken prior to compaction operations.
      - 3) The material shall be well graded and when properly tested shall meet the following requirements of Texas Department of Transportation Specifications for Type A Grade 2 Flexible Base:
        - (a) Retained on 2½ inch sieve: 0%
        - (b) Retained on 1¾ inch sieve: 0-10%
        - (c) Retained on No. 4 sieve: 45-75%
        - (d) Retained on 40 mesh sieve: 60-85%
      - 4) The material passing the 40 mesh sieve shall be known as "Soil Binder" and shall meet the following requirements:
        - (a) The liquid limit shall not exceed 40.
        - (b) The plasticity index shall not exceed 12.
        - (c) The linear shrinkage shall not exceed 10.(Note: The linear shrinkage shall be calculated from the volumetric shrinkage at the liquid limit.)
      - 5) The crushed stone or crushed gravel shall have an abrasion of no more than 40 when subjected to the Los Angeles Abrasion Test.
    2. Asphalt Treated Base
      - a. This item shall consist of a compacted mixture of mineral aggregate and asphaltic material mixed hot in a mixing plant. The asphalt stabilized base shall be constructed on an approved subgrade, as herein specified and in accordance with details shown on the plans. Unless otherwise specified in this Section, all asphalt stabilized base materials shall conform to TxDOT, Item 292 "Asphalt Treatment (Plant Mixed)".
        - 1) Mineral Aggregate:

- (a) Description: The material shall be crushed or uncrushed and shall be screened as necessary to meet the requirements hereinafter specified and shall consist of durable aggregate particles. Unless otherwise specified on the plans, one or more mineral aggregates may be used to produce the specified mixture.
- (b) Grades: The grading of the mineral aggregate shall conform to the limitations as shown below:

## GRADE 1:

<u>Sieve</u>	<u>Percent Retained</u>
1-1/2 inches	0%
1 inch	0-10%
3/8 inch	30-55%
No. 4	45-70%
No. 40	70-85%

- (c) Tests:
- (1) Unless otherwise specified on the plans, the mineral aggregate for Grade 1 shall meet the following physical requirements:  
Los Angeles Abrasion: 50 Max.\*  
Plasticity Index: 10 Max.  
Liquid Limit: 40 Max.  
Decantation: 5% Max.  
Crushed Faces: 60% Max.  
Sand equivalent value shall not be less than 40.  
\*Wet Ball Mill may be used in lieu of Los Angeles Abrasion when shown on the plans. A maximum Wet Ball Mill value of 50 is allowed.
- (2) Testing of the mineral aggregates shall be as required and in accordance with the following Texas Department of Transportation Standard Test Methods:  
Preparation of Soil  
Constants and Sieve Analysis: TEX-110-E  
Liquid Limit: TEX-104-E  
Plastic Limit: TEX-105-E  
Plasticity Index: TEX-106-E  
Sieve Analysis: TEX-110-E  
Wet Ball Mill: TEX-116-E  
Los Angeles Abrasion: TEX-410-A  
Sand Equivalent: TEX-203-F  
Decantation: TEX-406-A  
Crushed Faces: TEX-460-A
- (3) Samples for testing the material shall be taken prior to the mixing operations. Where more than one material is used, tests will be on the combined material unless otherwise shown on the plans.
- (d) Additives:



- (1) Additives to facilitate mixing and/or improve the quality of the asphaltic mixture shall be used when noted on the plans or may be used upon written authorization by the Engineer.
- (e) Material Sources:
  - (1) All materials shall be obtained from sources reviewed by the Engineer.
  - (2) The mineral aggregate sources shall be opened up in such manner as to immediately expose the vertical faces of all the various strata of acceptable material and, unless otherwise directed by the Engineer, the material shall be secured in successive vertical cuts extending through all the exposed strata in order that a uniformly mixed material will be secured.
  - (3) Unless otherwise shown on the plans, one or more types of mineral aggregate or binder may be used to produce the specified mixture.
  - (4) Recycled Materials, including reclaimed asphalt concrete pavement (RAP), are allowed when shown on plans and are to be in accordance with Item 292 of the Texas Department of Transportation Standard Specifications. Stockpile aggregates for each source and type separately. Do not add material to an approved stockpile unless approved by the Engineer.
- 2) Asphaltic Material:
  - (a) Asphaltic material shall be of the type determined by the Engineer and shall meet the requirements of Item No. 300, "Asphalt, Oils and Emulsions" of the Texas Department of Transportation Standard Specifications. The grade of asphalt shall be PG 70-22 unless otherwise designated on the plans. When RAP is allowed by plan note, use no more than 30% RAP in TxDOT Type A or B HMA, and use no more than 20% RAP for TxDOT Type C or D unless otherwise shown on the plans.
- 3) Asphaltic Stabilized Mixture:
  - (a) Paving Mixture:
    - (1) The mixture shall consist of a uniform mixture of mineral aggregate and asphaltic material. The mineral aggregate will conform to the gradation requirements specified. The asphaltic material shall form from 4.0 to 9.0 percent of the mixture by weight unless otherwise shown on the plans. The design percent asphalt shall be determined in accordance with Test Method TEX-126-E or Test Method TEX-204F and procedures outlined in the TxDOT Bulletin C-14. The required method of control along with any required strength, laboratory density shall be specified on the plans. The percent asphalt in the mix shall be determined by either Extraction, Test Method TEX-210-F, or Pressure Pycnometer, Test Method TEX-126-E.
  - (b) Tolerances:
    - (1) The Engineer will designate the asphalt content to be used in the mixture after design tests have been made with the aggregate to be used in the project. When tested as determined by the Engineer, samples of the mixture shall not vary from the asphalt content

designated by the Engineer by more than 0.5 percent dry weight (based on total mixture).

3. Water
  - a. Water shall be free from substances deleterious to the hardening of the treated base and shall be reviewed by the Engineer.
4. Cement for Stabilized Base
  - a. Cement shall be Type 1 Portland Cement of a standard brand and shall conform to the requirements of ASTM Designation C-150.
    - 1) One bag, containing one (1) cubic foot of cement shall be considered as weighing 94 pounds net. One (1) barrel of cement shall be considered as weighing 376 pounds net, and containing four (4) cubic feet.
    - 2) Contractors, as their option, may use bulk cement, provided the apparatus for handling and spreading the cement is reviewed by the Engineer. Bulk cement shall be weighed on platform scales or standard plant batch weighing equipment reviewed by the Engineer.
    - 3) Cement delivered in bags shall be plainly marked with the brand name of the manufacturer. All bags shall be in good condition at the time of delivery. Cement salvaged from discharge or used bags shall not be permitted.
5. Asphalt Oils and Emulsions
  - a. RC-250 Liquid Asphalt and SS-1/SS-1H Emulsion used as a seal shall meet the requirements set forth in Item 300 "Asphalts, Oils and Emulsions," of the Texas Department of Transportation Standard Specifications.
  - b. Where Emulsified Asphalts are used, the amount of emulsified asphalt as a percentage by volume of the total mixture shall be within the limits shown on the plans, or shall be of a percentage as directed by the Engineer.
    - 1) Prime Coat:
      - (a) Unless the type and grade are shown on the plans, utilize an MC-30 or AE-P asphalt cement in accordance with Item 300, "Asphalts, Oils and Emulsions" of the Standard Specifications of the Texas Department of Transportation for prime coat. Emulsified asphalts as a percentage by volume of the total mixture shall be used within the limits shown on the plans or as directed/approved by the Engineer.
    - 2) Tack Coat:
      - (a) The asphaltic material used for Tack Coat shall meet the requirements for "Asphalt Cement", "Cut-Back Asphalt", or "Emulsified Asphalt", in Item No. 300 "Asphalts, Oils and Emulsions," of the Texas Department of Transportation Standard Specifications. The asphaltic material used for Tack Coat shall be that type or grade shown on the plans, or shall be as directed, or approved by the Engineer.
    - 3) Asphaltic Materials for Surface Treatments:
      - (a) The asphaltic material used for surface treatments shall meet the requirements for "Asphaltic Cement" in Item No. 300 "Asphalts, Oils and Emulsions" of the Texas Department of Transportation Standard Specifications. The asphaltic material used shall be AC-10 or equal, or shall be as approved by the Engineer.
6. Aggregates for Surface Treatments

- a. Aggregates shall meet all the requirements of Item No. 302, "Aggregates for Surface Treatments" of the Texas Department of Transportation Standard Specifications and subsequent revisions thereto.
  - b. The percent of wear, as determined by Test Method TEX-410-A for the material shall not exceed 15 percent. The aggregate shall be Grade 3, in accordance with the following gradation requirements when tested by Test Method TEX-200.
    - 1) Retained on 3/4" sieve: 0%
    - 2) Retained on 5/8" sieve: 0-2%
    - 3) Retained on 1/2" sieve: 5-20%
    - 4) Retained on 3/8" sieve: --
    - 5) Retained on No. 4 sieve: 90-100%
    - 6) Retained on No. 10 sieve: 98-100%
7. Hot Mix Asphaltic Concrete
- a. Materials used in Hot Mix Asphaltic Concrete Pavement shall meet the requirements as set forth in Item 340, "Dense-Graded Hot Mix Asphalt (Method)" or Item 341, "Dense-Graded Hot Mix Asphalt (QC/QA)" of the Texas Department of Transportation Standard Specifications.
  - b. Paving Mixture used shall be Type D. This mixture shall conform to the requirements of Paragraph 4 of Item 340, "Dense-Graded Hot Mix Asphalt (Method)" of the Texas Department of Transportation Standard Specifications.
    - 1) Part (1) of Paragraph 4 shall be revised as follows, and no other requirements are waived or changed hereby.
      - (a) Density
        - (1) Minimum: 94%
        - (2) Maximum: 99%
        - (3) Optimum: 97%
8. Lime for Stabilized Subgrade
- a. Lime for this item shall conform to the requirements of TxDOT Item No. 260, "Lime-Treatment - Road Mixed" of the TxDOT Standard Specifications (Latest Edition). Acceptable forms of lime shall be:
    - 1) "Type A, Hydrated Lime"
    - 2) "Type B, Commercial Lime Slurry"
    - 3) "Type C, Quicklime"
  - b. The Contractor shall select, prior to construction, the grade to be used and shall notify the Engineer in writing before changing from one grade to another. Lime shall be placed in slurry form only, unless written permission is granted by the Engineer and a safety and containment plan is submitted to the Engineer by the Contractor seven days prior to use. In circumstances where it would be beneficial to utilize lime for "drying" subgrade materials to expedite construction, the Contractor may request approval from the Engineer to use pelletized lime.
  - c. Materials are to be provided in conformance with the following items and requirements:
    - 1) Lime: TxDOT DMS-6350 "Lime and Lime Slurry"
    - 2) Mix Design: The Engineer will determine the target lime content and optimum moisture content in accordance with TxDOT TEX-121-E.
    - 3) When treating existing materials, limit the amount of asphalt concrete pavement to no more than 50% of the mix.

9. Forms
  - a. Formwork shall conform to the provisions of TxDOT Item 360 "Concrete Pavement". Forms shall be constructed of wood or steel, profiled to suit condition.
10. Concrete Pavement
  - a. Concrete and concrete materials shall conform to the provisions of the TxDOT Standard Specifications Item 360, "Concrete Pavement", unless otherwise stated in this specification.
    - 1) Coarse aggregate will conform to the gradation requirements of Aggregate Grade No. 2.
    - 2) Fine aggregates will conform to the gradation requirements of Aggregate Grade No. 1.
    - 3) Portland Cement shall be Type I.
  - b. Air content shall be  $4\% \pm 1\%$ .
  - c. Slump shall range from 2 to 5 inches.
  - d. Provide concrete design per Geotechnical Report. If not specified, provide Class P concrete designed to meet a minimum average compressive strength of 4,000 psi at 28-days (75% strength at 7 days). Test in accordance with TxDOT TEX-448-A or TxDOT TEX-418-A.
  - e. For concrete curbs that are placed separately from the pavement, refer to specification Section 32 1313.10 (use Class A or P concrete for curbs that are placed separately from the pavement).
11. Steel Expansion Joint Dowels
  - a. Dowel bars shall be round smooth steel conforming to ASTM A 36, Grade 60. Coat dowels with an approved de-bonding material.
12. Reinforcing Steel
  - a. Reinforcing steel bars shall conform to TxDOT Item 440, "Reinforcing Steel", ASTM A 615 Grade 60, deformed bars. All reinforcing steel shall be new billet steel.

## **2.02 CUTTING AND REPLACING PAVEMENTS**

- A. All materials used for cutting and replacing pavements shall conform to the requirements of this section.

## **PART 3 EXECUTION**

### **3.01 GENERAL**

- A. The drives and parking areas shall be constructed to the lines, and typical section shown on the drawings.

### **3.02 SUBGRADE PREPARATION**

- A. Preparation of the subgrade including compaction shall be completed for the full width of the roadways and parking areas, or as shown on plans.
  1. The subgrade shall be compacted to at least 95 percent (95%) of maximum density between optimum moisture content and optimum moisture content +4% as determined by TxDOT TEX-114-E.
- B. No materials shall be placed on subgrades which are muddy or have water thereon.

### **3.03 CONSTRUCTION OF HOT MIX ASPHALTIC CONCRETE ROADWAYS, AND PARKING AREAS**

- A. General

1. The roadways, and parking areas shall be constructed to the lines, grades, and typical section shown on the Drawings.
  2. Conform to all applicable requirements of the Texas Department of Transportation Standard Specifications.
- B. Flexible Base Course
1. Equipment:
    - a. All equipment necessary to properly perform and complete the work shall be on the project prior to beginning the work, shall be subject to the review of the Engineer, and shall be maintained in a satisfactory condition at all times.
      - 1) Motor graders shall be self-propelled, shall have tandem or four-wheel drive, shall have a blade length of not less than 12 feet, shall have a wheel base length (the distance between front and rear axles) of not less than 16 feet, and shall be tight and in good operating condition and reviewed by the Engineer.
      - 2) Compaction equipment shall be of sufficient weight and adequately loaded to accomplish the required compaction.
      - 3) Water distributors shall be equipped with positive and rapidly working cut-off valves, approved spray bars equipped with bituminous nozzles and a power pump that will insure distribution of water in a uniform and controllable rate of application. Spray bars shall be so constructed that the effective length may be quickly and easily altered.
      - 4) All equipment shall meet these specifications and be reviewed by the Engineer. Equipment may be eliminated or substituted only upon review of the Engineer.
      - 5) Nothing in this section shall relieve the Contractor of his responsibility for producing finished work of the quality specified.
  2. Construction Methods:
    - a. General:
      - 1) It is the intent of this specification to obtain a complete course, or courses, of Flexible Base of uniform moisture and density, with a closely-knit surface free from laminations, cracks, ridges, or loose material, and to the surface requirements hereinafter specified.
    - b. Placing of Flexible Base Material:
      - 1) The Flexible Base material shall be placed on the approved subgrade in courses not to exceed six inches (6") compacted depth. It shall be the responsibility of the Contractor that the required amount of material be delivered and uniformly spread and shaped. All material shall be moved from the place where it is dumped by cutting into windrows. After the material has been cut into windrows, it shall be sprinkled, spread, shaped, and rolled in proper sequence to prevent segregation, and as necessary for required compaction.
    - c. Compaction and Finishing:
      - 1) Flexible Base shall be compacted to an apparent dry density of not less than 95 percent of the maximum dry density, as determined in accordance with TxDOT Test Method TEX-113-E. Tests for density will be made within 24 hours after compaction operations are completed. If the material fails to meet the density specified, it shall be reworked as necessary to meet the density required. Just prior to the placing of any succeeding course of Flexible Base, surfacing on any previously completed course, the density and moisture of the top three inches

(3") of Flexible Base shall be checked and if tests show the density to be more than 2 percent below the specified minimum, or the moisture content to be more than 3 percent above or below the optimum, the course shall be reworked as necessary to obtain the specified compaction and moisture content.

- 2) The surface upon completion shall be smooth and in conformity with the typical sections and to the established lines and grades. Any deviation in excess of 1/4 inch in cross-section and in length of 16 feet measured longitudinally, shall be corrected. All irregularities, depressions, or weak spots which develop shall be corrected.

C. Asphalt Stabilized Base Course

1. Equipment:

- a. All equipment for the handling of all materials and mixing and placing of the mixture shall be maintained in good repair and operating condition and subject to review by the Engineer. Any equipment found to be defective and affecting the quality of the mixture will be replaced.
- b. Mixing Plants:
  - 1) Mixing plants that will not consistently produce a paving mixture meeting all the requirements of this specification will be condemned. Mixing plants may be of the weight-batching type, the continuous mixing type or the dryer-drum type meeting all the requirements of Item 345, "Asphalt Stabilized Base (Plant Mix)" of the Texas Department of Transportation Standard Specifications and subsequent revisions and Special Provisions thereto.
- c. Asphalt Material Heating Equipment:
  - 1) Asphalt material heating equipment shall conform to Paragraph 345.4 of Item 345, "Asphalt Stabilized Base (Plant Mix)" of the Texas Department of Transportation Standard Specifications and subsequent revisions and Special Provisions thereto.
- d. Spreading and Finishing Machine:
  - 1) The spreading and finishing machine shall be of a type reviewed by the Engineer and shall be capable of producing a surface that will be smooth and true to the established line, grade and cross-section and acceptable to the Engineer. Unacceptable finish shall be corrected by the addition of mixture placed and finished at the entire expense of the Contractor.

2. Construction Methods:

- a. It shall be the responsibility of the Contractor to produce transport, place and compact the specified mixture in accordance with these specifications and as reviewed by the Engineer.
- b. The asphaltic mixture, when placed with a spreading and finishing machine shall not be placed when the air temperature is below 50°F, and is falling, but it may be placed when the air temperature is above 40°F and is rising. The mixture when placed with a motor grader shall not be placed when the air temperature is below 60°F, and is falling, but may be placed when the air temperature is above 50°F and is rising. The air temperature shall be taken in the shade away from artificial heat. It is further provided that the prime coat, tack coat or asphalt stabilized base shall be placed only when the humidity, general weather conditions and temperature and

moisture conditions of the subbase or subgrade, in the opinion of the Engineer are suitable.

- c. If, after being discharged from the mixer and prior to placing, the temperature of the asphaltic mixture is 50°F or more below the temperature reviewed by the Engineer all or any part of the load may be rejected and payment will not be made for the rejected material.
  - 1) Prime Coat:
    - (a) If a prime coat is required it shall be applied and paid for as a separate item conforming to the requirements of Texas Department of Transportation Item 300 "Prime Coat", except the air temperature for application shall be as provided above for asphaltic mixture to be laid by a spreading and finishing machine. The tack coat or asphalt stabilized base shall not be applied on a previously primed course until the prime coat has completely cured to the satisfaction of the Engineer.
  - 2) Tack Coat:
    - (a) Before the asphaltic mixture is laid, the surface upon which the tack coat is to be placed shall be cleaned thoroughly to the satisfaction of the Engineer. The surface shall be given a uniform application of tack coat using asphaltic materials of this specification. This tack coat shall be applied, as directed by the Engineer, with an approved sprayer at a rate not to exceed 0.10 gallon per square yard of surface. Where the mixture will adhere to the surface on which it is to be placed without the use of a tack coat, the tack coat may be eliminated by the Engineer. All contact surfaces of curbs and structures and all joints shall be painted with a thin uniform coat of the asphaltic material meeting the requirements for a tack coat. The tack coat shall be rolled with a pneumatic tire roller when directed by the Engineer.
  - 3) Transporting:
    - (a) The asphaltic mixture, prepared as specified above shall be hauled to the site in tight vehicles previously cleaned of all foreign material. The dispatching of the vehicles shall be arranged so that all material delivered may be placed, and all rolling shall be completed during daylight hours. The inside of the truck body may be given a light coat of oil, lime slurry or other material satisfactory to the Engineer, if necessary, to prevent mixture from adhering to the body. In cool weather or for long hauls, canvas covers and insulating of truck bodies may be required.
  - 4) Placing:
    - (a) Generally, the asphaltic mixture shall be dumped and spread on the approved prepared surface with the specified spreading and finishing machine, in such a manner that when properly compacted, the finished course will be smooth, of uniform density, and will conform with the typical sections shown on the plans and to the lines and grades established by the Engineer. During the application of asphaltic material, care shall be taken to prevent splattering of adjacent pavement, curb and gutter and structures. Any unsightly splattering of surroundings shall be clean and restored to original condition at the sole cost of the Contractor.

- (b) The mixture shall be spread and compacted in layers or lifts as specified on the plans or as directed by the Engineer. The sequence of compacting shall be such that undue displacement of the edge of the course does not occur. On deep lifts, the edge of the course may be rolled with a motor grader wheel or similar equipment or supported by blading a roll of earth against the edge of the course prior to compacting the surface.
  - (c) When the asphaltic mixture is placed in a narrow strip along the edge of an existing pavement, or used to level up small areas of an existing pavement or placed in small irregular areas when the use of a finishing machine is not practical, the finishing machine may be eliminated when authorized by the Engineer, provided a satisfactory surface can be obtained by other acceptable methods.
- 5) Compacting:
  - (a) As directed by the Engineer the asphalt stabilized base shall be compacted thoroughly and uniformly with the specified rollers. In lieu of the rolling equipment specified, the Contractor may, upon written authorization from the Engineer, operate other compacting equipment that will provide equivalent relative compaction as the specified equipment. If the substituted compaction equipment fails to produce the desired compaction as would be expected of the specified equipment, as determined by the Engineer, its use shall be discontinued. When directed by the Engineer, the initial compaction shall be accomplished with the pneumatic tire roller.
  - (b) When rolling with the three-wheel, tandem, or vibratory rollers, rolling shall start longitudinally at the sides and proceed toward the center of the pavement, overlapping on successive trips by at least half the width of the rear wheel unless otherwise directed by the Engineer. Alternate trips of the roller shall be slightly different in length. When roller with vibratory steel-wheel rollers, the manufacturer's recommendation shall be followed unless directed otherwise by the Engineer. Rolling with pneumatic-tire roller shall be done as directed by the Engineer. Roller shall be continued until no further increase in density can be obtained and all roller marks are eliminated. The motion of the roller shall be slow enough at all times to avoid displacement of the mixture. If any displacement occurs, it shall be corrected at once by the use of a rake, and of fresh mixtures where required. The roller shall not be allowed to stand on any portion of the mixture pavement which has not been fully compacted. To prevent adhesion of the mixture to the roller, the wheels shall be kept thoroughly moistened with water, but an excess of water will not be permitted. All rollers must be in good mechanical condition. Necessary precautions shall be taken to prevent the dropping of gasoline, oil, grease or other foreign matter on the roadway, either when the rollers are in operation or when standing.
  - (c) Hand Tamping:
    - (1) The edges of the pavement along curbs, headers and similar structures, and all places not accessible to the roller, or in such



positions as will not allow thorough compaction with the rollers, shall be thoroughly compacted with lightly oiled tamps.

(d) Surface Finish:

- (1) The compacted material shall conform to the typical cross sections, lines and grades as shown on plans and directed by the Engineer and shall have a surface smoothness as specified below and with a reasonably uniform texture acceptable to the Engineer. Unacceptable finished surfaces may be corrected by the placement of additional mixture, all at the expense of the Contractor.

(e) Surface Smoothness:

- (1) Test finished surface of each bituminous concrete course for smoothness, using a 10-foot straightedge applied parallel to and at right angles to centerline of paved areas.
- (2) Check surfaced areas at intervals as directed by Engineer.
- (3) Surfaces will not be acceptable if exceeding the following:  
Sub-base Courses: 3/8 inch in 10 feet  
Surface Course: 1/4 inch in 10 feet  
Crowned Surfaces:  
Test crowned surfaces with a crown template, centered and at right angle to the crown.  
Surfaces will not be acceptable if varying more than 1/4" from the template.  
Protection of the Work and Opening to Traffic:
- (4) The completed asphalt stabilized base course shall be opened to traffic as provided by the plans and as directed by the Engineer. All construction traffic allowed on the base course shall comply with the State laws governing traffic, unless otherwise authorized by the Engineer. When another roadway surface is provided for the traveling public and construction traffic through the project, the Engineer may prohibit traffic on the completed base course.

3. In Place Density:

- a. When in-place density is required, it is the intent of this specification that the material be placed and compacted to 96 percent of the maximum molded gyrated density as determined by Test Method TEX-126-E or as specified on the plans. The maximum molded gyrated density shall be determined from material sampled from the mixing plant and molded in accordance with Test Method TEX-126-E. Procedures and methods outlined in Test Method TEX-126-E shall also be used in determining the in-place density unless determined otherwise by the Engineer. The field specimens utilized for the in-place density testing may be either cores or sections of asphalt stabilized base tested according to Test Method TEX-207-F. Other methods of determining in-place density which correlate satisfactorily with those results obtained through use of Test Method TEX-126-E may be used. In-place density tests are intended for control tests. If the in-place density of the mixture produced has a value lower than that specified and, in the opinion of the Engineer is not due to a change in the quality of the material, production may proceed with subsequent changes in the mix and/or construction operations until the in-place density equals or exceeds the specified density. Requirements

specifying air temperature limitations for placing and types of rollers to be furnished are not applicable when in-place density is specified. Regardless of the method of compaction control followed, all rolling shall be completed before the mixture temperature drops below 175°F.

D. Cement Stabilized Base Course

1. Equipment:

- a. All equipment necessary to properly perform and complete the work shall be on the project prior to beginning the work, shall be subject to the review of the Engineer, and shall be maintained in a satisfactory condition at all times.
- b. The following list of equipment shall be considered the minimum necessary for cement stabilized base work:
  - 1) A single pass traveling mixing plant may be used if it can be made to meet the exact requirements of this Specification.
  - 2) In lieu of a traveling mixing plant, the following equipment shall be used for the "Mixed-in-Place" method of processing:
    - (a) 1-Motor Grader
    - (b) 1-7-foot self-powered, self-propelled, heavy-duty rotary speed mixer.
- c. Motor graders shall be self-propelled, shall have tandem or four-wheel drive, shall have a blade length of not less than 12 feet, shall have a wheel base length (the distance between front and rear axles) of not less than 16 feet, and shall be tight and in good operation condition and reviewed by the Engineer.
- d. Cement shall be distributed by cement spreaders equipped with hoppers of adequate capacity to prevent spillage. The proportioning and distributing devices shall be positive in action and capable of necessary adjustments in quantity of cement spread and width of lane spread. The spreader shall be so designed that its accuracy is not varied by changing conditions of the surface over which it operates. The cement spreader shall distribute cement to an accuracy of five percent (5%) of theoretical quantity per square yard and shall be approved by the Engineer.
- e. Compaction equipment shall be of sufficient weight and adequately loaded to accomplish the required compaction.
- f. Water distributors shall be equipped with positive and rapidly working cut off valves, approved spray bars equipped with bituminous nozzles and a power pump that will insure distribution of water in a uniform and controllable rate of application. Spray bars shall be so constructed that the effective length may be quickly and easily altered.
- g. All equipment shall meet these specifications and be reviewed by the Engineer. Equipment may be eliminated or substituted only upon review by the Engineer.
- h. Nothing in this section shall relieve the Contractor of his responsibility for producing finished work of the quality specified.

2. Test Section:

- a. If the Contractor has had no previous experience in construction of "Cement Stabilized Base", he shall be required to construct a "Test Section" in accordance with the following:
- b. The first section of each cement treated course shall serve as a test section. Its length (not less than 350 linear feet or more than 500 linear feet) shall be determined by the capability of the equipment to perform the work. In case it is found that the work is not satisfactory with respect to the specification

requirements, the Contractor shall revise his procedures and augment or replace equipment as necessary to assure work completed in accordance with the Specifications. Additional test sections may be required as directed by the Engineer. Test sections not conforming to the requirements of the Specifications shall be reconstructed.

3. Construction Methods:

a. General:

- 1) It is the intent of this specification to obtain a complete course or courses of cement stabilized base of uniform moisture and density, containing a uniform mixture of cement; a closely knit surface free from laminations, cracks, ridges, or loose material and to the surface requirements hereinafter specified. It shall be the responsibility of the Contractor to furnish adequate equipment and regulate his sequence of operation in such a manner as to provide a cement treated course or courses with the proper amount of cement for the depth as shown on the plans and to maintain or reconstruct the course or courses as necessary to conform to the specific requirements specified.

b. Placing of Base Material:

- 1) After approval of the subgrade, base material shall be delivered on the road and placed in windrows of uniform sections, then accurately bladed and shaped to required crown and grade to provide a base of compacted depth required by the plans.

c. Final Preparation of Section:

- 1) On the day immediately preceding processing, water, as required, shall be added and uniformly mixed full depth with the base material. This operation shall precede cement spreading by at least 12 hours. The section shall then be accurately bladed and shaped to required grade and section.

d. Application of Cement:

- 1) The specified quantity of Portland Cement required for the full depth of treatment shall be uniformly spread over the surface. Each pass of the cement spreader shall be positioned by either the curb line or a string line. Cement shall be applied only to such areas as can be completed as herein specified within the daylight hours of the same day. No equipment, except that used in spreading and mixing, will be allowed to pass over the freshly spread cement until it is mixed with the base material.

e. Mixing and Processing:

- 1) Either method (1) or (2) below may be used at the option of the Contractor. Method (3) shall be used only on sections less than 200 linear feet in length.

(a) Multiple-Pass Traveling Mixing Plant:

- (1) After the cement has been applied, it shall be mixed with the base or subbase material. Mixing shall continue until the cement has been sufficiently blended with the base or subbase material to prevent the formation of cement balls when water is applied. Any mixture that has not been compacted and finished shall not remain undisturbed for more than 30 minutes.
- (2) Immediately after the mixing of base or subbase material and cement is completed, water, as necessary, shall be uniformly applied and incorporated into mixture. Proper care shall be exercised to insure

proper moisture distribution at all times. After the last increment of water has been added, mixing shall continue until a thorough and uniform mix has been obtained.

(b) Single-Pass Traveling Mixing Plant:

- (1) After the cement has been applied, it shall be sufficiently mixed with the base or subbase material to prevent the formation of cement balls when water is applied. Unpulverized soil lumps in mixture will not be allowed. Should this condition prevail, the Contractor shall "pre-wet" the raw base or subbase material as necessary to correct this condition. The mixer shall be provided with means for visibly and accurately gauging the water application. The water shall be applied uniformly through a pressure spray bar. After cement is spread, mixing operations shall proceed as follows:
- (2) The mixer shall, in one continuous operation, mix the base or subbase material and cement full depth, add the required moisture uniformly, thoroughly moist-mix the material, cement, and water, spread the completed mixture evenly over the machine processed width of the subgrade, and leave it in a loose condition ready for immediate compaction.
- (3) The mixture shall not remain undisturbed, after mixing and before compacting, for more than 30 minutes.

(c) Blade Mixing:

- (1) On sections of street of 200 linear feet or less and authorization by the Engineer, the requirements for mixing equipment may be waived and the cement mixed with a mortar grader.
- (2) Immediately after the cement has been distributed, the material shall be scarified full depth and the cement mixed with the loose base material for the full depth of the treatment by blading into windrows. Mixing shall continue until the cement has been sufficiently blended with the base material to prevent formation of cement balls when water is applied.
- (3) Immediately after the mixing of base material and cement is complete, water as necessary shall be uniformly applied and incorporated into the mixture. Pressurized equipment and supply provided shall be adequate to insure continuous application of the required amount of water to the section being processed. Proper care shall be exercised to insure proper moisture distribution at all times. After the last increment of water has been added, mixing shall continue until thorough and uniform mix has been obtained.

f. Compaction and Finishing:

- 1) The material shall be compacted to not less than 95 percent of the maximum dry density as determined by TxDOT TEX-113-E. At the start of compaction, the percentage of moisture in the mixture shall be less than that quantity which will cause the mixture to become unstable during compaction and finishing.
- 2) The surface upon completion shall be smooth and in conformity with typical sections and to the established lines and grades. Any deviation in excess of ¼

inch in cross section and in a length of 16 feet measured longitudinally shall be corrected. All irregularities, depressions, or weak spots which develop shall be corrected.

- 3) All sections of cement stabilized base shall be processed full width each day without longitudinal construction joints.
  - 4) The density of the cement stabilized base shall be determined by the Engineer after construction. Any portion which has a density below that specified herein and which has not properly hardened after a suitable time interval shall be removed and replaced to meet this Specification at the expense of the Contractor.
- g. Protection and Cover:
- 1) The completed cement treated base course shall be protected against rapid drying by applying a minimum of 0.20 gallons per square yard of RC-2 Liquid Asphalt, or a minimum of 0.15 gallons per square yard of EA-11M Emulsion. The actual amounts may be varied in the field by the Engineer to insure that a complete and adequate seal is achieved.
  - 2) This curing seal shall be applied as soon as practicable, but not later than eight (8) hours after the completion of final compaction. The surface shall be kept moist until the curing seal is applied. It shall be the responsibility of the Contractor to protect the asphalt membrane from being picked up by traffic by either sanding or dusting the surface.
  - 3) The curing period shall be a minimum of 14 days (24 hours each) with a minimum temperature of 40°F unless waived by the Engineer.
- h. Weather Limitations:
- 1) Cement stabilized base construction shall not begin unless the temperature is at least 40°F in the shade and rising or when the wind velocity exceeds 15 MPH. The Contractor is responsible for the quality of the base under any weather conditions.
- i. Traffic:
- 1) The Contractor shall not be permitted to drive heavy equipment over completed portions, but pneumatic-tired equipment required for hauling cement and water may be permitted after the surface has hardened sufficiently to prevent the equipment from marring the surface, provided protection and cover specified herein are not impaired. The cement stabilized base may be opened to local traffic as soon as the RC-2 has been applied and dusted or sanded as necessary to prevent it from being picked up by traffic. It may be opened to all traffic after 7 days.
- j. Maintenance:
- 1) The Contractor shall be required to maintain at his own expense the entire cement stabilized base within the limits of his contract in good condition satisfactory to the Engineer from the time he first starts work until all work shall have been completed.
  - 2) Maintenance shall include immediate repairs of any defect that may occur after construction, which work shall be done by the Contractor at his own expense and repeated as often as necessary to keep the area continuously intact. Repairs are to be made in a manner to insure restoration of a uniform surface of good quality cement stabilized base. Faulty work shall be replaced

for the full depth of base. Any low area shall be remedied by replacing the material for the full depth of treatment, rather than adding a thin layer of base material to the completed work.

E. Prime Coat

1. When the area and/or base is satisfactory to receive the prime coat, the surface shall be cleaned by sweeping or other acceptable methods. If necessary, the surface shall be lightly sprinkled with water just prior to application of the asphaltic material. The asphaltic material shall be applied on the clean surface by an acceptable type of self-propelled pressure distributor so operated as to distribute the prime coat at a rate not to exceed 0.20 gallon per square yard of surface, evenly and smoothly, under a pressure necessary for proper distribution. During the application of prime coat, care shall be taken to prevent splattering of adjacent pavement, curb and gutters or structures.
2. Prime Coat shall not be applied when the air temperature is below 60° F and falling, but it may be applied when the air temperature is above 50° F and is rising; the air temperature being taken in the shade away from artificial heat.
3. Asphaltic material shall not be placed when general weather conditions, in the opinion of the Engineer, are not suitable.

F. Tack Coat

1. Before the tack coat is applied, the surface shall be cleaned thoroughly. The asphaltic material shall be applied on the clean surface by an acceptable type of self-propelled pressure distributor so operated as to distribute the tack coat at a rate not to exceed 0.10 gallon per square yard of surface, evenly and smoothly under a pressure necessary for proper distribution. Where the pavement mixture will adhere to the surface on which it is to be placed without the use of a tack coat, the tack coat may be eliminated by the Engineer. All contact surfaces of curbs and structures and all joints shall be painted with a thin uniform coat of the asphaltic material used for tack coat. The tack coat shall be rolled with a pneumatic tire roller. During the application of tack coat, care shall be taken to prevent splattering of adjacent pavement, curb and gutters or structures.

G. Hot Mix Asphaltic Concrete

1. Construction methods used in laying Hot Mix Asphaltic Concrete Pavement shall meet the requirements as set forth in Item 340 "Hot Mix Asphaltic Concrete Pavement" of the Texas Department of Transportation Standard Specifications, with the following exception:
2. Application of Hot Mix Asphaltic Concrete Pavement shall not begin unless the air temperature is at least sixty degrees Fahrenheit (60° F) and rising.

H. Surface Treatments

1. The area to be treated shall be cleaned of dirt, dust, or other deleterious matter by sweeping or other approved methods. If it is found necessary by the Engineer the surface shall be lightly sprinkled just prior to the first application of asphaltic material.
2. Asphaltic material shall be applied on the clean surface by an acceptable type of self-propelled distributor so operated as to distribute the material at the rate as shown on the plans, evenly and smoothly, under pressure necessary for proper distribution. The Contractor shall provide all necessary facilities for determining the temperature of the asphaltic material in all of the heating equipment and in the distributor, for

determining the rate of which it is applied, and for securing uniformity at the junction of two distributor loads. The distributor shall have been recently calibrated. Asphaltic materials shall not be applied until immediate covering is assured.

3. Aggregate shall be immediately and uniformly applied and spread by an acceptable self-propelled continuous feed aggregate spreader, unless otherwise authorized by the Engineer. The aggregate shall be applied at a rate as directed by the Engineer.
  4. The entire surface shall then be broomed or raked as required and shall be thoroughly rolled as soon as practicable after its application. The cover material shall be rolled for its entire width with a multiple wheel self-propelled pneumatic tired traffic roller with provisions for loading to 8 tons. Rolling shall begin longitudinally at the edges of the mat and progress toward the center, uniformly lapping each preceding track by at least 1/2 the width of the roller and be repeated as often as necessary to thoroughly key the cover material into the bitumen over the entire surface. The roller shall be in first class operating condition.
  5. Surface treatment or treatments shall not be applied when the air temperature is below 60°F and is falling, but is may be applied when the air temperature is above 50°F and is rising. Air temperature shall be taken in the shade and away from artificial heat. Asphaltic material shall not be placed when general weather conditions, in the opinion of the Engineer, are not suitable.
  6. The Contractor shall be responsible for the maintenance of the surface until the work is acceptable to the Engineer.
- I. Construction Joints
1. Construction joints shall be made in such a manner as to ensure a neat junction, thorough compaction, and bond throughout.
  2. A transverse joint extending over the full width of the strip being laid and at right angles to its centerline shall be constructed at the end of each day's work and at any other times when the operations of placing the hot mixture are suspended for a period of time which will permit the mixture to chill. The forward end of a freshly laid strip shall be thoroughly compacted by rolling before the mixture has become chilled. When work is resumed, the end shall be cut vertically for the full depth of the layer.
  3. When new pavement is to join pavement installed by others or previously laid pavement by the Contractor, the in-place pavement shall be neatly and carefully edged to allow for overlapping and feathering of the new surface course material. A tack coat of bituminous prime coat material shall be placed at the interface of new and previously laid material.
- J. Traffic Maintenance
1. The pavement shall be opened to traffic when directed by the Engineer. Construction traffic on the pavement shall be held to a minimum and shall be acceptable to the Engineer. Adequate protection methods as reviewed by the Engineer shall be utilized when crossing roadways is required.
- K. Field Quality Control
1. Contractor will employ a testing laboratory to perform field quality control. The testing laboratory will make compaction testing of flexible base. The testing laboratory will test the HMAC for Asphalt Extraction, Gradation, Bitumen Content, Stability and laboratory density. Contractor shall furnish all necessary assistance required by the testing laboratory. Contractor shall also furnish all labor, materials and equipment necessary

for sampling. Contractor shall furnish all necessary transportation to the Owner's Testing Laboratory required by the testing.

2. Quality Control Testing During Construction:
  - a. The testing laboratory will perform sampling and testing for field quality control during the placement of materials as follows:
    - 1) HMAC shall be tested as described in these Specifications, at least once for every 500 tons of materials placed or fraction thereof.
    - 2) Density:
      - (a) Compare density of in-place material against laboratory specimen or certificated on same for bituminous concrete mixture.
      - (b) Acceptable densities of in-place materials shall conform to applicable State Standard Requirements.
    - 3) Thickness:
      - (a) In-place compacted thickness shall average not less than the thickness specified.
    - 4) Surface Smoothness:
      - (a) Test finished surface of each bituminous concrete course for smoothness, using a 10-foot straight edge applied parallel to and at right angles to centerline of paved areas.
      - (b) Check surfaced areas at intervals as directed by Engineer.
      - (c) Surfaces will not be acceptable if exceeding the following:
        - (1) Base Courses: 3/8 inch in 10 feet.
        - (2) Surface Course: 1/4 inch in 10 feet.
        - (3) Crowned Surfaces:  
Test crowned surfaces with a crown template, centered and at right angle to the crown.  
Surfaces will not be acceptable if varying more than 1/4" from the template.

### 3.04 PORTLAND CEMENT CONCRETE PAVING

- A. General
  1. Unless otherwise noted in these Specifications, construction methods used to place all Portland Cement Concrete Paving are specified by TxDOT Standard Specifications Item 360, "Concrete Pavement".
- B. Inspection
  1. Verify subgrade is ready to support paving and imposed loads.
  2. Verify gradients and elevations of subgrade are correct.
- C. Preparation
  1. Moisten subgrade to minimize absorption of water from fresh concrete.
  2. Notify Engineer minimum 24 hours prior to commencement of concreting operations.
- D. Forming
  1. Place and secure forms to correct location, dimension, and profile.
  2. Assemble formwork to permit easy stripping and dismantling without damaging concrete.
  3. Place joint fillers vertical in position, in straight lines. Secure to form work during concrete placement.



- E. Reinforcement
  - 1. Reinforcing steel shall be grade 60 and shall be placed as shown on the plans. All concrete shall be continuously reinforced as shown on the plans.
- F. Formed Joints
  - 1. Transverse and longitudinal joints shall be constructed in accordance with the details on the plans and Texas Department of Transportation Specifications.
  - 2. Joints shall be filled in accordance with the construction drawings.
- G. Placing Concrete
  - 1. Ensure reinforcement, inserts, embedded parts and formed joints are not disturbed during concrete placement.
  - 2. Place concrete continuously between predetermined construction joints. Do not break or interrupt successive pours such that cold joints occur.
- H. Finishing
  - 1. All concrete pavement shall be finished in accordance with Texas Department of Transportation Item 360.
- I. Curing
  - 1. The Contractor shall select one of the methods of curing stated in TxDOT Standard Specifications, Item 360, Paragraph 360.11, "Curing".
  - 2. If the Contractor elects to use Membrane Curing, the curing compound must be reviewed by the Engineer.
- J. Field Quality Control
  - 1. Field inspection and testing will be performed according to the requirements of Division 1 - "General Requirements" of this project manual.
  - 2. Maintain records of placed concrete items. Record date, location of pour, quantity, air temperature, and test samples taken.
- K. Protection
  - 1. Immediately after placement, protect concrete under provisions of Section 03 3000 from premature drying excessive hot or cold temperatures, and mechanical injury.

### 3.05 PAVEMENT REPLACEMENT

- A. Cutting of Pavements
  - 1. Concrete and Asphaltic Concrete Pavements:
    - a. All concrete and asphaltic concrete pavements shall be cut with a concrete saw. The depth of the cut shall be such that upon removal of concrete and/or asphaltic concrete the sides of the cut will be straight and square. Care shall be taken, when cutting concrete pavement, not to cut transverse reinforcing steel.
- B. Removal of Bases
  - 1. Concrete and Cement Stabilized Bases:
    - a. Concrete and cement stabilized bases shall be removed by means of hand-held pneumatic pavement breakers with acceptable cutting bits. It is the intent of this specification that the base shall be removed in a manner that will leave the sides of the cut straight and square.
    - b. Where reinforcement is encountered in concrete bases, a minimum of one (1) foot shall be cleaned of all old concrete and left in place to tie to new reinforcement in the new concrete base.

2. Flexible Base:
  - a. Flexible bases shall be removed by normal trenching operations.
- C. Replacement of Bases
  1. Concrete Bases:
    - a. Concrete bases shall be replaced with Class "A" concrete conforming to the provisions of Item No. 300 "Concrete" of the City of San Antonio Standard Specifications for Public Works Construction. If existing concrete is steel reinforced, the reinforcing steel shall be replaced in accordance with Item No. 301, "Reinforcing Steel". The concrete shall have a slump of not more than three inches (3") and shall be spaded, tamped and finished to the satisfaction of the Engineer. Immediately following finishing operations, the surface shall be cured in accordance with the provisions of Item No. 305, "Membrane Curing". The concrete shall be protected from traffic for seventy-two (72) hours.
  2. Cement Stabilized Bases:
    - a. Cement stabilized base shall be replaced with cement stabilized base in accordance with Item No. 201, "Cement Stabilized Base" of the City of San Antonio Standard Specifications for Public Work Construction where width of trench permits, or with Class "C" concrete conforming to the requirements of Item No. 300, "Concrete". The concrete shall have a slump of not more than three inches (3") and shall be spaded, tamped, and finished to the satisfaction of the Engineer. The concrete shall be protected from traffic for seventy-two (72) hours prior to replacement of the surfacing.
  3. Flexible Base:
    - a. Flexible base shall be replaced with 1,000 psi (@ 28 days) concrete. The concrete shall have a slump of no more than three (3) inches and shall be spaded, tamped and finished to the satisfaction of the Inspector. The concrete shall be protected from traffic for seventy-two (72) hours prior to replacement of the surface.
  4. Asphalt Treated Base:
    - a. Asphalt treated base shall be furnished, placed and compacted in accordance with this Section.
- D. Replacement of Pavements
  1. Pavements shall be replaced under this item with either hot mix asphaltic concrete pavement, or Portland Cement concrete pavement in the thickness and type shown on the plans.
  2. When hot mix asphaltic concrete is shown on the plans as replacement of pavement, it shall be furnished and placed in accordance with TxDOT Item 340, "Hot Mix Asphaltic Concrete Pavement". Flexible bases shall be primed with asphalt or emulsion in accordance with the provision of TxDOT Item 310, "Prime Coat" prior to the placement of hot mix asphaltic concrete. All concrete bases shall receive a tack coat of asphalt or emulsion in accordance with the provisions of TxDOT Item 300, "Tack Coat" prior to replacement of hot mix asphaltic concrete.
  3. When the replacement pavement is Portland Cement concrete as indicated on the plans, "Class A" Concrete shall be furnished and placed in accordance with TxDOT Item 360, "Concrete Pavement". The concrete shall be placed, spaded, tamped and finished to the line, grade and texture of the surrounding concrete pavement.
  4. Prime Coat and Tack Coat:

- a. Prime and Tack coats shall be applied in accordance with the Texas Department of Transportation Standard Specifications Item 340.
- 5. Surface Course Applications:
  - a. The construction of all surface course shall conform to the applicable requirements of Item 340 of the TxDOT Standard Specifications.

**3.06 CLEANING AND PROTECTION**

- A. Cleaning
  - 1. After completion of paving operations, clean surfaces of excess or spilled bituminous materials and all foreign matter.
- B. Protect newly finished pavement until it has become properly hardened by cooling.

**END OF SECTION**

**CONCRETE CURBS, GUTTERS AND SIDEWALKS****PART 1 - GENERAL****1.01 SECTION INCLUDES**

- A. Concrete sidewalks, stair steps, curbs and gutters.

**1.02 RELATED SECTIONS**

- A. Section 03 3000 - Cast-in-Place Concrete.
- B. Section 31 2323 - Fill.
- C. Section 32 1123 - Aggregate Base Courses.
- D. Section 32 1250 - Site Pavement.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Sidewalks: By the square foot. Includes preparation of substrate, sand bedding, steel reinforcement, concrete sidewalk, jointing, and finishing.
- B. Concrete Curbs and Gutters: By the linear foot. Includes trenching, steel reinforcement, concrete curb installation, and cleaning.
- C. Stair Steps: By the unit price. Includes preparation of substrate, sand bedding, steel reinforcement, stair steps, jointing, and finishing.

**1.04 REFERENCES**

- A. ASTM D 1190, Concrete Joint Sealer Hot Poured Elastic Type.
- B. ASTM D 994-71 (R1977), Preformed Expansion Joint Filler For Concrete (Bituminous Type).
- C. ASTM D 1751-73 (R1978) Preformed Expansion Joint Fillers for concrete Paving Structural Construction.

**1.05 SUBMITTALS**

- A. Samples: Submit for review samples, applicable manufacturer's product data, test reports and material certifications.
- B. Shop Drawings:
  - 1. Detailed Reinforcing Steel Layout.
  - 2. Detailed Construction And Control Joint Layout.

**1.06 QUALITY ASSURANCE**

- A. The testing laboratory shall sample and test concrete in accordance with Section 03 3000 - Cast-In-Place Concrete.

**PART 2 - PRODUCTS****2.01 MATERIALS**

- A. Unless otherwise specified in this Section, all concrete and concrete materials shall conform to TxDOT, Item 529, "Concrete Curb, Gutter and Combined Curb and Gutter" and Item 531 "Sidewalks".
- B. Bituminous Joint Filler: Bituminous type conforming to ASTM D 994 or D 1751 unless otherwise indicated on drawings.
  - 1. Reinforcing steel shall be ASTM A 615 grade 60, deformed and conform to the provisions of TxDOT Standard Specifications, Item No. 529, "Concrete Curb, Gutter and Combined Curb and Gutter" and Item 531 "Sidewalks".

2. All reinforcing steel to be new billet steel.

## **PART 3 - EXECUTION**

### **3.01 SUBGRADE PREPARATION**

- A. Preparation of the subgrade including compaction shall be completed two feet (2') beyond the limits of the work:
  1. Where the subgrade is constructed by excavation of existing grade, the top six inches (6") of the subgrade shall be compacted to at least 95 percent of maximum density as determined by TxDOT TEX-113-E at a moisture content between optimum and optimum +4 percent unless otherwise indicated.
  2. The subgrade shall be brought to the final lines and grades utilizing select backfill.
  3. Pit Run Sand or Granular Embedment:
    - a. Pit run sand or granular embedment shall be provided as shown on drawings.
    - b. The material shall be as specified in Section 31 2323 and compacted as specified.

### **3.02 FORM CONSTRUCTION**

- A. Forms shall be in conformance with TxDOT Standard Specification, Item 529, "Concrete Curb, Gutter and Combined Curb and Gutter" and Item 531 "Sidewalks".
- B. Set forms to line and grade. Install forms over full length of curbs, gutters and sidewalks.

### **3.03 REINFORCEMENT**

- A. Locate, place, and support reinforcement as specified in TxDOT Standard Specifications, Item 529, "Concrete Curb, Gutter and Combined Curb and Gutter" and Item 531 "Sidewalks", unless otherwise shown on drawings.

### **3.04 CONCRETE PLACEMENT**

- A. General: Comply with the requirements of TxDOT Standard Specifications, Item 529, "Concrete Curb, Gutter and Combined Curb and Gutter" and Item 531 "Sidewalks".
- B. Machine Formed/Hand Formed:
  1. Automatic curb, gutter and sidewalk machine may be used in lieu of hand formed methods for forming and placing.
  2. Concrete shall have properties as previously specified, except that maximum slump shall be 2-1/2 inches (2 1/2") and air content shall be two percent (2%).
  3. Machine forming shall produce curbs, gutters and sidewalks to the required cross-section, lines, and grades, finish and jointing, as specified for conventionally formed concrete.
  4. Unacceptable work will be removed and replaced at Contractor's expense.

### **3.05 JOINTS**

- A. General:
  1. Construct expansion, contraction, and construction joints with faces perpendicular to surface of the curb, gutter and sidewalk.
  2. Construct transverse joints at right angles to the work centerline and as shown.
- B. Control Joints:
  1. Provide these joints at ten feet (10') on centers for curbs and gutters and five feet (5') on centers for sidewalks.
- C. Construction Joints

1. Place joints at locations where placement operations are stopped for a period of more than 1/2 hour, except where such pours terminate at expansion joints.
- D. Expansion Joints
  1. Provide 1/2 inch expansion joint filler where work abuts structures; at returns; and at 50-foot spacing for straight runs.
  2. Where gutter and sidewalk are not poured monolithically, provide expansion joints where each abuts the other.
  3. Place top of expansion joint filler not less than 1/2 inch or more than one inch (1") below concrete surface.
  4. Apply joint sealer on top of expansion joint material flush with concrete surface, and in accordance with manufacturer's instructions.

**3.06 CONCRETE FINISHING**

- A. Smooth the exposed surface by screeding and floating.
- B. Work edges of gutter and sidewalks, back top edge of curb, and transverse joints; and round to 1/4-inch radius.
- C. Complete surface finishing by drawing a fine-hair broom across surface, perpendicular to line of traffic unless alternative finish is indicated on drawings.

**3.07 CURING**

- A. Protect and cure finished concrete curbs, gutters and sidewalks, complying with applicable requirements of TxDOT Standard Specifications, Item 529, "Concrete Curb, Gutter and Combined Curb and Gutter" and Item 531 "Sidewalks".

**3.08 REPAIR AND CLEANING**

- A. Broken or defective curb, gutters and sidewalks shall be repaired or replaced as directed by the Engineer at the Contractor's expense.
- B. Sweep work and wash free of stains, discolorations, dirt or other foreign material.

**END OF SECTION**

**SEGMENTAL RETAINING WALLS****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Segmental retaining walls made of modular concrete units with or without soil reinforcement.
- B. Engineering design criteria for Contractor design submittals for segmental retaining walls.
- C. Retaining wall units.
- D. Cap units.
- E. Shear connectors.
- F. Concrete for leveling pad.
- G. Drainage fill.
- H. Reinforced backfill.
- I. Drainage pipe.

**1.02 PRICE AND PAYMENT PROCEDURES**

- A. Excavation of unsuitable soil and replacement with acceptable fill will be paid by the cubic yard.
- B. Segmental retaining walls inclusive of leveling pad and structural fill will be paid by the face square foot.

**1.03 RELATED REQUIREMENTS**

- A. Section 31 1000 - Site Clearing.
- B. Section 31 2200 - Grading.
- C. Section 31 2316 - Excavation.
- D. Section 31 2323 - Fill.
- E. Section 33 4100 - Subdrainage.

**1.04 REFERENCE STANDARDS**

- A. AASHTO M 288 - Standard Specification for Geotextiles; 2006.
- B. ASCE 7 - Minimum Design Loads for Buildings and Other Structures; 2010, with 2013 Supplements and Errata.
- C. ASTM C140/C140M - Standard Test Methods of Sampling and Testing Concrete Masonry Units and Related Units; 2014.
- D. ASTM C920 - Standard Specification for Elastomeric Joint Sealants; 2014.
- E. ASTM C1372 - Standard Specification for Dry-Cast Segmental Retaining Wall Units; 2011.
- F. ASTM D448 - Standard Classification for Sizes of Aggregate for Road and Bridge Construction; 2012.
- G. ASTM D698 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft<sup>3</sup> (600 kN-m/m<sup>3</sup>)); 2012.
- H. ASTM D1241 - Standard Specification for Materials for Soil-Aggregate Subbase, Base, and Surface Courses; 2007.
- I. ASTM D1557 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft<sup>3</sup> (2,700 kN m/m<sup>3</sup>)); 2012.

- J. ASTM D2487 - Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System); 2011.
- K. ASTM D4318 - Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils; 2010.
- L. ASTM D4355/D4355M - Standard Test Method for Deterioration of Geotextiles by Exposure to Light, Moisture and Heat in a Xenon Arc Type Apparatus; 2014.
- M. ASTM D4491/D4491M - Standard Test Methods for Water Permeability of Geotextiles by Permittivity; 2017.
- N. ASTM D4595 - Standard Test Method for Tensile Properties of Geotextiles by the Wide-Width Strip Method; 2017.
- O. ASTM D4632/D4632M - Standard Test Method for Grab Breaking Load and Elongation of Geotextiles; 2015a.
- P. ASTM D4751 - Standard Test Method for Determining Apparent Opening Size of a Geotextile; 2012.
- Q. ASTM D5262 - Standard Test Method for Evaluating the Unconfined Tension Creep and Creep Rupture Behavior of Geosynthetics; 2007 (Reapproved 2016).
- R. ASTM D5321/D5321M - Standard Test Method for Determining the Shear Strength of Soil-Geosynthetic and Geosynthetic-Geosynthetic Interfaces by Direct Shear; 2014.
- S. ASTM D5818 - Standard Practice for Exposure and Retrieval of Samples to Evaluate Installation Damage of Geosynthetics; 2011.
- T. ASTM D6638 - Standard Test Method for Determining Connection Strength Between Geosynthetic Reinforcement and Segmental Concrete Units (Modular Concrete Blocks); 2011.
- U. ASTM D6706 - Standard Test Method for Measuring Geosynthetic Pullout Resistance in Soil; 2001 (Reapproved 2013).
- V. ASTM D7928 - Standard Test Method for Particle-Size Distribution (Gradation) of Fine-Grained Soils Using the Sedimentation (Hydrometer) Analysis; 2017.
- W. NCMA TR-127 - Design Manual for Segmental Retaining Walls; 2010, Third Edition.
- X. NCMA TR-160 - Seismic Design Manual for Segmental Retaining Walls; National Concrete Masonry Association; 1998.

#### **1.05 ADMINISTRATIVE REQUIREMENTS**

- A. Preinstallation Meeting: Conduct a preinstallation meeting one week prior to the start of the work of this section with the material supplier, installer, and the Engineer.

#### **1.06 SUBMITTALS**

- A. Segmental Retaining Wall Units:
  - 1. Manufacturer's product data.
  - 2. ICC-ES evaluation report.
  - 3. Test data on freeze-thaw durability per ASTM C1372.
  - 4. Test data on unit strength and shear resistance between units.
  - 5. Test data on soil reinforcement connection.
  - 6. Manufacturer's certification that units meet requirements of specification.
  - 7. Storage and handling requirements and recommendations.
  - 8. Installation methods.



- B. Soil Reinforcement:
  - 1. Manufacturer's product data.
  - 2. ICC-ES evaluation report.
  - 3. Manufacturer's certificate that product meets requirements of specification.
  - 4. Preparation instructions and recommendations.
  - 5. Storage and handling requirements and recommendations.
  - 6. Installation methods.
- C. Shop Drawings: Engineering drawings for installation, including elevations, large-scale details of elevations, typical sections, details, and connections, soil reinforcement, and drainage provisions.
  - 1. Include marked up contract drawings showing exact dimensions for blocks, required coping, and other minor revisions.
  - 2. Design Data: Submit detailed design calculations showing compliance with specified design criteria and material evaluations performed in accordance with specified design standard, signed and sealed by Design Engineer.
  - 3. Submit no less than 2 weeks prior to start of work.
  - 4. Obtain approval of Engineer prior to start of work.
- D. Unit Sample for Selection: Minimum 3 inch square pieces of actual units showing colors and finish textures available.
- E. Soil Reinforcement Sample: Two pieces of each specified type, labeled, 12 by 12 inches.
- F. Soil Reinforcement to Unit Connector: One connector.
- G. Design Engineer's Qualification Statement.
- H. Concrete Unit Manufacturer Qualification Statement.
- I. Installer Qualification Statement.
- J. Maintenance Materials: Provide the following for Owner's use in maintenance of project.
  - 1. Extra Retaining Wall Units: Three.

#### 1.07 QUALITY ASSURANCE

- A. Design Engineer Qualifications: Provide design by or under direct supervision of Professional Engineer experienced in the work of this section and licensed in the State in which the Project is located and:
  - 1. Having experience in design of reinforced soil structures.
  - 2. Employed by firm that has designed a minimum of 500,000 square feet of segmental retaining walls.
  - 3. Having minimum of \$1,000,000 aggregate liability insurance.
- B. Geotechnical Engineer: Employed by Owner; licensed in the State in which the Project is located.
- C. Manufacturer Qualifications -- Concrete Units: Firm specializing in manufacturing products specified in this section.
- D. Installer Qualifications: Firm specializing in design and installation of segmental retaining walls and:
  - 1. Having site supervisor with verifiable qualified experience suitable for this project.

**1.08 DELIVERY, STORAGE, AND HANDLING**

- A. Store products above ground on wood pallets or blocking, in manufacturer's unopened packaging, until ready for installation.
- B. Prevent excessive soil and mud from coming in contact with face of concrete units.
- C. Polymeric Materials: Rolled materials may be laid flat or stood on end.
- D. Protect material from damage. Do not use damaged material. Remove damaged material from the site.
- E. Store and dispose of solvent-based materials, and materials used with solvent-based materials, in accordance with requirements of local authorities having jurisdiction.

**PART 2 PRODUCTS****2.01 MANUFACTURERS**

- A. Segmental Retaining Walls:
  - 1. Anchor Wall Systems, Inc: [www.anchorwall.com/#sle](http://www.anchorwall.com/#sle).
  - 2. Keystone Retaining Wall Systems LLC; Keystone Compac: [www.keystonewalls.com/#sle](http://www.keystonewalls.com/#sle).
  - 3. Other manufacturers may be considered subject to Engineer's approval.

**2.02 RETAINING WALLS**

- A. Contractor is responsible for design of the retaining walls unless a sign and sealed retaining wall plan is provided.
- B. Alternative solutions will be considered provided Contractor provides complete engineering design at no extra cost to Owner.
  - 1. Alternative Solutions: Comply with requirements specified for substitution review.
  - 2. Shop drawing submittal is required only for approved alternative solutions.
- C. Design Standard: Design retaining walls to be capable of withstanding the effects of gravity loads due to soil pressures resulting from grades indicated, determined in accordance with:
  - 1. Include effects of sloped backfill as indicated on drawings.
  - 2. Include effects of superimposed loads (surcharge) as indicated on drawings.
  - 3. In addition, comply with applicable local, state, and federal codes and regulations.
  - 4. This design method considers potential failure modes categorized by external, internal, local, compound, and global stability.
  - 5. Engineer has considered global stability and has indicated minimum design requirements in Contract Documents.
  - 6. Seismic Stability: Perform analysis in accordance with local requirements.
  - 7. Provide engineering services as required for analysis for all modes of stability.
  - 8. Use of design software for calculations is permitted.
  - 9. Submit complete shop drawings showing all features of the design.
- D. Setback: 1/8 inch minimum back from face per course or as indicated on drawings.
- E. Mass (Weight) Per Wall Face Area: 35 pounds per square foot, minimum, including filled voids.
- F. Shear Resistance: Design the wall not to exceed the capacity of materials and soils to resist shear:
  - 1. Shear Resistance Between Units: Determine in accordance with ASTM D6916.

2. Connection Between Units and Soil Reinforcement: Determine in accordance with ASTM D6638. Strength shall exceed the maximum tensile force with a Factor of Safety of 1.5.
  3. Coefficient for Direct Shear of Reinforcement on Soil: Determine in accordance with ASTM D5321/D5321M using soil similar in gradation and texture to that to be used for fill in the reinforced zone.
- G. Soil Reinforcement:
1. Test reinforcement to be used in accordance with ASTM D6706 using soil taken from project site.
  2. Do not use more than one type of reinforcement attached to units within the same wall; do not use products made by different manufacturers in the same wall; minimize the number of different reinforcement and filter products to avoid confusion in placement.
  3. Walls Less Than 12 feet High: Use only one type of reinforcement of one grade and strength.
  4. Length Back from Wall: Not less than dimensions shown on drawings.
  5. Long Term Design Strength of Reinforcement:  $LTDS = T_{ult} / (RF_d \times RF_{id} \times RF_{cr})$ , where:
    - a.  $T_{ult}$  = Ultimate (tensile) strength per ASTM D4595.
    - b.  $RF_d$  = Reduction Factor for chemical and biological durability; minimum 2.0 if durability testing has not been conducted, otherwise 1.1 for High-density polyethylene (HDPE), and 1.1 for polyethylene terephthalate (PET).
    - c.  $RF_{id}$  = Reduction Factor for Installation Damage; minimum 1.1 and 3.0 if testing per ASTM D5818 has not been conducted.
    - d.  $RF_{cr}$  = Reduction Factor for Creep; consistent with test procedure used for determining the ultimate strength per ASTM D5262.
    - e. The product  $RF_d \times RF_{id} \times RF_{cr}$  shall be greater than 2.0.
- H. Drainage: Design to prevent water accumulation in retained soil; use drainage fill and drainage pipe as specified in Section 33 4100; provide outlets at 50 foot intervals along length of wall, minimum.
- I. Minimum Factor of Safety: Design with the following stability requirements:
1. Sliding = 1.5.
  2. Pullout = 1.5.
  3. Tensile Overstress = 1.5.
  4. Overturning = 2.0.
  5. Bearing Capacity = 2.0.

## 2.03 MATERIALS

- A. Retaining Wall Units: Machine-formed concrete blocks of shapes and sizes suitable for the retaining wall configuration required and complying with ASTM C1372 and the following:
1. Face Color: Coordinate color with Owner .
  2. Texture: Coordinate color with Owner.
  3. Face Shape: Straight (flat).
  4. Curved Walls: Provide unit shapes that accommodate the required curves without cutting and with gaps between faces of adjacent units of not more than 1/8 inch and a minimum radius of 3 1/2 feet.
  5. Acute Corners: Provide special shapes to form corners without cutting; exposed faces finished to match.
  6. Unit Face Area: 0.75 sq ft, minimum.

7. Height: 8 inches, minimum.
  8. Length (Face Width): 12 inches minimum, 18 inches maximum.
  9. Width (Depth from Face): 10 inches, minimum, not including textured finish.
  10. Face Wall Thickness: 2 inches, minimum.
  11. Batter Dimensional Control: Provide integral positive control to maintain consistent batter dimension.
  12. Shear Resistance Mechanism: Manufacturer's standard.
    - a. Peak Shear Capacity: Determined in accordance with NCMA SRWU-2.
    - b. Service State Shear Capacity: Determined in accordance with NCMA SRWU-2.
  13. Moisture Absorption: 8 percent, maximum.
  14. Compressive Strength, 28 Day: 3000 pounds per square inch, minimum in accordance with ASTM C140/C140M.
  15. Concrete Density: 125 pounds per cubic foot, minimum, oven dry.
  16. Dimensional Tolerances: Plus/minus 1/16 inch from specified dimension.
  17. Appearance: No visible chips, cracks, or other imperfections when viewed from 10 feet under diffuse lighting.
- B. Cap Units: Portland cement concrete machine-formed solid blocks, matching segmental retaining wall units, complying with ASTM C1372, with abutting edges saw cut or formed to provide tight fitting, flush end-to-end joints.
1. Height: 4 inches, minimum.
  2. Width: Same as wall units.
  3. Depth: To fully cover wall units.
  4. Masonry Adhesive: To secure cap units as top course of wall.
    - a. Expected Life Span: 30 years.
    - b. Provide adhesive complying with ASTM C920, Type S, Grade NS, Class 25, and as approved by unit manufacturer.
- C. Shear Connectors: Connection method to withstand design stresses and prevent movement of segmental retaining wall units, and to hold soil reinforcement in proper design position during grid pre-tensioning and backfilling.
1. Flexural Strength: 128,000 pounds per square inch, minimum, determined in accordance with ASTM D6638.
  2. Short Beam Shear: 6,400 pounds per square inch, minimum, determined in accordance with ASTM D6638.
  3. Maintain strength over design temperature range of minus 10 degrees F to plus 100 degrees F.
- D. Soil Reinforcement: Polymeric geosynthetic specifically fabricated for use as soil reinforcement, dimensionally stable and able to retain geometry under manufacture, transport, and installation. The following standards are the minimum requirements unless otherwise indicated on drawings.
1. Polymeric Material: 100 percent virgin resin with maximum of 5 percent in-plant regrind material; polypropylene, polyethylene, or polyester.
    - a. Polyethylene and Polypropylene: Stabilized with long term antioxidants.
    - b. Polyester: Minimum molecular weight of 25,000 and carboxyl end group number less than 30.
  2. Construction: Woven.

3. Long Term Design Strength: Per Engineer's design requirements when tested in accordance with NCMA TR 127.
  4. Ultimate Tensile Strength: Per Engineer's design requirements when tested in accordance with NCMA TR127.
  5. Permittivity: 0.5 per second, minimum, when tested in accordance with ASTM D4491/D4491M.
  6. UV Resistance: 70 percent after 500 hours, when tested in accordance with ASTM D4355/D4355M.
  7. Coefficient for Direct Shear: In accordance with ASTM D 5321.
  8. Durability: Comply with minimum requirements of AASHTO M 288 Class 1; minimum mass of 8 ounces per square yard.
  9. Source Quality Control: Test each 40,000 square feet of production, per lot or per day, for compliance with specified design properties.
- E. Drainage Filter: Geosynthetic textile. The following standards are the minimum requirements unless otherwise indicated on drawings.
1. Tensile Strength (Grab) - Woven: 250 pounds, when tested in accordance with ASTM D4632/D4632M.
  2. Tensile Strength (Grab) - Non-Woven: 160 pounds, when tested in accordance with ASTM D4632/D4632M.
  3. Apparent Opening Size: 70 to 100 U.S. Sieve size, when tested in accordance with ASTM D4751.
  4. Permittivity: 0.5 per second, minimum, when tested in accordance with ASTM D4491/D4491M.
  5. Durability: Comply with minimum requirements of AASHTO M 288 Class 1; minimum mass of 8 ounces per square yard.
- F. Aggregate for Leveling Pad: Compacted sand, gravel, or crushed rock as indicated on drawings.
- G. Concrete for Leveling Pad: Unreinforced concrete with compressive strength of 3,000 pounds per square inch.
- H. Drainage Fill: Clean, freely draining aggregate placed within, between, or immediately behind segmental retaining wall units; do not use pea gravel; use one of the following:
1. Aggregate as approved by Engineer.
  2. Aggregate meeting requirements of ASTM D448, Size No. 57.
  3. Crushed stone or coarse gravel, meeting requirements of ASTM D7928.
    - a. Sieve Size 1 inch: 100 percent passing.
    - b. Sieve Size 3/4 inch: 75 to 100 percent passing.
    - c. Sieve Size No. 4: 0 to 60 percent passing.
    - d. Sieve Size No. 40: 0 to 50 percent passing.
    - e. Sieve Size No. 200: 0 to 5 percent passing.
- I. Reinforced Backfill: Compacted soil placed behind drainage fill within reinforced soil mass; do not use heavy clay or organic soils; comply with one of the following:
1. Granular soil with less than 35 percent passing No. 200 sieve per ASTM D7928.
- J. Drainage Pipe: 4 inch Perforated schedule 40 PVC, complying with ASTM D3034; or corrugated HDPE complying with ASTM F405; with geotextile filter wrap.

**PART 3 EXECUTION****3.01 EXAMINATION**

- A. Verify location of existing structures and utilities prior to excavation.
- B. Protect adjacent structures from the effects of excavation.
- C. Verify that layout dimensions are correct and substrate is in proper condition for installation.
- D. Notify Engineer of unsatisfactory conditions.
- E. Do not proceed with installation until unsatisfactory conditions have been corrected.

**3.02 PREPARATION**

- A. Sitework:
- B. Excavation:
  - 1. Excavate to lines and grades indicated on drawings.
  - 2. Do not disturb embankment or foundation beyond lines. Minimize over-excavation; fill over-excavated areas with compacted reinforced backfill or leveling pad material at Contractor's expense.
  - 3. After excavation, and prior to placement of leveling materials, Geotechnical Engineer will examine bearing soil surface to verify strength meets or exceeds design requirements and assumptions.
  - 4. Replace unsuitable bearing soil as directed by Engineer.
- C. Leveling Pad:
  - 1. Depth: 6 inches, minimum or as indicated on drawings.
  - 2. Width: 6 inches minimum extension beyond front and back faces of units.
  - 3. In lieu of pad made solely of aggregate or concrete, pad may be 6 inches, minimum, of thick compacted crushed rock, unless otherwise indicated on drawings.
  - 4. Aggregate Base: Crushed stone or granular fill meeting the following gradation as determined in accordance with ASTM D448 unless otherwise indicated on drawings:

<u>Sieve Size</u>	<u>Percent Passing</u>
1 inch	100
No. 4	35 to 70
No. 40	10 to 35
No. 200	3 to 10

- a. Base Thickness: 6 inches (minimum compacted thickness).
  - 5. Compact aggregate to lines and grades on drawings, in lifts 6 inches thick, maximum.
  - 6. Compact aggregate to a minimum of 95 percent standard Proctor density, when determined in accordance with ASTM D 698 at moisture content within +/- 4 percent of optimum.
  - 7. Use only hand-operated compaction equipment within 36 inches of back of wall.
- D. Verify level grade before proceeding.
- E. Install drainage collection pipe with a continuous fall in the direction of flow. Cap open ends as necessary to prevent soil and debris from entering.

**3.03 INSTALLATION**

- A. Install in accordance with drawings, manufacturer instructions, and applicable codes and regulations.
- B. Segmental Retaining Wall Units:
  - 1. Place first course of units on leveling pad; check alignment and level. Check for full contact with base and for stability.
  - 2. Place units side by side for full length of wall, aligning back face of straight walls using string line or offset from base line and back face of curved walls using flexible pipe or other method recommended by manufacturer.
  - 3. Do not leave gaps between units.
  - 4. Lay out corners and curves in accordance with manufacturer's instructions. Do not leave gaps to produce wall batter or curvature.
  - 5. Cut blocks with saw; do not split units.
  - 6. Sweep excess material from tops of units before laying succeeding courses.
  - 7. Place a maximum of 2 succeeding courses above level backfill. Check for proper alignment and batter.
  - 8. Where top of wall changes elevation, step units to match grade or turn top course into embankment.
  - 9. Where bottom of wall changes elevation, step base leveling pad and extend lowest course a minimum of two units into slope.
  - 10. Install shear connectors per manufacturer recommendations.
- C. Soil Reinforcement: Install each layer on fully compacted fill.
  - 1. Orient soil reinforcement material with highest strength axis perpendicular to wall alignment.
  - 2. Attach to top of wall units and extend horizontally, full length, over compacted backfill slightly sloping downward away from wall.
  - 3. Install in one piece lengths with 100 percent coverage in each layer at each level. Do not splice or leave gaps between panels or ends of pieces.
  - 4. Pull taut and remove slack prior to backfill placement.
- D. Drainage Fill: Place drainage fill in, between, and behind units.
  - 1. Compact to lines and grades on drawings, in lifts 6 inches thick, maximum; decrease lift thickness where necessary to achieve required density.
  - 2. Extend drainage fill beyond back face of units in accordance with drawings.
  - 3. Base of drainage fill elevation shall not exceed two courses or 16 inches from base of wall units.
- E. Backfill: Place, spread, and compact backfill from behind drainage fill to undisturbed soil while minimizing the development of slack in the soil reinforcement.
  - 1. Use only lightweight hand-operated compaction equipment within 3 feet from back wall face, or one half of wall height, whichever is greater.
  - 2. Place backfill in lifts of maximum 6 inches to 8 inches loose thickness where hand compaction is used and 8 inches to 10 inches where heavy compaction equipment is used.
  - 3. Compact backfill to 95 percent maximum density and upper 2 feet of backfill to 98 percent maximum density, standard Proctor, as determined in accordance with ASTM D698, or as recommended by Geotechnical Engineer.

4. Moisture content of backfill prior to and during compaction to be within plus or minus 2 percentage points dry of optimum and uniform throughout each layer.
  5. Do not operate tracked construction equipment directly upon soil reinforcement. Maintain a minimum fill thickness of 6 inches for operation of tracked vehicles over soil reinforcement. Minimize turning of tracked vehicles while over soil reinforcement.
  6. Operate wheeled equipment at speeds less than 10 miles per hour over soil reinforcement.
  7. Prevent contamination of the filter fabric, unit fill, blanket drains, chimney drains, and/or drainage composite from poor drainage materials such as fine grained silt and clay.
- F. Cap Units: Install and top two courses of units with masonry adhesive.
1. Verify in-place top of wall elevation prior to installation of cap units and adjust accordingly.
  2. Clear cap units and top course of segmental retaining wall units of debris and standing water before applying adhesive.
  3. Apply masonry adhesive to top surface of top unit and place cap into position over projecting pins. Protect wall face from masonry adhesive.
- G. Site Drainage:
1. At end of each day:
    - a. Grade backfill a minimum of 2 percent away from wall to prevent runoff from adjacent areas from entering wall site and to prevent ponding at the wall.
    - b. Construct a berm at the crest of the wall to prevent surface water from overtopping.
  2. At completion, if other work adjacent to wall is not to be done immediately (paving, landscaping, etc), grade top of backfill and provide temporary drainage to prevent water runoff toward the wall.
  3. Surface water control and groundwater seepage shall be the responsibility of the project Engineer.

### 3.04 TOLERANCES

- A. Top of Wall:
1. Plan Location: Maximum of plus/minus 1 inch from plan location.
  2. Elevation: Maximum of plus/minus 1-1/2 inch from elevations shown on drawings.
- B. Face of Wall Flatness: Measured as deviation from a straight edge.
1. In the Vertical Dimension: Plus/minus 1-1/4 inch per 10 foot section.
  2. In the Horizontal Dimension of Straight Walls: Plus/minus 1-1/2 inch per 10 foot section.
- C. Overall Wall Batter: Within 2 degrees of design, measured from the vertical.
- D. Gap Between Adjacent Units: 1/8 inch, maximum.

### 3.05 FIELD QUALITY CONTROL

- A. See Section 01 4000 - Quality Requirements, for additional requirements.
- B. Provide manufacturer's field representative to observe and inspect concrete units.
- C. Observe and inspect:
1. Concrete units: For correct type, for quality installation with courses that are level and follow the designed batter ratio.
  2. Soil backfill: For correct type, for specified compaction with level grading prior to reinforcement installation.



- 3. Soil reinforcement: For correct type, for solid connection to concrete units, and for smooth and taut installation.
- 4. Field location in plan and elevation.
- D. Soil Tests: For every new soil type and/or for every 2,000 cubic yard per running foot perform Atterberg Limit, Sieve Analysis, and Proctor Compaction tests. Perform additional testing per project Engineer.
- E. Owner will engage inspection and testing services, including independent laboratories, to provide quality assurance and testing services during construction. Contractor will secure necessary construction control testing during construction.
- F. Contractor shall correct work found deficient and not in accordance with drawings and specifications.

**3.06 CLEANING**

- A. Clean wall face to remove debris and stains.
- B. Leave adjacent paved areas broom clean.

**3.07 PROTECTION**

- A. Prevent damage to wall and earthwork by subsequent construction and uncontrolled runoff until substantial completion; repair damage due to failure to protect wall or earthwork.
- B. Do not operate equipment with wheel loads in excess of 150 pounds per square foot live load within 10 feet from the wall face.
- C. Do not place temporary soil or fill stockpiles adjacent to wall.

**END OF SECTION**

**SEEDING****PART 1 GENERAL****1.01 Section Includes**

- A. Preparation of subsoil.
- B. Placing topsoil.
- C. Hydroseeding, mulching and fertilizer.
- D. Maintenance.

**1.02 Related Requirements**

- A. Section 31 2200 - Grading.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Topsoil:
  - 1. Basis of Measurement: By the cubic yard.
  - 2. Basis of Payment: Includes topsoil, placing topsoil.
- B. Grassed Areas:
  - 1. Basis of Measurement: By the square yard.
  - 2. Basis of Payment: Includes preparation of subsoil, preparation of topsoil, placing topsoil, seeding, watering and maintenance for 90 days or until established.

**1.04 DEFINITIONS**

- A. Weeds: Include Dandelion, Jimsonweed, Quackgrass, Horsetail, Morning Glory, Rush Grass, Mustard, Lambsquarter, Chickweed, Cress, Crabgrass, Canadian Thistle, Nutgrass, Poison Oak, Blackberry, Tansy Ragwort, Bermuda Grass, Johnson Grass, Poison Ivy, Nut Sedge, Nimble Will, Bindweed, Bent Grass, Wild Garlic, Perennial Sorrel, and Brome Grass.

**1.05 Submittals**

- A. Topsoil samples.
- B. Maintenance Data: Include maintenance instructions, cutting method and maximum grass height; types, application frequency, and recommended coverage of fertilizer.
- C. Maintenance Contract.

**1.06 DELIVERY, STORAGE, AND HANDLING**

- A. Deliver grass seed mixture in sealed containers. Seed in damaged packaging is not acceptable. Deliver seed mixture in containers showing percentage of seed mix, year of production, net weight, date of packaging, and location of packaging.
- B. Deliver fertilizer in waterproof bags showing weight, chemical analysis, and name of manufacturer.

**PART 2 PRODUCTS****2.01 SEED MIXTURE**

- A. Seed Mixture: As determined by the Landscape Architect or Owner.

**2.02 SOIL MATERIALS**

- A. Topsoil: Fertile, agricultural soil, typical for locality, capable of sustaining vigorous plant growth, taken from drained site; free of subsoil, clay or impurities, plants, weeds and roots; pH value of minimum 5.4 and maximum 7.0.

**2.03 ACCESSORIES**

- A. Mulching Material: Hay, oat or wheat straw, free from weeds, foreign matter detrimental to plant life, and dry.
- B. Fertilizer: Recommended for grass, with fifty percent of the elements derived from organic sources; of proportion necessary to eliminate any deficiencies of topsoil, as indicated by analysis.
- C. Water: Clean, fresh and free of substances or matter that could inhibit vigorous growth of grass.
- D. Erosion Fabric: Jute matting, open weave.
- E. Herbicide.
- F. Stakes: Softwood lumber, chisel pointed.
- G. String: Inorganic fiber.
- H. Edging: Galvanized steel.

**2.04 TESTS**

- A. Analyze to ascertain percentage of nitrogen, phosphorus, potash, soluble salt content, organic matter content, and pH value.
- B. Submit minimum 10 oz sample of topsoil proposed. Forward sample to approved testing laboratory in sealed containers to prevent contamination.
- C. Testing is not required if recent tests are available for imported topsoil. Submit these test results to the testing laboratory for approval. Indicate, by test results, information necessary to determine suitability.

**PART 3 EXECUTION****3.01 EXAMINATION**

- A. Verify that prepared soil base is ready to receive the work of this Section.

**3.02 PREPARATION**

- A. Prepare subgrade in accordance with Section 31 2200.
- B. Place topsoil in accordance with Section 32 9119.
- C. Install edging at periphery of seeded areas in straight lines to consistent depth.

**3.03 FERTILIZING**

- A. Apply fertilizer in accordance with manufacturer's instructions.
- B. Apply after smooth raking of topsoil and prior to roller compaction.
- C. Do not apply fertilizer at same time or with same machine as will be used to apply seed.
- D. Mix thoroughly into upper 2 inches of topsoil.
- E. Lightly water to aid the dissipation of fertilizer.

**3.04 SEEDING**

- A. Apply seed per seed providers recommendation.
- B. Do not seed areas in excess of that which can be mulched on same day.
- C. Planting Season: Dependent upon seed species.

- D. Do not sow immediately following rain, when ground is too dry, or during windy periods.
- E. Roll seeded area with roller not exceeding 112 lbs.
- F. Immediately following seeding, apply mulch to a thickness of 1/8 inches. Maintain clear of shrubs and trees.
- G. Apply water with a fine spray immediately after each area has been mulched. Saturate to 4 inches of soil.
- H. Following germination, immediately re-seed areas without germinated seeds that are larger than 24 by 24 inches.

**3.05 HYDROSEEDING**

- A. Apply seeded slurry per seed providers recommendations.
- B. Do not hydroseed area in excess of that which can be mulched on same day.
- C. Immediately following seeding, apply mulch to a thickness of 1/8 inches. Maintain clear of shrubs and trees.
- D. Apply water with a fine spray immediately after each area has been mulched. Saturate to 4 inches of soil.
- E. Following germination, immediately re-seed areas without germinated seeds that are larger than 24 by 24 inches.

**3.06 PROTECTION**

- A. Cover seeded slopes where grade is 4 inches per foot or greater with erosion fabric. Roll fabric onto slopes without stretching or pulling.
- B. Lay fabric smoothly on surface, bury top end of each section in 6 inch deep excavated topsoil trench. Provide 12 inch overlap of adjacent rolls. Backfill trench and rake smooth, level with adjacent soil.
- C. Secure outside edges and overlaps at 36 inch intervals with stakes.
- D. Lightly dress slopes with topsoil to ensure close contact between fabric and soil.
- E. At sides of ditches, lay fabric laps in direction of water flow. Lap ends and edges minimum 6 inches.

**3.07 Maintenance**

- A. Provide maintenance at no extra cost to Owner; Owner will pay for water.
- B. Provide a separate maintenance contract for specified maintenance service.
- C. Maintain seeded areas immediately after placement until grass is well established and exhibits a vigorous growing condition.
- D. Mow grass at regular intervals to maintain at a maximum height of 3 inches. Do not cut more than 1/3 of grass blade at any one mowing.
- E. Neatly trim edges and hand clip where necessary.
- F. Immediately remove clippings after mowing and trimming.
- G. Water to prevent grass and soil from drying out.
- H. Roll surface to remove minor depressions or irregularities.
- I. Control growth of weeds. Apply herbicides in accordance with manufacturer's instructions. Remedy damage resulting from improper use of herbicides.

- J. Immediately reseed areas that show bare spots.
- K. Protect seeded areas with warning signs during maintenance period.

**END OF SECTION**

**SODDING****PART 1 GENERAL****1.01 Section Includes**

- A. Preparation of subsoil.
- B. Placing topsoil.
- C. Fertilizing.
- D. Sod installation.
- E. Maintenance.

**1.02 Related Requirements**

- A. Section 31 2200 - Grading.
- B. Section 31 2323 - Fill.
- C. Section 32 9119 - Landscape Grading.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Topsoil:
  - 1. Basis of Measurement: By the cubic yard.
  - 2. Basis of Payment: Includes topsoil, placing topsoil.
- B. Sodded Areas:
  - 1. Basis of Measurement: By the square yard.
  - 2. Basis of Payment: Includes preparation of subsoil, preparation of topsoil, placing topsoil, sodding, watering and maintenance.

**1.04 DEFINITIONS**

- A. Weeds: Includes Dandelion, Jimsonweed, Quackgrass, Horsetail, Morning Glory, Rush Grass, Mustard, Lambsquarter, Chickweed, Cress, Crabgrass, Canadian Thistle, Nutgrass, Poison Oak, Blackberry, Tansy Ragwort, Bermuda Grass, Johnson Grass, Poison Ivy, Nut Sedge, Nimble Will, Bindweed, Bent Grass, Wild Garlic, Perennial Sorrel, and Brome Grass.
- B. Grass: Bermuda, St. Augustine, Buffalo; other acceptable grasses suitable for the region and climate.

**1.05 Reference Standards**

- A. TPI (SPEC) - Guideline Specifications to Turfgrass Sodding; 2006.

**1.06 Submittals**

- A. Certificate: Certify grass species and location of sod source.
- B. Maintenance Data: Include maintenance instructions, cutting method and maximum grass height; types, application frequency, and recommended coverage of fertilizer.

**1.07 QUALITY ASSURANCE**

- A. Sod Producer: Company specializing in sod production and harvesting, and certified by the State of Texas.
- B. Installer Qualifications: Company approved by the sod producer.

**1.08 DELIVERY, STORAGE, AND HANDLING**

- A. Deliver sod on pallets or in rolls. Protect exposed roots from dehydration.

- B. Do not deliver more sod than can be laid within 48 hours.

**PART 2 PRODUCTS****2.01 MATERIALS**

- A. Sod: TPI (SPEC), Certified Turfgrass Sod quality; cultivated grass sod; type indicated in plant schedule on Drawings; with strong fibrous root system, free of stones, burned or bare spots; containing no more than 5 weeds per 1000 sq ft. Minimum age of 18 months, with root development that will support its own weight without tearing, when suspended vertically by holding the upper two corners.
  - 1. Thickness: "Thick" sod, minimum 1 inch and maximum 1-3/8 inch topsoil base.
  - 2. Machine cut sod and load on pallets in accordance with TPI (SPEC) Guidelines.
- B. Topsoil: Fertile, agricultural soil, typical for locality, capable of sustaining vigorous plant growth, taken from drained site; free of subsoil, clay, or impurities, plants, weeds and roots; pH value of minimum 5.4 and maximum 7.0.
- C. Fertilizer: Recommended for grass, with fifty percent of the elements derived from organic sources; of proportion necessary to eliminate any deficiencies of topsoil, to the following proportions unless otherwise indicated:
  - 1. Nitrogen: 16 percent.
  - 2. Phosphoric Acid: 8 percent.
  - 3. Soluble Potash: 8 percent.
- D. Water: Clean, fresh and free of substances or matter that could inhibit vigorous growth of grass.

**2.02 ACCESSORIES**

- A. Wood Pegs: Softwood, sufficient size and length to ensure anchorage of sod on slope.
- B. Wire Mesh: Interwoven hexagonal plastic mesh of 2 inch size.
- C. Edging: Galvanized steel, painted steel.
- D. Herbicide.

**2.03 SOURCE QUALITY CONTROL**

- A. Analyze to ascertain percentage of nitrogen, phosphorus, potash, soluble salt content, organic matter content, and pH value.
- B. Submit minimum 10 oz sample of topsoil proposed. Forward sample to approved testing laboratory in sealed containers to prevent contamination.
- C. Testing is not required if recent tests are available for imported topsoil. Submit these test results to the testing laboratory for approval. Indicate, by test results, information necessary to determine suitability.

**PART 3 EXECUTION****3.01 EXAMINATION**

- A. Verify that prepared soil base is ready to receive the work of this section.

**3.02 PREPARATION**

- A. Prepare subgrade in accordance with Section 31 2200.
- B. Place topsoil in accordance with Section 32 9119.
- C. Install edging at periphery of seeded areas in straight lines to consistent depth.

**3.03 FERTILIZING**

- A. Apply fertilizer in accordance with manufacturer's instructions.
- B. Apply after smooth raking of topsoil and prior to installation of sod.
- C. Apply fertilizer no more than 48 hours before laying sod.
- D. Mix thoroughly into upper 2 inches of topsoil.
- E. Lightly water to aid the dissipation of fertilizer.

**3.04 LAYING SOD**

- A. Moisten prepared surface immediately prior to laying sod.
- B. Lay sod immediately after delivery to site to prevent deterioration.
- C. Lay sod smooth and tight with no open joints visible, and no overlapping; stagger end joints 12 inches minimum. Do not stretch or overlap sod pieces.
- D. Where new sod adjoins existing grass areas, align top surfaces.
- E. Where sod is placed adjacent to hard surfaces, such as curbs, pavements, etc., place top elevation of sod 1/2 inch below top of hard surface.
- F. On slopes 4 inches per foot and steeper, lay sod perpendicular to slope and secure every row with wooden pegs at maximum 2 feet on center. Drive pegs flush with soil portion of sod.
- G. Prior to placing sod, on slopes exceeding 4 inches per foot or where indicated, place wire mesh over topsoil. Securely anchor in place with wood pegs sunk firmly into the ground.
- H. Water sodded areas immediately after installation. Saturate sod to 4 inches of soil.
- I. After sod and soil have dried, roll sodded areas to ensure good bond between sod and soil and to remove minor depressions and irregularities.

**3.05 Maintenance**

- A. Provide maintenance at no extra cost to Owner; Owner will pay for water.
- B. Provide a separate maintenance contract for specified maintenance service.
- C. Provide maintenance of sodded areas for one month from Date of Substantial Completion.
- D. Maintain sodded areas immediately after placement until grass is well established and exhibits a vigorous growing condition.
- E. Mow grass at regular intervals to maintain at a maximum height of 2-1/2 inches. Do not cut more than 1/3 of grass blade at any one mowing.
- F. Neatly trim edges and hand clip where necessary.
- G. Immediately remove clippings after mowing and trimming.
- H. Water to prevent grass and soil from drying out.
- I. Roll surface to remove irregularities.
- J. Control growth of weeds. Apply herbicides in accordance with manufacturer's instructions. Remedy damage resulting from improper use of herbicides.
- K. Immediately replace sod to areas that show deterioration or bare spots.
- L. Protect sodded areas with warning signs during maintenance period.

**END OF SECTION**





**DISINFECTION OF WATER UTILITY PIPING SYSTEMS****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Disinfection of site domestic water lines and site fire water lines specified in Section 33 1416.
- B. Testing and reporting results.

**1.02 RELATED REQUIREMENTS**

- A. Section 33 1416 - Site Water Utility Distribution Piping.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Disinfection: By the linear foot. Includes preparing, disinfecting, testing, and reporting.

**1.04 REFERENCE STANDARDS**

- A. AWWA B300 - Hypochlorites; 2011.
- B. AWWA C651 - Disinfecting Water Mains; 2005.

**1.05 SUBMITTALS**

- A. Test Reports: Indicate results comparative to specified requirements.
- B. Certificate: From authority having jurisdiction indicating approval of water system.
- C. Disinfection report:
  - 1. Type and form of disinfectant used.
  - 2. Date and time of disinfectant injection start and time of completion.
  - 3. Test locations.
  - 4. Initial and 24 hour disinfectant residuals (quantity in treated water) in ppm for each outlet tested.
  - 5. Date and time of flushing start and completion.
  - 6. Disinfectant residual after flushing in ppm for each outlet tested.
- D. Bacteriological report:
  - 1. Date issued, project name, and testing laboratory name, address, and telephone number.
  - 2. Time and date of water sample collection.
  - 3. Name of person collecting samples.
  - 4. Test locations.
  - 5. Initial and 24 hour disinfectant residuals in ppm for each outlet tested.
  - 6. Coliform bacteria test results for each outlet tested.
  - 7. Certification that water complies, or fails to comply, with bacterial standards of NBU.

**1.06 QUALITY ASSURANCE**

- A. Testing Firm: Company specializing in testing potable water systems, approved by governing authorities of the State in which the Project is located.
- B. Submit bacteriologist's signature and authority associated with testing.

**PART 2 PRODUCTS****2.01 DISINFECTION CHEMICALS**

- A. Chemicals: AWWA B300, Hypochlorite.

**PART 3 EXECUTION****3.01 EXAMINATION**

- A. Verify that piping system has been cleaned, inspected, and pressure tested.
- B. Schedule disinfecting activity to coordinate with start-up, testing, adjusting and balancing, demonstration procedures, including related systems.

**3.02 DISINFECTION**

- A. Use method prescribed by the applicable state or local codes, or health authority or water service purveyor having jurisdiction, or in the absence of any of these follow AWWA C651.
- B. Provide and attach equipment required to perform the work.
- C. Inject treatment disinfectant into piping system.
- D. Maintain disinfectant in system for 24 hours.
- E. Flush, circulate, and clean until required cleanliness is achieved; use municipal domestic water.
- F. Replace permanent system devices removed for disinfection.
- G. Pressure test system to 200 psi. Repair leaks and re-test.

**3.03 FIELD QUALITY CONTROL**

- A. Test samples in accordance with AWWA C651.

**END OF SECTION**

**SITE CONCRETE ENCASEMENT, CRADLES, SADDLES AND COLLARS****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. All work required to install and complete all concrete encasements, cradles, saddles and collars.

**1.02 RELATED REQUIREMENTS**

- A. Section 31 2316 - Excavation.
- B. Section 31 2316.13 - Trenching.
- C. Section 33 0561 - Concrete Manholes
- D. Section 33 3113 - Site Sanitary Sewerage Gravity Piping.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Encasement, Cradles, Saddles, and Collars: By the cubic yard. Includes formwork, concrete, placement accessories, consolidating and curing.

**1.04 REFERENCES**

- A. Texas Department of Transportation Standard Specification, Item 420 – Concrete for Structures.

**1.05 SUBMITTALS**

- A. Product Data: Submit manufacturer's data on manufactured products showing compliance with specified requirements.
- B. Manufacturer's Installation Instructions: Indicate installation procedures and interface required with adjacent construction for concrete accessories.

**1.06 QUALITY ASSURANCE**

- A. The testing laboratory shall sample and test concrete in accordance with geotechnical report unless otherwise indicated.

**PART 2 PRODUCTS****2.01 MATERIALS**

- A. Concrete: Shall conform to Class B in accordance with Item 420, "Concrete for Structures", TxDOT Standard Specifications.
- B. Reinforcement: If required, shall be Grade 60, deformed bars, new billet steel.

**PART 3 EXECUTION****3.01 INSTALLATION**

- A. Concrete Encasement
  1. The trench shall be excavated and fine graded to a depth conforming with details and sections shown on the plans.
  2. The pipe shall be securely tied down to prevent flotation and supported by precast concrete blocks of the same strength as the concrete for encasement.
  3. Encasement shall then be placed to a depth and width conforming with details and sections shown on the plans.
- B. Concrete Cradles

1. The trench shall be prepared and the pipe supported in the same manner as described in this Section.
  2. Concrete cradles shall be constructed in accordance with details and sections shown on the plans.
- C. Concrete Saddles
1. Pipe to receive concrete saddle shall be backfilled in accordance with Section 31 2316.13 – Trenching to the spring line.
  2. Concrete placed to a depth and width conforming with details and sections shown on the plans.
- D. Concrete Collars
1. Concrete collars shall be constructed in accordance with details and sections shown on the plans.

**3.02 CLEANING**

- A. Properly dispose of all debris, trash containers, residue, remnants and scraps which result from the work of this Section.

**END OF SECTION**

**SITE CASTINGS****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Castings shall include labor, materials, equipment and incidentals to construct manhole frames and covers, catch basin inlet frames and grates, trench drain frames and grates, and area drains.
- B. Castings include metal items which are not a part of the miscellaneous metal fabrications or metal systems in other sections of these specifications.

**1.02 RELATED SECTIONS**

- A. Section 33 0561 - Concrete Manholes

**1.03 SUBMITTALS**

- A. Shop Drawings:
  - 1. Submit Shop Drawings to the Engineer for the fabrication and erection of all casting assemblies. Include plans, elevations, and details of sections and connections. Show anchorage and accessory items.
  - 2. Include setting drawings for location and installation of castings and anchorage devices.
  - 3. Copies of manufacturer's specifications, load tables, dimension diagrams, anchor details and installation instructions.

**1.04 QUALITY ASSURANCE**

- A. Shop Assembly
  - 1. Preassemble items in the shop to the greatest extent possible, so to minimize field splicing and assembly of units at the site. Disassemble units only to the extent necessary for shipping and handling limitations. Clearly mark units for reassembly and coordinated installation.

**PART 2 PRODUCTS****2.01 MANHOLE FRAMES AND COVERS**

- A. Drainage Manholes
  - 1. Material:
    - a. Cast iron conforming to ASTM A 48, Class 30A.
  - 2. Size:
    - a. As shown on the Drawings.
  - 3. Construction:
    - a. Heavy duty suitable for H-20 loading, with bearing surfaces between frames and covers machined, fitted together, and match marked to prevent rocking.
  - 4. Product and Manufacturer:
    - a. Drainage manhole frames and covers shall be as manufactured by:
      - 1) As shown on plans for type "C" inlet.
      - 2) Alamo Ironworks No. 860-72, or equal, for Junction Box.
- B. Sanitary Sewer Manholes
  - 1. Material:
    - a. Cast iron conforming to ASTM A 48, Class 30.
  - 2. Size:
    - a. As shown on the Drawings.

3. Construction:
  - a. Heavy duty suitable for H-20 loading, with bearing surfaces between frames and covers machined, fitted together, and match marked to prevent rocking.
4. Product and Manufacturer:
  - a. Manhole frames and covers shall be as shown on the Plans, or as manufactured by an approved vendor authorized by the local jurisdictional authority.

**2.02 CATCH BASIN INLET FRAMES AND GRATES**

- A. Catch Basin Inlets
  1. Material:
    - a. Ductile iron conforming to ASTM A 536.
  2. Size:
    - a. As shown on the Drawings.
  3. Construction:
    - a. Heavy duty suitable for H-20 loading, with machine bearing surfaces.

**2.03 TRENCH DRAIN FRAMES AND GRATES**

- A. Trench Drain Frames and Grates
  1. Material:
    - a. Ductile iron conforming to ASTM A536.
  2. Size:
    - a. As shown on the Drawings.
  3. Construction:
    - a. Heavy duty suitable for H-20 loading, with machine bearing surfaces.
- B. Trench Drain: As shown on the Plans, or as manufactured by an approved vendor authorized by the local jurisdictional authority.

**2.04 AREA DRAINS**

- A. Area Drains
  1. Material:
    - a. Ductile iron conforming to ASTM A536.
  2. Size:
    - a. As shown on the Drawings.
  3. Construction:
    - a. Heavy duty suitable for H-20 loading, with machine bearing surfaces.

**2.05 DESIGN AND FABRICATION**

- A. Design all frames, covers, and grates to prevent rocking and rattling under traffic.
- B. Fabricate castings true to pattern so that component parts fit together.

**2.06 FINISH**

- A. Shall be in accordance with manufacturer's recommendation, unless shown or specified otherwise.

**PART 3 EXECUTION****3.01 INSTALLATION**

- A. Follow manufacturer's printed instructions and Shop Drawings.

- B. Set castings accurately to required locations, alignment and elevation, plumb, level, true and free of rack, measured from established lines and levels. Brace temporarily or anchor temporarily in formwork.
- C. Area and planting drains shall be installed in accordance with the manufacturer's recommendations.

**END OF SECTION**



**CONCRETE MANHOLES****PART 1 GENERAL****1.01 Section Includes**

- A. Monolithic concrete manholes with masonry transition to lid frame, covers, anchorage, and accessories.
- B. Modular precast concrete manhole sections with tongue-and-groove joints, covers, anchorage, and accessories.

**1.02 Related Requirements**

- A. Section 03 3000 - Cast-in-Place Concrete.
- B. Section 04 0511 - Masonry Mortaring and Grouting.

**1.03 Price and Payment Procedures**

- A. Manhole: By the unit. Includes excavating, concrete base pad, concrete manhole sections, FRP manhole sections, brick masonry manhole construction, brick masonry transition to cover frame, cover frame and cover, to indicated depth, forming and sealing pipe inlets and outlets.

**1.04 Reference Standards**

- A. AASHTO HB - Standard Specifications for Highway Bridges; Seventeenth Edition.
- B. ASTM A48/A48M - Standard Specification for Gray Iron Castings; 2003 (Reapproved 2012).
- C. ASTM C478/C478M - Standard Specification for Circular Precast Reinforced Concrete Manhole Sections; 2020.
- D. ASTM C923/C923M - Standard Specification for Resilient Connectors Between Reinforced Concrete Manhole Structures, Pipes, and Laterals; 2020.
- E. ASTM D3753 - Standard Specification for Glass-Fiber-Reinforced Polyester Manholes and Wetwells; 2012.
- F. ACI 530/530.1/ERTA - Building Code Requirements and Specification for Masonry Structures and Related Commentaries; 2011.
- G. Local Municipal Specifications.
- H. Local Public Service Provider Specifications.

**1.05 Submittals**

- A. Shop Drawings: Indicate manhole locations, elevations, piping sizes and elevations of penetrations.
- B. Product Data: Provide manhole covers, component construction, features, configuration, and dimensions.

**1.06 Quality Assurance**

- A. Manufacturer: Company specializing in manufacturing products specified in this section.

**1.07 Field Conditions**

- A. Cold and Hot Weather Requirements: Comply with requirements of ACI 530/530.1/ERTA or applicable building code, whichever is more stringent.

**PART 2 PRODUCTS****2.01 Concrete Manholes**

- A. Weight Rating: H 10 according to AASHTO HB.

**2.02 MANHOLES**

- A. Manhole Sections: Reinforced precast concrete in accordance with ASTM C478/C478M, with resilient connectors complying with ASTM C923/C923M.
- B. Manhole Sections: ASTM D 3753, glass-fiber reinforced polyester (only if indicated on drawings) .
- C. Concrete: As specified in Section 03 3000.
- D. Concrete Reinforcement: As specified in Section 03 3000, Class A.

**2.03 Accessories**

- A. Lid and Frame: ASTM A 48/A 48M, Class 30B Cast iron construction, machined flat bearing surface, removable bolted lid, closed lid design; heavy duty suitable for H-20 loading .
- B. Throat Rings: Reinforced concrete rings, maximum thickness of 2 inches, minimum width 5 inches, and internal diameter not less than 30 inches.
- C. Inside Epoxy Coating: Raven 805 Series High Build Epoxy Liner (125 mils thickness) or Spray Wall Polyurethane System (150 mils thickness) or approved equivalent.
- D. Manhole Insert: Rainstopper as manufactured by Southwestern Packing and Seals, or approved equivalent.
- E. Water tight manhole rings and covers shall be Trane-TEX A77 "O" ring or approved equivalent.

**2.04 CONFIGURATION**

- A. Shaft Construction: Concentric with concentric cone top section; lipped male/female dry joints; sleeved to receive pipe sections.
- B. Shape: Cylindrical.
- C. Clear Inside Dimensions: 48 inches diameter.
- D. Design Depth: As indicated on drawings.
- E. Clear Lid Opening: 30 inches diameter.
- F. Pipe Entry: Provide openings as indicated on drawings.

**PART 3 EXECUTION****3.01 Examination**

- A. Verify items provided by other sections of work are properly sized and located.
- B. Verify that built-in items are in proper location, and ready for roughing into Work.
- C. Verify excavation for manholes is correct.

**3.02 Preparation**

- A. Coordinate placement of inlet and outlet pipe or duct sleeves required by other sections.

**3.03 Installation**

- A. Establish elevations and pipe inverts for inlets and outlets as indicated in drawings.

**3.04 MANHOLES**

- A. Place concrete base pad, trowel top surface level.
- B. Place manhole sections plumb and level, trim to correct elevations, anchor to base pad.
- C. Form and place manhole cylinder plumb and level, to correct dimensions and elevations.
- D. Cut and fit for pipe.
- E. The minimum angle of flow for a connecting sewer to the direction of flow by a collection system is 90 degrees.
- F. Inverts shall conform accurately to the size and elevation of the adjoining pipes. Side inverts shall be curved and main inverts, where direction changes, shall be laid out in smooth curves of the longest possible radius which is tangent to the centerlines of adjoining pipelines.
- G. Grout base of shaft sections to achieve slope to exit piping. Trowel smooth. Contour as required.
- H. Set cover frames and covers level without tipping, to correct elevations.
- I. A minimum of 2 and a maximum of 4 throat rings shall be used to adjust the manhole top.
- J. Coordinate with other sections of work to provide correct size, shape, and location.
- K. Install manhole inserts in each vented sanitary sewer manhole in accordance with manufacturer's recommendations.
- L. Vacuum test manholes for water tightness.
- M. All new sanitary sewer manholes shall be watertight and coated with an approved sewer structural coating.
  - 1. Epoxy coating: With specified thickness application.
    - a. Raven 405 Series High Build Epoxy Liner: Required thickness 125 mils.
    - b. Spray Wall Polyurethane System: Required thickness 150 mils.

**END OF SECTION**

**SITE WATER UTILITY DISTRIBUTION PIPING****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Water pipe for site conveyance lines.
- B. Pipe valves.
- C. Fire hydrants.
- D. Valves, Fire Hydrants, Backflow Preventer, and Accessories.

**1.02 RELATED REQUIREMENTS**

- A. Section 03 3000 - Cast-in-Place Concrete.
- B. Section 21 1100 - Facility Fire-Suppression Water-Service Piping.
- C. Section 31 2316 - Excavation.
- D. Section 31 2316.13 - Trenching.
- E. Section 31 2323 - Fill.
- F. Section 33 0110.58 - Disinfection of Water Utility Piping Systems: Disinfection of site service utility water piping.
- G. Section 33 1300 - Disinfecting of Water Utility Distribution.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Pipe and Fittings: By the linear foot. Includes excavation, pipe and fittings, bedding, thrust blocks, restraint joints, connection to building service piping, and to municipal utility water source.
- B. Valves: By the unit. Includes valve, fittings and accessories.
- C. Hydrant: By the unit. Includes excavation, gravel sump, hydrant, valve, connection, and accessories.
- D. Fittings: By the ton. Includes installation and accessories.

**1.04 REFERENCE STANDARDS**

- A. Standards and specifications of local authority having jurisdiction over private potable water systems.
- B. Standards and specifications of local authority having jurisdiction over public potable water systems.
- C. ASME B16.18 - Cast Copper Alloy Solder Joint Pressure Fittings; 2012.
- D. ASME B16.22 - Wrought Copper and Copper Alloy Solder-Joint Pressure Fittings; 2013.
- E. ASTM B88 - Standard Specification for Seamless Copper Water Tube; 2014.
- F. ASTM D1785 - Standard Specification for Poly(Vinyl Chloride) (PVC) Plastic Pipe, Schedules 40, 80, and 120; 2015.
- G. ASTM D2466 - Standard Specification for Poly(Vinyl Chloride) (PVC) Plastic Pipe Fittings, Schedule 40; 2013.
- H. ASTM D2855 - Standard Practice for Making Solvent-Cemented Joints with Poly(Vinyl Chloride) (PVC) Pipe and Fittings; 1996 (Reapproved 2010).

- I. ASTM D3139 - Standard Specification for Joints for Plastic Pressure Pipes using Flexible Elastomeric Seals; 1998 (Reapproved 2011).
- J. AWS A5.8M/A5.8 - Specification for Filler Metals for Brazing and Braze Welding; 2011-AMD 1.
- K. AWWA C105/A21.5 - Polyethylene Encasement for Ductile-Iron Pipe Systems; 2010.
- L. AWWA C111/A21.11 - Rubber-Gasket Joints for Ductile-Iron Pressure Pipe and Fittings; 2012.
- M. AWWA C502 - Dry-Barrel Fire Hydrants; 2014.
- N. AWWA C504 - Rubber-Seated Butterfly Valves 3 In. (75 mm) Through 72 In. (1,800 mm); 2010.
- O. AWWA C508 - Swing-Check Valves for Waterworks Service, 2 In. (50 mm) Through 24 In. (600 mm) NPS; 2011.
- P. AWWA C509 - Resilient-Seated Gate Valves for Water Supply Service; 2009.
- Q. AWWA C600 - Installation of Ductile-Iron Water Mains and Their Appurtenances; 2010.
- R. AWWA C900 - Polyvinyl Chloride (PVC) Pressure Pipe, 4 In. Through 12 In. (100 mm Through 300 mm), for Water Transmission and Distribution; 2007.
- S. UL 246 - Hydrants for Fire-Protection Service; Current Edition, Including All Revisions.

#### **1.05 SUBMITTALS**

- A. Product Data: Provide data on pipe materials, pipe fittings, valves and accessories.
- B. Manufacturer's Certificate: Certify that products meet or exceed specified requirements.
- C. Project Record Documents: Record actual locations of piping mains, valves, connections, thrust restraints, and invert elevations and provide to the Engineer. Identify and describe unexpected variations to subsoil conditions or discovery of uncharted utilities.
- D. Provide Owner with two valve keys for each type of valve.

#### **1.06 QUALITY ASSURANCE**

- A. Perform Work in accordance with water service purveyor requirements.
- B. Perform Work in accordance with requirements of local authority having jurisdiction of private water systems.

#### **1.07 DELIVERY, STORAGE, AND HANDLING**

- A. Deliver and store valves in shipping containers with labeling in place.

#### **1.08 REGULATORY REQUIREMENTS**

- A. Conform to applicable code for materials and installation of the Work of this section.

### **PART 2 PRODUCTS**

#### **2.01 WATER PIPE**

- A. Ductile Iron Pipe: AWWA C151/A21.51:
  - 1. Fittings: Ductile iron, standard thickness.
  - 2. Joints: AWWA C111/A21.11, Styrene butadiene rubber (SBR) or vulcanized SBR gasket with rods.
  - 3. Jackets: AWWA C105/A21.5 polyethylene jacket.
- B. Copper Tubing: ASTM B88, Type K, Annealed:
  - 1. Fittings: ASME B16.18, cast copper, or ASME B16.22, wrought copper.
  - 2. Joints: Compression connection or AWS A5.8M/A5.8, BCuP silver braze.

- C. PVC Pipe: ASTM D1785, Schedule 40.
  - 1. Fittings: ASTM D2466, PVC.
  - 2. Joints: ASTM D2855, solvent weld.
- D. PVC Pipe greater than 4 inches in diameter: AWWA C900 Class 150:
  - 1. Fittings: AWWA C110, gray iron and ductile iron.
  - 2. Joints: ASTM D3139 compression gasket ring.
- E. Trace Wire: Magnetic detectable conductor, brightly colored plastic covering, imprinted with "Water Service" (14-gauge insulated) in large letters.

## 2.02 VALVES

- A. Valves: Manufacturer's name and pressure rating marked on valve body.
- B. All mechanical joint valves shall be supplied with glands, bolts, and gaskets. Bolts for mechanical joints shall be high strength low alloy steel meeting requirements of the latest revision of AWWA Standard C111.
- C. All valves shall open right (clockwise) unless otherwise specified.
- D. Valve ends shall be either flanged, mechanical joint, hub-end, push-on joint ("Ring-Tite"), or any combination thereof.
- E. Valves shall have a minimum 10 YEAR LIMITED WARRANTY from the manufacturer on material and workmanship.
- F. Gate Valves Up To 3 Inches:
  - 1. Brass or Bronze body, non-rising stem, inside screw, single wedge or disc, compression ends, with control rod, post indicator, valve key, and extension box.
- G. Gate Valves 3 Inches and Over:
  - 1. AWWA C509, iron body, bronze trim, non-rising stem with square nut, single wedge, resilient seat, flanged ends, control rod, post indicator, valve key, and extension box.
  - 2. Coatings: All coatings in contact with potable water shall be approved for potable water immersion service per ANSI/NSF Standard 61.
- H. Butterfly Valves From 24 Inches to 48 Inches:
  - 1. AWWA C504, iron body, bronze disc, resilient replaceable seat, water or lug ends, ten position lever handle.

## 2.03 VALVE BOXES

- A. Each valve box assembly shall be of cast-iron and shall consist of a base, top section, and lid as required by local authority.
- B. All valve boxes shall be of a single size with a nominal diameter of 6 inches.
- C. The valve box lid shall be so designed so that it will remain firmly seated in place when subjected to vehicular traffic.
- D. The valve box assembly shall be coated with a bituminous coating of either coal tar or asphalt basic applied to all inside and outside surfaces.

## 2.04 HYDRANTS

- A. Hydrants: AWWA C502, UL 246, dry barrel type or as required by water service purveyor or local authority having jurisdiction of private water systems.
  - 1. Inside dimension: 7 inches minimum, with minimum 5 inches diameter valve seat opening.

2. 6 inch bell or mechanical joint inlet connection with accessories, gland bolts, and gaskets.
  3. Open: Right
- B. Hydrant Extensions: Fabricate in multiples of 6 inches with rod and coupling to increase barrel length.
  - C. Hose and Streamer Connection: Match sizes with water service purveyor, two hose nozzles with a 2-1/2 inch nominal inside diameter, one pumper nozzle with a 4 inch nominal inside diameter. Nozzle threads to conform with ASA B-26 National Standard Fire Hose Coupling Screw Thread.
  - D. Hydrants shall have at least one untapped drain opening. When the main valve is in fully opened position leakage through the drain opening shall be cause for rejection.
  - E. All gaskets shall be of rubber composition, copper asbestos, lead or impregnated fiber composition.
  - F. All fire hydrants having mechanical joint inlets shall be supplied with glands, bolts, and gaskets. Bolts shall be high strength low alloy steel meeting requirements of AWWA Specification C111.
  - G. Hydrant shall have non-rising stems.
  - H. Hydrants shall be equipped with a breakable coupling on the barrel section and the stem. These couplings shall be at least two inches above the finished grade line. The breakable coupling shall be so designed that in case of traffic collision, the barrel safety flange and stem safety collar will break before any other part of the hydrant. The coupling shall be designed to afford the hydrant to rotate 360°.
  - I. Valve stems shall have a diameter of 1-1/4 inch for hydrants up to and including a 5'-0" bury. Hydrants with a bury of greater than 5'-0" shall have a stem diameter of not less than 1-3/8 inch.
  - J. Hydrant shall have a limited warranty from the manufacturer against defects in materials or workmanship for a period of ten (10) years from date of manufacture. Original Manufacturer repair and replacement parts must be used to maintain valid warranty.
  - K. Finish: Primer and two coats of enamel in color required by utility company.

## **2.05 BEDDING AND COVER MATERIALS**

- A. Bedding: As specified in Section 31 2316.13.
- B. Cover: As specified in Section 31 2316.13.

## **2.06 REDUCED PRESSURE PRINCIPLE BACKFLOW PREVENTER**

- A. Two check valve assemblies, relief valve assembly, two shutoff valves, and four test codes.
- B. Check Valve and Relief Valve Assemblies: Single cast body with replaceable seats, capable of being serviced independent of each other.
- C. Relief Valve Assembly: Between check valves, supply pressure via a sensing package integral to main body.
- D. Shutoff Valves: Each end of assembly, resilient seated.
- E. Four Test Codes: Supply side of supply shutoff valve, relief valve cover (sensing supply pressure), after first check valve (sensing zone pressure), and after second check valve (sensing demand pressure).

- F. Meet requirements of USC-FCCHR, ASSE 1013, AWWA, UL (2-1/2"-10" models), and FM (2-1/2"-10" models).
- G. Product: Must comply with water service purveyor specifications.

## **2.07 ACCESSORIES**

- A. Concrete for Thrust Restraints: Concrete type specified in Section 03 3000.
- B. Permanent and Temporary Blowoff Assemblies: As shown on plans.
  - 1. Permanent Blowoff Assembly: 2-inch galvanized iron pipe, ball valve, fittings, 6-inch valve box assembly, and concrete collar around valve box where subjected to vehicular traffic.
  - 2. Temporary Blowoff Assembly: 2-inch galvanized iron pipe, ball valve, and fittings.

## **PART 3 EXECUTION**

### **3.01 EXAMINATION**

- A. Verify that building service connection and water service purveyor main size, location, and invert are as indicated.

### **3.02 PREPARATION**

- A. Cut pipe ends square, ream pipe and tube ends to full pipe diameter, remove burrs.
- B. Remove scale and dirt on inside and outside before assembly.
- C. Prepare pipe connections to equipment with flanges or unions.

### **3.03 TRENCHING**

- A. See Section 31 2316.13 for additional requirements.
- B. Form and place concrete for pipe thrust restraints at each change of pipe direction. Place concrete to permit full access to pipe and pipe accessories.
- C. Backfill around sides and to top of pipe with cover fill, tamp in place and compact, then complete backfilling.

### **3.04 INSTALLATION - PIPE**

- A. Maintain separation of water main from sewer piping in accordance with Texas Commission on Environmental Quality (TCEQ) requirements. Minimum 9 ft separation when in parallel with sewer piping.
- B. Group piping in the same trench as other site piping work whenever practical.
- C. Establish elevations of buried piping to ensure not less than 5 feet of cover.
- D. Install ductile iron piping and fittings to AWWA C600.
- E. Route pipe in straight line.
- F. Install pipe to allow for expansion and contraction without stressing pipe or joints.
- G. Install access fittings to permit disinfection of water system performed under Section 33 0110.58.
- H. Slope water pipe and position drains at low points.
- I. Install trace wire taped to pipe, coordinate with Section 31 2316.13.

### **3.05 Installation - Valves, Hydrants, Backflow Preventers**

- A. Set valves on solid bearing. Concrete pad required for valves 12 inches and larger.



- B. Center and plumb valve box over valve. Set box cover flush with finished grade.
- C. Set hydrants plumb; locate pumper nozzle perpendicular to and facing roadway, no less than 18 inches nor more than 7 feet from face of curb.
- D. Set hydrants to grade, with nozzles at least 20 inches above ground in accordance with Section 21 1100.
- E. Provide a drainage pit 36 inches square by 24 inches deep filled with 2 inches washed gravel. Encase elbow of hydrant in gravel to 6 inches above drain opening. Do not connect drain opening to sewer.
- F. Paint hydrants in accordance with water service purveyor standards and specifications or for private hydrants with standards and specifications of local authority.

**3.06 SERVICE CONNECTIONS**

- A. Provide water service to utility company requirements .

**3.07 FIELD QUALITY CONTROL**

- A. See Section 01 4000 - Quality Requirements, for additional requirements.
- B. Flush mains to remove all dust, debris, or foreign matter.
- C. Pressure test water piping to 200 pounds per square inch.
- D. Pressure test duration shall be a minimum of 4 hours.
- E. Leakage allowance shall be no greater than listing in table at end of this section.
- F. If tests indicate Work does not meet specified requirements, remove Work, replace and retest at no cost to Owner.

**HYDROSTATIC TEST LEAKAGE ALLOWANCES (MAXIMUM) @ 200 PSI**  
**ALLOWABLE LEAKAGE IN GALLONS PER HOUR (GPH)\***

NOMINAL DIAMETER & TYPE PIPE	100 LF	200 LF	300 LF	400 LF	500 LF	600 LF	700 LF	800 LF	900 LF	1000 LF
6" DI**	0.13	0.25	0.38	0.51	0.64	0.6	0.89	1.02	1.14	1.27
8" DI**	0.17	0.34	0.51	0.68	0.85	1.02	1.19	1.36	1.53	1.7
12" DI**	0.26	0.51	0.77	1.02	1.28	1.53	1.79	2.04	2.3	2.55
16" DI**	0.34	0.68	1.02	1.36	1.7	2.04	2.38	2.72	3.06	3.4
20" DI**	0.43	0.85	1.28	1.7	2.13	2.55	2.98	3.4	3.83	4.25
20" CSC	0.08	0.16	0.24	0.32	0.4	0.47	0.55	0.63	0.71	0.79
24" DI**	0.51	1.02	1.53	2.04	2.55	3.06	3.57	4.08	3.59	5.1
24" CSC	0.1	0.19	0.29	0.38	0.48	0.57	0.67	0.76	0.86	0.95
30" DI**	0.64	1.27	1.91	2.55	3.19	3.82	4.46	5.1	5.73	6.37
30" CSC	0.12	0.24	0.35	0.47	0.59	0.71	0.83	0.94	1.06	1.18
36" DI**	0.76	1.53	2.29	3.06	3.82	4.58	5.35	6.11	6.88	7.64
36" CSC	0.14	0.28	0.43	0.57	0.71	0.85	0.99	1.14	1.28	1.42
42" DI**	0.89	1.78	2.68	3.57	4.46	5.35	6.24	7.14	8.03	8.92
42" CSC	0.17	0.33	0.5	0.66	0.83	1	1.16	1.33	1.49	1.66
48" DI**	1.02	2.04	3.06	4.08	5.1	6.11	7.13	8.15	9.17	10.1 9
48" CSC	0.19	0.38	0.7	0.76	0.95	1.13	1.32	1.51	1.7	1.89
54" CSC	0.21	0.42	0.63	0.84	1.05	1.26	1.47	1.68	1.89	2.1
60" CSC	0.23	0.46	0.69	0.92	1.15	1.38	1.61	1.84	2.07	2.3

\*PVC pipe shall be tested to DI pressures. GPH for CSC pipe are manufacturer's maximum.

\*\*DI pipe includes mechanical and push-on joints.

END OF SECTION

**SITE SANITARY SEWERAGE GRAVITY PIPING****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Sanitary sewerage drainage piping, fittings, and accessories.
- B. Connection of building sanitary drainage system to public sanitary sewers.
- C. Cleanout access.

**1.02 RELATED REQUIREMENTS**

- A. Section 03 3000 - Cast-in-Place Concrete: Concrete for cleanout base pad construction.
- B. Section 31 2316 - Excavation: Excavating of trenches.
- C. Section 31 2316.13 - Trenching: Excavating, bedding, and backfilling.
- D. Section 33 0561 - Concrete Manholes.

**1.03 DEFINITIONS**

- A. Bedding: Fill placed under, beside and directly over pipe, prior to subsequent backfill operations.

**1.04 REFERENCE STANDARDS**

- A. ASTM A74 - Standard Specification for Cast Iron Soil Pipe and Fittings; 2015.
- B. ASTM A746 - Standard Specification for Ductile Iron Gravity Sewer Pipe; 2009 (Reapproved 2014).
- C. ASTM C12 - Standard Practice for Installing Vitrified Clay Pipe Lines; 2014.
- D. ASTM C14 - Standard Specification for Nonreinforced Concrete Sewer, Storm Drain, and Culvert Pipe; 2015.
- E. ASTM C14M - Standard Specification for Nonreinforced Concrete Sewer, Storm Drain, and Culvert Pipe (Metric); 2015.
- F. ASTM C76 - Standard Specification for Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe; 2015.
- G. ASTM C76M - Standard Specification for Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe (Metric); 2014.
- H. ASTM C425 - Standard Specification for Compression Joints for Vitrified Clay Pipe and Fittings; 2004 (Reapproved 2013).
- I. ASTM C443 - Standard Specification for Joints for Concrete Pipe and Manholes, Using Rubber Gaskets; 2012.
- J. ASTM C443M - Standard Specification for Joints for Concrete Pipe and Manholes, Using Rubber Gaskets (Metric); 2011.
- K. ASTM C564 - Standard Specification for Rubber Gaskets for Cast Iron Soil Pipe and Fittings; 2014.
- L. ASTM C700 - Standard Specification for Vitrified Clay Pipe, Extra Strength, Standard Strength, and Perforated; 2013.
- M. ASTM D1785 - Standard Specification for Poly(Vinyl Chloride) (PVC) Plastic Pipe, Schedules 40, 80, and 120; 2015.

- N. ASTM D2321 - Standard Practice for Underground Installation of Thermoplastic Pipe for Sewers and Other Gravity-Flow Applications; 2014.
- O. ASTM D2680 - Standard Specification for Acrylonitrile-Butadiene-Styrene (ABS) and Poly(Vinyl Chloride) (PVC) Composite Sewer Piping; 2001 (Reapproved 2014).
- P. ASTM D2729 - Standard Specification for Poly(Vinyl Chloride) (PVC) Sewer Pipe and Fittings; 2011.
- Q. ASTM D3034 - Standard Specification for Type PSM Poly(Vinyl Chloride) (PVC) Sewer Pipe and Fittings; 2015.
- R. ASTM D3350 - Standard Specification for Polyethylene Plastics Pipe and Fittings Material; 2012.
- S. AWWA C111/A21.11 - Rubber-Gasket Joints for Ductile-Iron Pressure Pipe and Fittings; 2012.

#### **1.05 ADMINISTRATIVE REQUIREMENTS**

- A. Coordination: Coordinate the installation of the sanitary sewer with the local authority having jurisdiction for permits and inspections with regard to size, location and installation of service utilities.
- B. Preinstallation Meeting: Conduct a preinstallation meeting one week prior to the start of the work of this section; require attendance by all affected installers, and local inspection authorities.
- C. Sequencing: Ensure that utility connections are achieved in an orderly and expeditious manner.

#### **1.06 SUBMITTALS**

- A. Product Data: Provide data indicating pipe and pipe accessories.
- B. Manufacturer's Certificate: Certify that products meet or exceed specified requirements.
- C. Field Quality Control Submittals: Document results of field quality control testing.
- D. Project Record Documents:
  - 1. Record location of pipe runs, connections, manholes, and invert elevations.
  - 2. Identify and describe unexpected variations to subsoil conditions or discovery of uncharted utilities.

### **PART 2 PRODUCTS**

#### **2.01 SEWER PIPE MATERIALS**

- A. Provide products that comply with applicable code(s).
- B. Plastic Pipe: ASTM D 3034, SDR 26, Type PSM, Poly(Vinyl Chloride) (PVC) material, inside nominal diameter as shown on drawings, bell and spigot style joint end with the exception that solvent cement joints shall not be used.
- C. Fittings: Same material as pipe molded or formed to suit pipe size and end design, in required tee, bends, elbows, cleanouts, reducers, traps and other configurations required.

#### **2.02 PIPE ACCESSORIES**

- A. Trace Wire: Magnetic detectable conductor, clear plastic covering, imprinted with "Sewer Service " in large letters, if required.

**2.03 BEDDING AND COVER MATERIALS**

- A. Pipe Bedding Material: As specified in Section 31 2316.13.
- B. Pipe Cover Material: As specified in Section 31 2316.13.

**PART 3 EXECUTION****3.01 GENERAL**

- A. Perform work in accordance with applicable code(s) of local or state jurisdictional authority.

**3.02 TRENCHING**

- A. See Section 31 2316.13 for additional requirements.
- B. Backfill around sides and to top of pipe with cover fill, tamp in place and compact, then complete backfilling.

**3.03 INSTALLATION - PIPE**

- A. Verify that trench cut is ready to receive work and excavations, dimensions, and elevations are as indicated on layout drawings.
- B. Install pipe, fittings, and accessories in accordance with manufacturer's instructions. Seal watertight.
- C. Lay pipe to slope gradients noted on drawings; with maximum variation from true slope of 1/8 inch in 10 feet.
- D. Connect to building sanitary sewer outlet and public sanitary sewer system.
- E. Install cleanouts a minimum of every 100 linear feet unless otherwise indicated on drawings. See drawings for cleanout detail.
- F. Install trace wire 6 inches above top of pipe, if required. Coordinate with Section 31 2316.13.

**3.04 WATER MAIN CROSSINGS**

- A. Requirements of Texas Commission on Environmental Quality, 30 TAC § 217.5 shall be met.

**3.05 INSTALLATION - CLEANOUTS**

- A. Form bottom of excavation clean and smooth to correct elevation.
- B. Establish elevations and pipe inverts for inlets and outlets as indicated.
- C. Mount cap and frame to match slope of finished grade. Secure to top of pipe at elevation indicated.

**3.06 FIELD QUALITY CONTROL**

- A. Perform field inspection and testing.
- B. If tests indicate Work does not meet specified requirements, remove Work, replace and retest at no cost to Owner.
- C. Pressure Test: Pressure test in accordance with sewer service purveyor standards or local authority having jurisdiction of private sanitary sewer systems.
- D. Deflection Test: Deflection test in accordance with sewer service purveyor or local authority having jurisdiction of private sanitary sewer systems.
- E. Obtain inspections from sewer service purveyor or local authority having jurisdiction. Do not bury pipe until inspection is approved.

**3.07 PROTECTION**

- A. Protect pipe and bedding cover from damage or displacement until backfilling operation is in progress.

**END OF SECTION**

**SUBDRAINAGE****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Building Perimeter, Retaining Wall, and Under-Slab Drainage Systems.
- B. Filter aggregate and fabric and bedding.

**1.02 RELATED REQUIREMENTS**

- A. Section 31 2316 - Excavation.
- B. Section 31 2316.13 - Trenching.
- C. Section 31 2316.14 - Trench Excavation Protection.
- D. Section 31 2323 - Fill.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Pipe and Fittings: By linear \_\_\_\_ feet. Includes hand trimming excavating, bedding, pipe and fittings, filter aggregate, filter fabric connecting to municipal sewer.

**1.04 REFERENCE STANDARDS**

- A. ASTM C4 - Standard Specification for Clay Drain Tile and Perforated Clay Drain Tile; 2004 (Reapproved 2014).
- B. ASTM C412 - Standard Specification for Concrete Drain Tile; 2015.
- C. ASTM C412M - Standard Specification for Concrete Drain Tile (Metric); 2015.
- D. ASTM D2729 - Standard Specification for Poly(Vinyl Chloride) (PVC) Sewer Pipe and Fittings; 2011.

**1.05 SUBMITTALS**

- A. Shop Drawings: Indicate dimensions, layout of piping, high and low points of pipe inverts, gradient of slope between corners and intersections.
- B. Product Data: Provide data on pipe drainage products, pipe accessories, filter fabric and gravel bedding material.
- C. Manufacturer's Certificate: Certify that products meet or exceed specified requirements.
- D. Project Record Documents: Record location of pipe runs, connections, cleanouts and principal invert elevations.

**PART 2 PRODUCTS****2.01 PIPE MATERIALS**

- A. Polyvinyl Chloride Pipe: ASTM D 2729; tapered pipe end and coupling joints, 4 inch inside diameter; with required fittings unless otherwise indicated on drawings.
- B. Bituminous Fiber Pipe: With Type tapered pipe end and coupling joints; 4 inch inside diameter; with necessary fittings unless otherwise indicated on drawings.
- C. Corrugated Plastic Tubing: Flexible type; 4 inch diameter, with required fittings unless otherwise indicated on drawings.
- D. Use perforated pipe along horizontal subdrainage system; unperforated through sleeved walls or vertical subdrainage system.

**2.02 AGGREGATE AND BEDDING**

- A. Filter Aggregate and Bedding Material: Granular fill or gravel as specified in Section 31 2323.
- B. Filter Sand and Bedding Material: Sand as specified in Section 31 2323.

**2.03 ACCESSORIES**

- A. Pipe Couplings: Solid plastic.
- B. Filter Fabric: Water pervious, black.

**PART 3 EXECUTION****3.01 EXAMINATION**

- A. Verify that trench cut or excavated base is ready to receive work and excavations, dimensions, and elevations are as indicated on layout Drawings.

**3.02 PREPARATION**

- A. Hand trim excavations to required elevations. Correct over-excavation with Type A1 aggregate.
- B. Remove large stones or other hard matter that could damage drainage piping or impede consistent backfilling or compaction.

**3.03 INSTALLATION**

- A. Install and join pipe and pipe fittings in accordance with pipe manufacturer's instructions.
- B. Place drainage pipe on gravel bedding material.
- C. Lay pipe to slope gradients noted on drawings; with maximum variation from true slope of 1/8 inch in 10 feet.
- D. Place pipe with perforations facing down. Mechanically join pipe ends.
- E. Install pipe couplings.
- F. Install filter aggregate at sides and top of pipe. Provide top cover compacted thickness of 12 inches minimum or as indicated on drawings.
- G. Place filter fabric over levelled top surface of aggregate cover prior to subsequent backfilling operations.
- H. Place aggregate in maximum 8 inch lifts, compacting each lift.
- I. Refer to Section 31 2323 for compaction requirements. Do not displace or damage pipe when compacting.
- J. Place impervious fill over drainage pipe aggregate cover and compact.
- K. Connect to storm drain system with unperforated pipe .
- L. Coordinate the Work with connection to storm drain system, and trenching.

**3.04 FIELD QUALITY CONTROL**

- A. Section 01 4000 - Quality Requirements: Field inspection and testing.
- B. Request inspection prior to and immediately after placing aggregate cover over pipe.



**3.05 PROTECTION**

- A. Protect pipe and aggregate cover from damage or displacement until backfilling operation begins.

**END OF SECTION**

**STORMWATER GRAVITY PIPING****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Stormwater drainage piping.
- B. Stormwater pipe accessories.

**1.02 RELATED REQUIREMENTS**

- A. Section 03 3000 - Cast-in-Place Concrete.
- B. Section 31 2316 - Excavation.
- C. Section 31 2316.13 - Trenching.
- D. Section 31 2316.14 - Trench Excavation Protection.
- E. Section 31 2323 - Fill.
- F. Section 33 0561 - Concrete Manholes.
- G. Section 33 4213 - Pipe Culverts.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Pipe and Fittings:
  - 1. Basis of Measurement: By the linear foot.
  - 2. Basis of Payment: Includes hand trimming excavation, bedding and backfilling, pipe and fittings, connection to building service piping and to municipal system.
- B. Catch Basins and Cleanouts:
  - 1. Basis of Measurement: By the unit.

**1.04 REFERENCE STANDARDS**

- A. ASTM A74 - Standard Specification for Cast Iron Soil Pipe and Fittings; 2015.
- B. ASTM C76 - Standard Specification for Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe; 2015.
- C. ASTM D 667 - Standard Specification for Large Diameter Corrugated Polyethylene Pipe and Fittings.
- D. ASTM C443 - Standard Specification for Joints for Concrete Pipe and Manholes, Using Rubber Gaskets; 2012.
- E. ASTM C564 - Standard Specification for Rubber Gaskets for Cast Iron Soil Pipe and Fittings; 2014.
- F. ASTM D1785 - Standard Specification for Poly(Vinyl Chloride) (PVC) Plastic Pipe, Schedules 40, 80, and 120; 2015.
- G. ASTM D2321 - Standard Practice for Underground Installation of Thermoplastic Pipe for Sewers and Other Gravity-Flow Applications; 2014.
- H. ASTM D2729 - Standard Specification for Poly(Vinyl Chloride) (PVC) Sewer Pipe and Fittings; 2011.
- I. ASTM F 2648 - Standard Specification for 2 to 60-inch Annular Corrugated Profile Wall Polyethylene Pipe and Fittings.

**1.05 ADMINISTRATIVE REQUIREMENTS**

- A. Coordination: Coordinate the installation of storm drains with local authority having jurisdiction for permits and inspections with regard to size, material, location and installation of service utilities.
- B. Preinstallation Meeting: Conduct a preinstallation meeting one week prior to the start of the work of this section; require attendance by all affected installers and local inspection authorities.
- C. Sequencing: Ensure that utility connections are achieved in an orderly and expeditious manner.

**1.06 SUBMITTALS**

- A. Product Data: Provide data indicating pipe and pipe accessories .
- B. Manufacturer's Installation Instructions: Indicate special procedures required to install Products specified.
- C. Manufacturer's Certificate: Certify that materials meet or exceed specified requirements.
- D. Project Record Documents:
  - 1. Record location of pipe runs, connections, and invert elevations.
  - 2. Identify and describe unexpected variations to subsoil conditions or discovery of uncharted utilities.

**PART 2 PRODUCTS****2.01 STORM SEWER PIPE MATERIALS**

- A. Cast Iron Soil Pipe: ASTM A 74, Extra Heavy grade, hub and spigot end.
- B. Concrete Pipe Joint Devices: ASTM C443 (ASTM C443M) rubber compression gasket joint.
- C. Concrete Pipe: Reinforced, ASTM C 76 (ASTM C 76M), Class II with Wall type A; mesh reinforcement; bell and spigot end joints.
- D. Reinforced Concrete Pipe Joint Device: ASTM C443 (ASTM C443M) rubber compression gasket joint.
- E. Plastic Pipe: ASTM D 2751, SDR 35, Acrylonitrile-Butadiene-Styrene (ABS) material; bell and spigot style solvent sealed joint end.
- F. Plastic Pipe: ASTM D 2729, Poly(Vinyl Chloride) (PVC) material; bell and spigot style solvent sealed joint end.
- G. Plastic Pipe: ASTM D 3034, Type PSM, Poly(Vinyl Chloride) (PVC) material; bell and spigot style solvent sealed joint end.
- H. Plastic Pipe: ASTM D 1785, Schedule 40, Poly(Vinyl Chloride) (PVC) material; bell and spigot style solvent sealed joint end.
- I. Corrugated Steel Pipe: AASHTO M 36M Type I; helical lock seam; coated inside and out with 0.050 inch thick bituminous coating.
- J. Coupling Bands: Galvanized steel, 0.052 inches thick x 10 inches (250 mm) wide; connected with two neoprene "O" ring gaskets and two galvanized steel bolts.
- K. ADS N-12 WT 1B Pipe: ASTM F 2648; 4 inches to 60 inches High Density Polyethylene; bell and spigot ends with a gasket seal.

**2.02 PIPE ACCESSORIES**

- A. Fittings: Same material as pipe molded or formed to suit pipe size and end design, in required tee, bends, elbows, cleanouts, reducers, traps and other configurations required.
- B. Filter Fabric: Non-biodegradable, woven.

**2.03 CATCH BASIN, TRENCH DRAIN, CLEANOUT, AND AREA DRAIN COMPONENTS**

- A. Lids and Drain Covers: Cast iron.
  - 1. Catch Basin:
    - a. H-20 loaded.
    - b. Nominal Lid and Frame Size: As shown on drawings.
  - 2. Cleanout:
    - a. Lid Design: As shown on drawings.
    - b. H-20 loaded.
    - c. Nominal Lid and Frame Size: As shown on drawings.
  - 3. Area Drain:
    - a. Lid Design: As shown on drawings.
    - b. H-20 loaded.
    - c. Nominal Lid and Frame Size: As shown on drawings.
- B. Trench Drain System: Trench drain system assembled from factory fabricated, concrete castings in standard lengths; with or without built in slope; with integral joints and optional grating support rails; includes grating.
  - 1. Grating Material and Style: Slotted cast iron.
  - 2. Trench Width: As shown on the drawings.
  - 3. Trench Section Length: As shown on the drawings.

**2.04 BEDDING AND COVER MATERIALS**

- A. Bedding: As specified in Section 31 2316.13.
- B. Cover: As specified in Section 31 2316.13.

**PART 3 EXECUTION****3.01 TRENCHING**

- A. See Section 31 2316.13 for additional requirements.
- B. Hand trim excavation for accurate placement of pipe to elevations indicated.
- C. Backfill around sides and to top of pipe with cover fill, tamp in place and compact, then complete backfilling.

**3.02 INSTALLATION**

- A. Verify that trench cut or excavation base is ready to receive work and excavations, dimensions, and elevations are as indicated on drawings.
- B. Install pipe, fittings, and accessories in accordance with manufacturer's instructions. Seal watertight.
  - 1. Plastic Pipe: Also comply with ASTM D2321.
- C. Lay pipe to slope gradients noted on layout drawings; with maximum variation from true slope of 1/8 inch in 10 feet.

- D. Connect to building storm drainage system, foundation drainage system, and utility/municipal system.
- E. Make connections through walls through sleeved openings, where provided.

**3.03 INSTALLATION - CATCH BASINS, TRENCH DRAINS AND CLEANOUTS**

- A. Form and place cast-in-place concrete base pad, with provision for storm sewer pipe end sections.

**3.04 FIELD QUALITY CONTROL**

- A. If tests indicate Work does not meet specified requirements, remove Work, replace and retest at no cost to Owner.

**3.05 PROTECTION**

- A. Protect pipe and bedding cover from damage or displacement until backfilling operation is in progress.

**END OF SECTION**

**STORMWATER CULVERTS****PART 1 GENERAL****1.01 SECTION INCLUDES**

- A. Installation of pipe culvert, joints and accessories.
- B. Bedding and slope protection at pipe end.

**1.02 RELATED REQUIREMENTS**

- A. Section 03 3000 - Cast-in-Place Concrete.
- B. Section 31 2316 - Excavation.
- C. Section 31 2316.13 - Trenching.
- D. Section 31 2316.14 - Trench Excavation Protection
- E. Section 31 2323 - Fill.
- F. Section 31 3700 - Riprap.
- G. Section 33 4111 - Site Storm Utility Drainage Piping.

**1.03 PRICE AND PAYMENT PROCEDURES**

- A. Pipe Culvert: By the total linear foot invert length of pipe including tapered ends and the diameter in inches. Includes excavating; removing soft subsoil, bedding fill, compacting; pipe, catch basins, junction boxes, fittings and accessories assembled; repair of damaged coating and connection to existing pipe culverts.

**1.04 REFERENCE STANDARDS**

- A. ASTM A929/A929M - Standard Specification for Steel Sheet, Metallic-Coated by the Hot-Dip Process for Corrugated Steel Pipe; 2001 (Reapproved 2013).
- B. ASTM C14 - Standard Specification for Nonreinforced Concrete Sewer, Storm Drain, and Culvert Pipe; 2015.
- C. ASTM C14M - Standard Specification for Nonreinforced Concrete Sewer, Storm Drain, and Culvert Pipe (Metric); 2015.
- D. ASTM C76 - Standard Specification for Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe; 2015.
- E. ASTM C443 - Standard Specification for Joints for Concrete Pipe and Manholes, Using Rubber Gaskets; 2012.
- F. ASTM F 2698 - Standard Specification for 2 to 60-inch Annular Corrugated Profile Wall Polyethylene Pipe and Fittings for Land Drainage Applications.

**1.05 ADMINISTRATIVE REQUIREMENTS**

- A. Preinstallation Meeting: Convene one week prior to commencing work of this section.

**1.06 PRODUCT DELIVERY, STORAGE AND HANDLING**

- A. Handle all pipe, fittings and accessories carefully with handling devices. Do not drop or roll pipe off trucks. Do not otherwise drop, roll or skid pipe. Chains or cables will not be allowed to load or unload pipe. Materials cracked, gouged, chipped, dented or otherwise damaged will not be allowed for use on the project.
- B. Store pipe and fittings on heavy wood blocking or platforms so they are not in contact with the ground.

- C. Pipe, fittings and specials shall be loaded opposite to or as close to the place where it is to be laid as is practical to avoid unnecessary handling. Interior shall be kept completely free from dirt and foreign matter.
- D. Obtain each type of pipe and fittings from a single manufacturer.

#### 1.07 SUBMITTALS

- A. Product Data: Provide data on pipe, fittings and accessories.
- B. Manufacturer's Installation Instructions: Indicate special procedures required to install Products specified.
- C. Accurately record actual locations of pipe runs, connections, and invert elevations.
- D. Identify and describe unexpected variations to subsoil conditions or discovery of uncharted utilities.

#### 1.08 REGULATORY REQUIREMENTS

- A. Conform to applicable or municipal code for materials and installation of the work of this section.

### PART 2 PRODUCTS

#### 2.01 CULVERT PIPE

- A. Pipe
  - 1. Pipe for culverts shall be of the following types, manufactured in accordance with the requirements of the specifications for each type:
    - a. Corrugated Steel Pipe: Fabricated of ASTM A 929/A 929M galvanized steel sheet:
      - 1) Helical lock seam.
      - 2) Coated inside and out with 0.050 inch thick bituminous coating.
      - 3) Shape: Circular.
      - 4) Tapered Ends: Same material as pipe, machine cut, for joining to pipe end.
      - 5) Coupling Bands: Galvanized steel, 0.052 inches thick x 10 inches wide; connected with two neoprene "O" ring gaskets and two galvanized steel bolts.
    - b. Reinforced Concrete Culvert Pipe: Reinforced concrete culvert pipe shall conform to all requirements of ASTM Standard Specification C-76. The minimum "D-Load" for design of the pipe under Specification C-655, or the selection of pipe class under Specification C-76.
    - c. Polyvinyl Chloride Pipe: Polyvinyl chloride pipe and fittings PVC 1120, conforming to ASTM D1785-76.
    - d. PVC Pipe and Fittings: Standard and perforated PVC pipe and fittings conforming to ASTM D2729 sewer piping or D2665 DWV piping.
    - e. Ductile Iron Pipe and Fittings: Ductile iron pipe and fittings shall conform to all requirements of AWWA C 151 and C 111 and shall be equivalent to a class 150 pipe.
    - f. High Density Polyethylene Pipe (HDPE): High Density Polyethylene double wall pipe and fittings conforming to all requirements of ASTM F 2698; ADS N-12 or equivalent.
- B. Joints
  - 1. Reinforced Concrete Pipe Jointing Materials: Jointing material shall be either a plastic type joint or cement grouted joint. Where cement grouted joints are specified, the

Contractor may, at his expense, substitute plastic type joints after review by the Engineer.

- a. Rubber Gasket Joints: Where rubber gasket pipe joints are required, the joint assembly shall be made according to the recommendations of the gasket manufacturer. Water tight joints will be required when using rubber gaskets.
- b. Plastic Joints: Plastic joints for reinforced concrete culvert pipe of all kinds shall meet the requirements covered in either Paragraph (13)(b) or Paragraph (13)(d), Section 464.2, of Item 464, "Reinforced Concrete Pipe Culverts", of the Texas Department of Transportation Standard Specifications.
- c. Cement Mortar Grouted Joints: Pipe joints shall be made with Portland Cement Mortar. All joints of concrete pipe shall be filled with stiff mortar composed of one part Portland Cement and not more than two parts sand. The mortar shall be used within 30 minutes from the time that the ingredients are mixed with water.
- d. Diaper Bands: Diaper bands shall be placed at every joint when the pipe is laid on a curve. Gauze or cloth fabric bands to hold grout in place at joints shall be made and used as follows:
  - 1) The fabric bands shall be cut into such lengths that they will extend the full circumference of the pipe. The longitudinal edges of fabric bands shall be rolled and stitched around two (2) pieces of wire. The width of the fabric bands shall be such that after being filled with mortar, a band will be formed. The wires shall be cut into such lengths that they pass around the pipe with sufficient extra length for the ends to be twisted at the top of the pipe to hold the fabric securely in place, and shall be accurately centered around the joint.
2. PVC Joints: Materials and methods shall be in accordance with the manufacturer's recommendations.

## **2.02 BEDDING AND COVER MATERIALS**

- A. Bedding: As specified in Section 31 2316.13.
- B. Cover: As specified in Section 31 2316.13.

## **2.03 ACCESSORIES**

- A. Filter Fabric: Non-biodegradable, woven.
- B. Fill at Pipe Ends: Riprap as specified in Section 31 3700.

## **PART 3 EXECUTION**

### **3.01 EXCAVATING**

- A. All excavation shall be in accordance with the requirements of Section 31 2316.13 and 31 2316.14 except where tunneling or jacking methods are shown on the plans or permitted by the Engineer. The Contractor shall make such temporary provision as may be necessary to insure adequate drainage of the trench and bedding during the construction operation.

### **3.02 INSTALLATION - PIPE**

- A. Verify that trench cut is ready to receive work and excavations, dimensions, and elevations are as indicated on layout drawings.
- B. Install pipe and accessories in accordance with manufacturer's instructions
- C. Lift or roll pipe into position. Do not drop or drag pipe over prepared bedding.



- D. Shore pipe to required position; retain in place until after compaction of adjacent fills. Ensure pipe remains in correct position and to required slope.
- E. Repair surface damage to pipe protective coating with two coats of compatible bituminous paint coating.
- F. Install culvert end gratings.

### 3.03 JOINTING

- A. Reinforced Concrete Pipe
  - 1. Application of the jointing material and construction methods recommended by the manufacturer shall be strictly adhered to.
  - 2. Rubber Gasket Joints:
    - a. Rubber gasket joints shall be made with materials specifically intended for the purpose intended and installed according to the manufacturer's recommendations.
  - 3. Plastic Joints:
    - a. Plastic joints shall be made with Cold Applied Plastic Asphalt Sewer Joint Compound or Cold Applied Preformed Plastic Gaskets meeting the requirements as set forth earlier in these sections.
  - 4. Mortar Joints:
    - a. All pipe shall be joined tight and sealed with stiff mortar, composed of one part Portland and not more than two parts sand, so placed as to form a durable water tight joint.
    - b. The ends of the pipe shall be cleaned thoroughly and wetted before making the joint.
    - c. After any section of pipe is laid an even layer of mortar shall be thoroughly troweled onto the lower half of the bell or groove of the last laid section.
    - d. Mortar shall be applied to the upper portion of the tongue or spigot of the pipe section being laid. The spigot or tongue end of the next section of pipe shall then be inserted and the joint pulled up tight, taking care that the inner surfaces of the abutting pipe sections are flush and even.
    - e. After the section is laid and uniformly matched and fitted as close as the construction of the pipe will permit, the lower half of the inner circumference of the joints shall be sealed and packed with mortar and finished smooth and even with the adjacent section of pipe.
    - f. Before this mortar has attained initial set, additional mortar then shall be applied from the outside and forced into the unfilled portion of the bell or groove to fill completely the annular space around the spigot or tongue.
    - g. For bell and spigot pipe, a bead shall be formed on the outside by troweling on mortar downward at an angle of 45 degrees from the outer edge of the bell to the spigot of the last laid section.
    - h. For tongue and groove pipe, a bead shall be formed extending at least 1 inch on either side of the joint and of approximately semi-circular cross-section.
    - i. After the initial set, the mortar on the outside shall be protected from air and sun with thoroughly wetted earth or burlap cover or acceptable equivalent, and shall be kept wet for a minimum of 48 hours or until the backfill has been completed. No jointing shall be done when the atmospheric temperature is at or below 40°F, and when necessary, because of a sudden drop in temperature, joints shall be protected against freezing for at least 24 hours.

5. Diaper Bands:
  - a. After diaper bands have been secured in place, grout, composed of one (1) part Portland Cement and not more than two (2) parts sand, shall be poured between the gauze band and the pipe, to insure a thorough sealing of the joint.
  - b. The completed joints shall immediately be protected from the air and sun with proper covering and shall be kept protected for such period as necessary to secure satisfactory curing of the mortar.
  - c. The placing of this type joint shall be kept at least five joints behind the actual laying of the pipe.
  - d. No backfilling around joints shall be done until the joints have cured for a minimum of 24 hours.
- B. PVC Pipe
  1. PVC pipe less than six inches (6") in diameter shall be joined with PVC solvent cement conforming to ASTM D 2564, or manufacturer's recommendations.
  2. PVC pipe six inches (6") in diameter and larger shall be joined with gasket type connections in accordance with the manufacturer's recommendation.
- C. HDPE Pipe
  1. Shall be joined with gasket type connections in accordance with manufacturer's recommendations.

### 3.04 PIPE ENDS

- A. Place fill at pipe ends, at embankment slopes.

### 3.05 TOLERANCES

- A. Lay pipe to alignment and slope gradients noted on layout drawings; with maximum variation from true slope of 1/8 inch in 10 feet.
- B. Maximum Variation From Intended Elevation of Culvert Invert: 1/4 inch.
- C. Maximum Offset of Pipe From True Alignment: 1 inch.
- D. Maximum Variation in Profile of Structure From Intended Position: 0.25 percent.

### 3.06 PROTECTION

- A. Protect pipe and bedding from damage or displacement until backfilling operation is in progress.

### 3.07 BACKFILL

- A. After the pipe has been placed, bedded, and jointed as specified filling and/or backfilling shall be done in accordance with the requirements of Section 31 2316.13 - Trenching.

**END OF SECTION**